

January 16, 2025

Planning Commission

Case 2024-7998: Los Prados Preliminary Subdivision Plat

ATTACHMENT A

Applicant Materials



JENKINSGAVIN
LAND USE | PROJECT MANAGEMENT

June 27, 2024

Janice Biletznikoff, Senior Planner
Current Planning Division
City of Santa Fe
200 Lincoln Ave.
Santa Fe, NM 87501

**RE: Los Prados, Phase 1 – Preliminary Subdivision Plat Application- Revisions
3730 South Meadows Road**

Dear Janice:

This letter is respectfully submitted on behalf of Homewise, Inc. in application for Preliminary Subdivision Plat approval for the Los Prados community located at 3730 South Meadows Road. The 14.41-acre subject parcel is zoned R-6 (6 Dwellings per Acre).

Background

On February 8, 2023, the City Council approved General Plan Amendment, Rezone, and Master Plan applications permitting 97 single family dwelling units (Phase 1), 64 condominiums (Phase 2), and a 5.12-acre Neighborhood Park (Phase 1).

Project Description

Phase 1 of Los Prados will be developed on Tract 4-A totaling 14.41 acres and will comprise 42 townhomes, 55 single family detached homes, for a total of 97 homes as permitted by the Master Plan, and a 5.12-acre Neighborhood Park. The Park will be developed at the Applicant's expense on Tract 4-B, which was dedicated to the City of Santa Fe pursuant to the Lot Split Plat recorded on November 16, 2023 (attached).

Subdivision Approval Criteria

The Subdivision approval criteria in §14-3.7(C) are addressed below:

- (1) In all subdivisions, due regard shall be shown for all natural features such as vegetation, water courses, historical sites and structures, and similar community assets that, if preserved, will add attractiveness and value to the area or to Santa Fe.*

The subject parcel is undeveloped and gently sloped. The development of a 5.12-acre Neighborhood Park on the adjacent parcel will preserve natural open space and provide much needed outdoor recreational opportunities for the community.

- (2) The planning commission shall give due regard to the opinions of public agencies and shall not approve the plat if it determines that in the best interest of the public health, safety or welfare the land is not suitable for platting and development purposes of the kind proposed. Land subject to flooding and land deemed to be topographically unsuited for building, or for other reasons uninhabitable, shall not be for residential occupancy, nor for other uses that may increase danger to health, safety or welfare or aggravate erosion or flood hazard. Such land shall be set aside within the plat for uses that will not be endangered by periodic or occasional inundation or produce unsatisfactory living conditions. See also Section 14-5.9 (Ecological Resource Protection Overlay District) and Section 14-8.3 (Flood Regulations).*

The subject property is gently sloped and contains no arroyos, floodways, or steep areas subject to erosion. The Master Plan was approved for the project partly in recognition of the fact that the land is suitable for the proposed development.

- (3) All plats shall comply with the standards of Chapter 14, Article 9 (Infrastructure Design, Improvements and Dedication Standards).*

The proposed subdivision plat conforms with all applicable standards of SFCC §14-9.

- (4) A plat shall not be approved that creates a nonconformity or increases the extent or degree of an existing nonconformity with the provisions of Chapter 14 unless a variance is approved concurrently with the plat .*

The proposed plat creates no non-conformities and no variances are requested with this application.

- (5) A plat shall not be approved that creates a nonconformity or increases the extent or degree of an existing nonconformity with applicable provisions of other chapters of the Santa Fe City Code unless an exception is approved pursuant to the procedures provided in that chapter prior to approval of the plat .*

The proposed subdivision plat creates no non-conformities nor increases the degree of an existing non-conformity.

Neighborhood Park

As depicted in the Los Prados Master Plan, Phase 1 includes the construction of new a 5.12-acre Neighborhood Park north of the subdivision and adjacent to the future extension of the City of Santa Fe's Acequia Trail. The effect of placing the park next to the trail corridor is an expanded amenity that will serve as an activity node along the Acequia Trail and provide enhanced community benefit as a result of its connectivity to the City's network of trails and open spaces. The park design as shown in the subdivision plans was developed in close collaboration with the City Parks Division and area residents. Homewise conducted extensive outreach last year,

including a community open house/workshop and two Early Neighborhood Notification meetings dedicated to the park design where valuable community input was provided. The resultant design delivers an appropriate mix of active and passive park elements, including walking trails, natural open space, mini-pitch soccer court, basketball court, bocce ball courts, and active and natural children's play areas. In addition, 17 off-street parking spaces are provided. Homewise will construct the park, and it will then be dedicated to the City of Santa Fe for ongoing maintenance.

The Master Plan includes a condition of approval requiring Homewise to provide a financial guaranty for construction of the Park. A cost estimate was prepared based on the preliminary park design and approved by Land Use and the Parks Division on January 19, 2024, and a bond is now in place with the City. Prior to recording the Final Subdivision Plat, an updated park estimate will be submitted based on the final design and a new financial guaranty provided for the park construction.

Affordable Housing

A total 49 affordable homes will be provided, comprising 20% Santa Fe Homes Program units and an additional 30% Low Priced Dwelling Units. Please see the attached draft SFHP Affordable Housing Agreement for more detail.

Access and Traffic

The subject property is located at the southwest corner of Rufina Street and South Meadows Road. Access to the project will be via two access points on South Meadows – the north access road providing right-in, right-out, and left-in turning movements and a south road connection with full access. According to the Traffic Impact Analysis approved as part of the Master Plan process, all intersections studied (South Meadows/Rufina, Airport/South Meadows, and the two new entry roadways) meet NMDOT operational requirements during the AM and PM Peak Hours both in the existing conditions and in the implementation year (2026). Additionally, in response to neighborhood concerns, traffic counts were conducted for roadways intersecting South Meadows from the east, and all maintain acceptable levels of service.

Right-turn deceleration lanes will be constructed in South Meadows at both access points. In addition, a median will be constructed to accommodate the northbound left-turn lane at the north entrance in order to prevent left-turns exiting the subdivision.

All internal roadways will be constructed to sub-collector standards with 50-foot rights-of-way and will be dedicated to the City as public roadways. Ample on-street parking is provided as depicted in the subdivision plans. Pedestrian connections are provided to the existing neighborhood to the west at two locations – at the northwest corner of the Park and further south from the internal sidewalk network. Furthermore, as described above, the Park includes trail connections to the future Acequia Trail Extension to enhance access to the City's network of trails and open space.

Water and Wastewater

The Project will be served by the existing 16-inch City waterline in South Meadows Rd. via two 8-inch connections at the proposed roadways. Four new fire hydrants are proposed as depicted in the attached plans. A Water Budget is submitted herewith for review and new water demand will be offset in accordance with City Code requirements.

Terrain Management

The site's terrain slopes gradually from the southeast to the northwest with an overall grade change of ± 15 feet over the 14.41-acre site. No significant drainageways or flood plains are present on the site, and no natural 30% or greater slopes will be disturbed by future development.

Under current conditions, stormwater flows to the northwest corner of the Park, which will be developed as a landscaped water harvesting area to collect the stormwater flows from the subdivision as depicted in the approved Master Plan. This "rain garden" approach was developed in collaboration with the Parks Division and is considered an opportunity to educate park visitors about the importance of water harvesting to passively irrigate plantings and recharge the water table. An easement will be established for drainage improvements, which will be dedicated to Los Prados Homeowners' Association for ongoing maintenance, per the note below from the Master Plan:

"There is an existing, natural depression at the northwest area of the site that currently collects all stormwater flows from the site. Therefore, it is the intent of the future park design to maximize passive stormwater harvesting opportunities through employing rain gardens, permaculture, and other similar low-impact techniques. Any formal drainage improvements accepting stormwater flows from Los Prados will be placed in a drainage easement dedicated to the Los Prados homeowners association who will be responsible for the maintenance of such improvements in accordance with the City's adopted drainage maintenance requirements and schedule."

Landscaping

Per the requirements of SFCC §14-8.4, street trees will be provided along all roadways within the 5-foot planting strips. All disturbed areas will be reseeded or stabilized with gravel mulch. In addition, the landscaping plans for the Park are included in the subdivision submittal plans.

Archaeology

The Project is located in both the River and Trails and the Suburban Archaeological Districts and archaeological clearance from the City of Santa Fe Archaeological Review Committee is required prior to development. In accordance with Santa Fe County Ordinance No. 1996-10, Article VI, Section 3 and State of New Mexico Historic Preservation Division guidelines and state regulations, an archaeological survey was completed at the time that Santa Fe County acquired the property in 2001 (NMCRIS No. 76184). Further, in 2010, an historical study of the

property was conducted (NMCRIS No. 116848). Prior to final plat approval, the City of Santa Fe Archaeological Review Committee will be required to review these reports and issue archaeological clearance.

Affordable Housing

A total 49 affordable homes will be provided, comprising 20% Santa Fe Homes Program units and an additional 30% Low Priced Dwelling Units. Please see the attached draft SFHP Affordable Housing Agreement for more detail.

Early Neighborhood Notification

Early Neighborhood Notification (ENN) meetings were conducted to discuss the park design on August 31 and September 19, 2023. Subsequently, an ENN meeting was held regarding the subdivision application on February 21, 2024, the meeting notes for which are attached.

Application Fees

The Application Fees are calculated as follows:

49 Market Rate Lots X \$150.00 =	\$7,350.00
Posters	\$ 0.00 (<i>will re-use ENN posters</i>)
TOTAL:	\$7,350.00

In support of this request, the following documentation is submitted herewith for your review:

- | | |
|---|--|
| 1. Subdivision Application | 10. Preliminary Water Budget |
| 2. Letter of Authorization | 11. Terrain Management Report |
| 3. Property Deed 2022 | 12. Geotechnical Report |
| 4. Lot of Record: Lot Split + Park
Dedication Plat | 13. Utility Service Application |
| 5. Approved Master Plan | 14. Specifications Stabilized Gravel |
| 6. ENN Meeting Notes | 15. Preliminary Subdivision Plat
Revisions |
| 7. ENN Participants | 16. Conditions of Approval/ Corrections
Spreadsheet |
| 8. Approved Phasing Plan | |
| 9. Draft SFHP Agreement | |

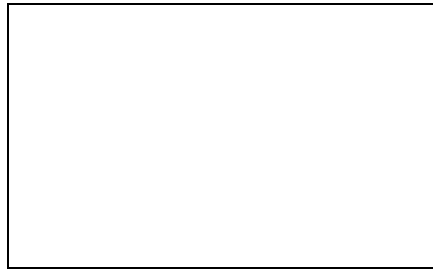
Please let me know if you have any questions or need additional information.

Thank you for your consideration.

Sincerely,



Jennifer Jenkins
JenkinsGavin, Inc.



SUBDIVISION APPLICATION

Parcel Information

Project Name: Los Prados

Address: 3730 South Meadows Rd Property Size: 14.41 acres

Current Use of Land: Vacant Proposed Use of Land: Subdivision

Does a Rezoning application accompany this application? YES ☐ NO ☒ Are any variances required? YES ☐ NO ☒

Preapplication Conference Date: 5/25/2023

Early Neighborhood Notice (ENN) meeting date: 2/21/2024 Zoning: R-6

Check One: Preliminary Plat ☒ Final Plat ☐ UPC Number: 1048096450107000000

Property Owner Information

Company Name: Homewise, Inc.

Name: _____

Address: 1301 Siler Rd Bldg D

Street Address Suite/Unit #

Santa Fe NM 87507

City State ZIP Code

Phone: () E-mail Address: _____

Applicant/Agent Information (if different from owner)

Company Name: JenkinsGavin Inc.

Name: Jennifer Jenkins

First Last

Address: 130 Grant Ave 101

Street Address Suite/Unit #

Santa Fe NM 87501

City State ZIP Code

Phone: (505) 820-7444 E-mail Address: jennifer@jenkinsgavin.com

Correspondence Directed to: ☐ Owner ☒ Applicant ☐ Both

Agent Authorization (if applicable)

I am the owner and record title holder of the property located at: << See authorization form >>

I authorize _____ to act as my agent to execute this application.

Signed: _____ Date: _____

Submittal Checklist (Requirements found in Section 14-3.7 SFCC 1987)

☐ Six (6) 24"x36" plan sets and one CD containing the same information as the paper set (PDF) are required. Please include the following:

☐ Letter of Application (intent, location, acreage)	☐ Legal Lot of Record, Legal Description	☐ Preliminary Plat or Final Plat	☐ Landscape, Parking and Lighting Plan, Signage Specifications	☐ Terrain Management Plans (as required by Section 14-8.2 SFCC 2001)
☐ Traffic Impact Analysis (if required)	☐ Sewer and Water Plan (Profiles and details – Final Plat only)	☐ Phasing Plan (if applicable)	☐ Archaeological Clearance (if applicable)	☐

Subdivisions per Section 14-3.7 SFCC 1987:

Preliminary Plat (Section 14-3.7(B)(3) SFCC 1987)

See above for submittal requirements. See also Section 14-9 Infrastructure Design, Improvement and Dedication Standards.

Final Plat (Section 14-3.7(B)(4) SFCC 1987)

The final plat shall conform substantially to the preliminary plat as approved.

Signature

I hereby certify that the documents submitted for review and consideration by the City of Santa Fe have been prepared to meet the minimum standards outlined in the Land Development Code, Chapter 14 SFCC 1987. Failure to meet these standards may result in the rejection of my application. I also certify that I have met with the City's Current Planning staff in a preapplication meeting to verify that the attached proposal is in compliance with the City's zoning and development plan requirements.

Signature: Jennifer Jenkins Date: 3.11.24

A case manager will be assigned to your project and will notify you within 10 business days if any additional information is needed. After your application has been reviewed by City staff, you will be contacted by us regarding public notice requirements. A packet of information and instructions will be provided regarding the required mailing and sign posting. Please contact the Land Use Department staff at (505) 955-6585 with any questions.

June 22, 2021

City of Santa Fe
Land Use Department
200 Lincoln Avenue
Santa Fe, NM 87501

RE: **Homewise – South Meadows & Rufina**

To Whom It May Concern:

This letter shall serve as authorization for JenkinsGavin, Inc. to act on our behalf with respect to the referenced property regarding land use applications to be submitted to the City of Santa Fe.

Thank you.

Sincerely,

Homewise, Inc.

By: Jaime Saramillo

Jaime Saramillo

Print Name

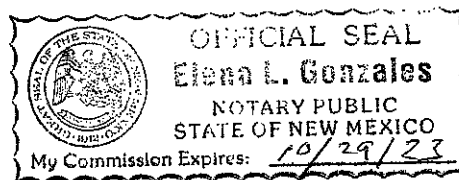
6/22/21

Date

State of New Mexico)

) ss

County of Bernalillo)



The foregoing instrument was acknowledged before me this 22nd day of June, 2021 by Jaime Saramillo.

Elena L. Gonzales
Notary Public



Return to First American Title Insurance Company
File No. 2614116-SF01 jlo

QUITCLAIM DEED

County of Santa Fe, a political subdivision of the State of New Mexico, for consideration paid, quitclaims to Homewise, Inc., a New Mexico non-profit corporation, whose address is 1301 Siler Rd., Bldg. D, Santa Fe, New Mexico 87507, the following described real estate in Santa Fe County New Mexico:

Blocks 3 and 4, of South Meadows Lot Split, as shown and delineated a the plat thereof, filed January 29, 1985, recorded in Plat Book 148, Page 040, as Document No. 560,068; and as further shown "Boundary Survey Prepared for Paul & Joanne Peppard Peppard Family Trust of Blocks 3 & 4 as shown on Plat Recorded in Plat Book 402, Page 14, Within Section 1, T.16N., R.8E., and Section 6, T.16N., R.9E., Santa Fe County, New Mexico", filed July 12, 2001, recorded in Plat Book 478, Page 32, as Document No. 1163,822, in the records of Santa Fe County, New Mexico.

WITNESS our hands and seal this 29th day of April, 2022.

County of Santa Fe, a political subdivision of the
State of New Mexico


By: Katherine Miller, Santa Fe County Manager

Representative Capacity

State of New Mexico)
County of Santa Fe) ss

This instrument was acknowledged before me on April 29, 2022, by Katherine Miller as Santa Fe County Manager of the County of Santa Fe, a political subdivision of the State of New Mexico, on behalf thereof.

My commission expires:

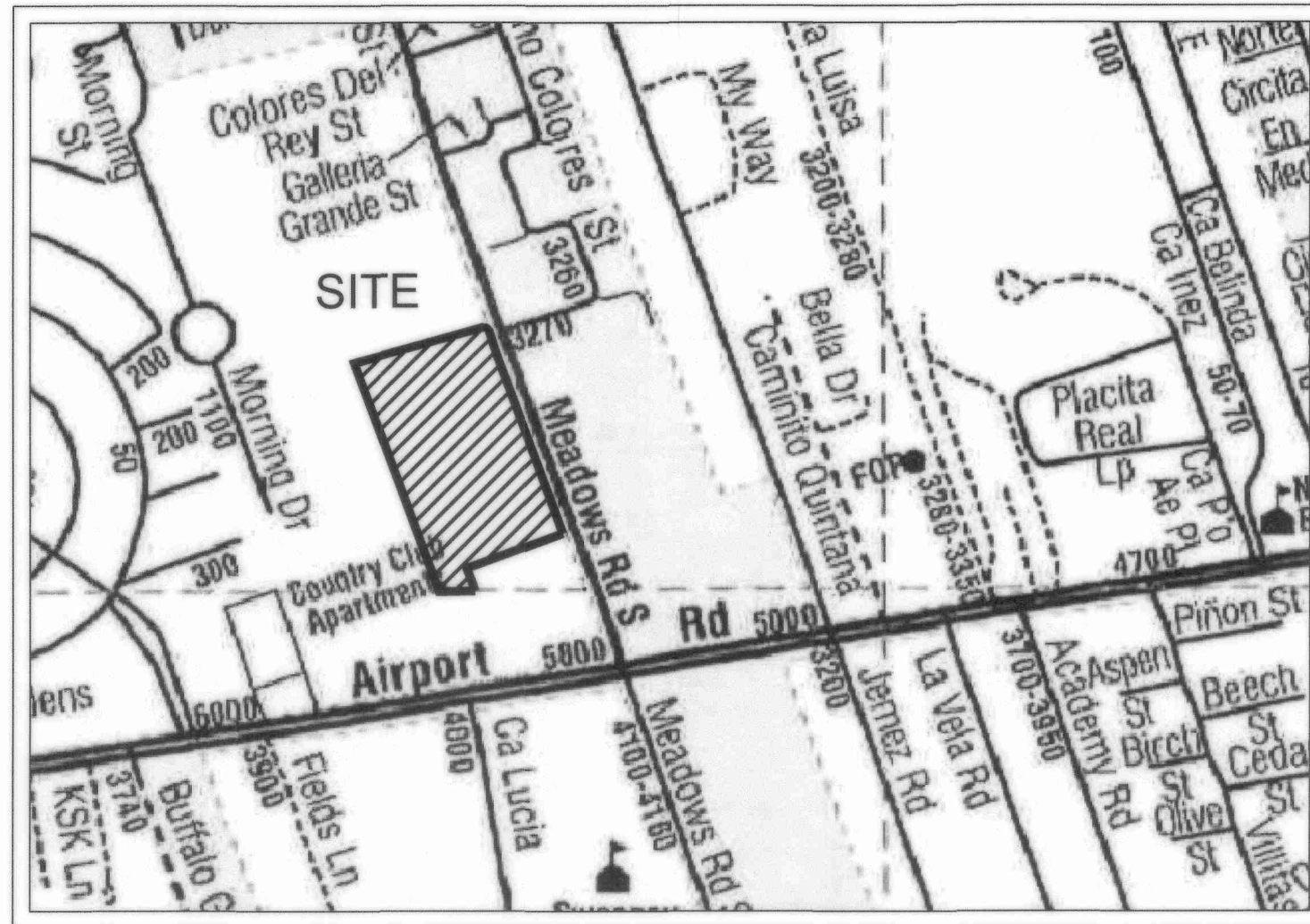
Notary Public



(Seal)

STATE OF NEW MEXICO
NOTARY PUBLIC
JAMES P. LUCERO
COMMISSION #1013504
COMMISSION EXPIRES 08/21/2022

Recorded 1987215 04/29/22 SFC



VICINITY MAP (NTS)

ADJOINER INFORMATION

LETTER	OWNER	DOC#-BK/PG
A	SANTA FE COUNTY	
B	MORRISON	1763014
C	WUNNICKE	1292/707A
D	ALDERETE	1024/371
E	WINN	1779532
F	TORREZ	698/877
G	RUCKEL	2121/401
H	MOUTON	638/554
I	VRABEL	714/185
J	MENIKOTOS	1887191
K	MONTOYA	1919071
L	RABI REV. TRUST	1518940
M	CITY OF SANTA FE	1310905
N	MICHAEL	1742279
O	MARYOL	1312187
P	OKEEFE	953/522
Q	LOPEZ	1785159
R	DEWITT	1048/350
S	ENGLERT	1851668
T	SHERRILL	1893963
U	PACHECO	1821934

PLAT REFERENCE

PLAT REFERENCE#1

"BOUNDARY SURVEY PREPARED FOR PAUL & JOANNE PEPPARD" BY GARY E. DAWSON NMPLS#7014 AND FILED IN PLAT BOOK 478, PG. 32 IN THE OFFICE OF THE SANTA FE COUNTY CLERK RECORD DATA SHOWN IN ().

PLAT REFERENCE#2

"BOUNDARY SURVEY PREPARED FOR PAUL & JOANNE PEPPARD" BY GARY E. DAWSON NMPLS#7014 AND FILED IN PLAT BOOK 402, PG. 14 IN THE OFFICE OF THE SANTA FE COUNTY CLERK

PLAT REFERENCE#3

"REVISED BOUNDARY SURVEY PREPARED FOR PAUL & JOANNE PEPPARD" BY GARY E. DAWSON NMPLS#7014 AND FILED IN PLAT BOOK 296, PG. 14 IN THE OFFICE OF THE SANTA FE COUNTY CLERK RECORD DATA SHOWN IN ().

PLAT REFERENCE#4

"SOUTH MEADOWS LOT SPLIT" BY GARY E. DAWSON NMPLS#7014 AND FILED IN PLAT BOOK 148, PG. 40 IN THE OFFICE OF THE SANTA FE COUNTY CLERK

PLAT REFERENCE#5

"CITY OF SANTA FE RUFINA STREET ROW PROJECT PHASE II C.I.P. NO.826" BY ALARID & ASSOCIATES, INC. NMPLS#5338 AND NOT RECORDED. RECORD DATA SHOWN IN ().

PLAT REFERENCE#6

"AMENDED PLAT OF THE JIMENEZ SUBDIVISION TRACT C & D" BY MORRIS A. APODACA NMPLS#5300 AND FILED IN PLAT BOOK 146, PG. 23 IN THE OFFICE OF THE SANTA FE COUNTY CLERK

SURVEYORS CERTIFICATE

I HEREBY CERTIFY THAT THIS PLAT AND THE NOTES HEREON ARE AN ACCURATE DELINEATION OF A FIELD SURVEY COMPLETED BY ME OR UNDER MY DIRECTION ON JUNE 20TH, 2023, AND ARE TRUE AND CORRECT TO THE BEST OF MY KNOWLEDGE AND BELIEF, AND MEET THE MINIMUM STANDARDS FOR PROFESSIONAL LAND SURVEYORS PRACTICING IN NEW MEXICO.

RICHARD A. CHATROOP

11/07/23
N.M.P.L.S.#11011

DEDICATION AND AFFIDAVIT

KNOW ALL MEN BY THESE PRESENTS THAT THE UNDERSIGNED OWNER(S), HAVE CAUSED TO BE DIVIDED AND DEDICATED THOSE LANDS SHOWN HEREON THIS DIVISION AND DEDICATION IS MADE WITH THE FREE CONSENT AND IN ACCORDANCE WITH THE WISHES AND DESIRES OF SAID OWNER(S).

THE AREA DESIGNATED AS CITY OF SANTA FE PARK (5.12 AC.±) IS HEREBY DEDICATED TO THE CITY OF SANTA FE FEE SIMPLE.

THIS DIVISION AND DEDICATION CONTAINS 19.53 AC.±, AND LIES WITHIN THE PLANNING AND PLATTING JURISDICTION OF THE CITY OF SANTA FE, NEW MEXICO.

EASEMENTS ARE VACATED AS SHOWN.

by Michael D. Loftin
MICHAEL D. LOFTIN, EXECUTIVE DIRECTOR HOMEWIS INC.

STATE OF NEW MEXICO

COUNTY OF SANTA FE

THE FOREGOING INSTRUMENT WAS SWORN, ACKNOWLEDGED AND

SUBSCRIBED BEFORE ME BY

MICHAEL D. LOFTIN, EXECUTIVE DIRECTOR HOMEWIS INC.

THIS 1 DAY OF November, 2023 Samie SaramilloMY COMMISSION EXPIRES 7/14/2024 NOTARY PUBLIC

STATE OF NEW MEXICO
NOTARY PUBLIC
NORMA L. CAMPOS
COMMISSION # 1129045
COMMISSION EXPIRES 07/09/2024

VACATION OF EASEMENT OF WAY

KNOW ALL MEN BY THESE PRESENTS THAT THE UNDERSIGNED OWNER(S), HAVE CAUSED TO BE VACATED A CERTAIN EASEMENT OF WAY FOR A 15' WIDE MAINTENANCE EASEMENT FOR SEWER INTERCEPTOR LINES GRANTED BY DEED FOR EASEMENT OF WAY IN BOOK 175 MISC., PAGE 497, AS RECORDED IN THE OFFICE OF THE SANTA FE COUNTY CLERK ON FEB. 20TH 1961. EASEMENT VACATED ONLY WITHIN THE LIMITS OF TRACTS 3, 4A & 4B SHOWN HEREON.

THIS VACATION OF EASEMENT LIES WITHIN THE PLANNING AND PLATTING JURISDICTION OF THE CITY OF SANTA FE, NEW MEXICO.

PUBLIC UTILITY EASEMENTS

PUBLIC UTILITY EASEMENTS shown on this plat are granted for the common and joint use of:

A. Public Service Company of New Mexico

corporation, (PNM Electric) for installation, maintenance, and service of overhead and underground electrical lines, transformers, and other equipment and related facilities reasonably necessary to provide electrical services.

B. New Mexico Gas Company for installation, maintenance, and service of natural gas lines, valves and other equipment and facilities reasonably necessary to provide natural gas services.

C. Qwest Corporation d/b/a CenturyLink QC for the installation, maintenance, and service of such lines, cable, and other related equipment and facilities reasonably necessary to provide ground communication services, including but not limited to above ground pedestals and closures. This plat has been approved for easement purposes only. The signing of this plat does not in any way guarantee telephone services to the subdivision.

D. Cable TV for the installation, maintenance, and service of such lines, cable, and other related equipment and facilities reasonably necessary to provide Cable services.

Included, is the right to build, rebuild, construct, reconstruct, locate, relocate, change, remove, replace, modify, renew, operate and maintain facilities for purposes described above, together with free access to, from, and over said easements, with the right and privilege of going upon, over and across adjoining lands of Grantor for the purposes set forth herein and with the right to utilize the right of way and easement to extend services to customers of Grantee, including sufficient working area space for electric transformers, with the right and privilege to trim and remove trees, shrubs or bushes which interfere with the purposes set forth herein. No building, sign, pool (aboveground or subsurface), hot tub, concrete or wood pool decking, or other structure shall be erected or constructed on said easements, nor shall any well be drilled or operated thereon. Property owners shall be solely responsible for correcting any violations of National Electrical Safety Code by construction of pools, decking, or any structures adjacent to or near easements shown on this plat.

Easements for electric transformer/switchgears, as installed, shall extend ten (10) feet in front of transformer/switchgear doors and five (5) feet on each side.

Disclaimer

In approving this plat, Public Service Company of New Mexico (PNM), New Mexico Gas Company (NMGC) and Qwest Corporation d/b/a CenturyLink (QWEST) did not conduct a Title Search of the properties shown hereon. Consequently, PNM, NMGC and QWEST do not waive or release any easement or easement rights which may have been granted by prior plat, replat or other document and which are not shown on this plat.

The utility companies listed below hereby release, waive, quitclaim and discharge their right, title and interest to the present owner(s), and their assigns, in the Public Utility Easement as shown to be vacated on this plat. As to all property covered by such easement, which is not specifically described herein, the right, title, and interest of the following companies shall not be affected by this release.

UTILITIES APPROVAL

Frank A. Nguyen 10/24/2023
NEW MEXICO GAS COMPANY DATE
Chantelle Duenas-Gonzalez 10/30/23
PNM ELECTRIC SERVICES DATE
Dan Dole 10-24-2023
QWEST CORP d/b/a CENTURYLINK QC DATE
SPR 10-31-23
COMCAST DATE

COUNTY OF SANTA FE)
STATE OF NEW MEXICO)
I hereby certify that this instrument was filed
for record on the 16 day of November A.D.
2023 at 10:31 o'clock
and was duly recorded in book 11011 of the records of
page 15 of the County of Santa Fe County.

Witness my Hand and Seal of Office
Katharine Clark
County Clerk, Santa Fe County, N.M.

Deputy



COUNTY OF SANTA FE REVIEW

Nov 6, 2023
COUNTY TREASURER DATE

CITY OF SANTA FE APPROVAL,

NOTES AND CONDITIONS:

11/13/23
CITY ENGINEER FOR LAND USE DATE
11/16/2023
CITY PLANNING DATE

NOTES

- PROPERTY DEVELOPMENT IS REQUIRED TO COMPLY WITH APPLICABLE PROVISIONS OF CHAPTER 14, LAND DEVELOPMENT CODE, SFCC 1987 AND SUBSEQUENT AMENDMENTS.
- WASTEWATER UTILITY EXPANSION CHARGES (UEC) SHALL BE PAID AT THE TIME OF CONSTRUCTION PERMIT APPLICATION.
- NEW DEVELOPMENT SHALL HAVE WATER SUPPLY THAT MEETS FIRE FLOW REQUIREMENTS AS PER IFC OR INSTALL AN AUTOMATIC SPRINKLER SYSTEM.
- FIRE DEPARTMENT SHALL HAVE 150 FEET DISTANCE TO ANY PORTION OF THE BUILDING ON ANY NEW CONSTRUCTION.
- PRIOR TO ANY NEW CONSTRUCTION OR REMODELING ALL FIRE DEPARTMENT REQUIREMENTS MUST BE MET OR AUTOMATIC SPRINKLER SYSTEMS MAY BE REQUIRED.
- TRACT 4 LIES WITHIN ZONE "X" AREAS DETERMINED TO BE OUTSIDE OF THE 0.2% ANNUAL CHANCE FLOODPLAIN AS PER F.I.R.M. PANEL# 35049C0393D DATED 6/17/08.
- CITY APPROVAL OF THIS PLAT DOES NOT ALTER OR REMOVE ANY EASEMENTS, RESTRICTIONS, OR RESERVATIONS OF RECORD PERTAINING TO THE LAND HEREIN DESCRIBED.
- PARCELS 4-A AND 4-B ARE BOUND BY THE MASTER PLAN CONDITIONS AS OUTLINED IN LAND USE CASE #2022-5618, TITLED "3600 & 3740 SOUTH MEADOWS MASTER PLAN". THESE CONDITIONS WERE PROPOSED BY THE PLANNING COMMISSION TO THE GOVERNING BODY ON NOVEMBER 3, 2022 AND SUBSEQUENTLY ADOPTED AS CONDITIONS OF APPROVAL BY THE GOVERNING BODY ON FEBRUARY 1, 2023.
- ANY FORMAL DRAINAGE IMPROVEMENTS ACCEPTING STORMWATER FLOWS FROM THE FUTURE DEVELOPMENT OF TRACT 4-A WILL BE PLACED IN A DRAINAGE EASEMENT DEDICATED TO THE TRACT 4-A HOMEOWNERS ASSOCIATION WHO WILL BE RESPONSIBLE FOR THE MAINTENANCE OF SUCH IMPROVEMENTS IN ACCORDANCE WITH THE CITY'S ADOPTED DRAINAGE MAINTENANCE REQUIREMENTS AND SCHEDULE.

LEGEND AND NOTES

- DENOTES ELEC. TRANSFORMER
- DENOTES SANITARY SEWER MANHOLE
- DENOTES STORM SEWER MANHOLE
- ▲ DENOTES WATER MARKER
- △ DENOTES FIRE HYDRANT
- △ DENOTES GAS TEST
- △ DENOTES TEL./COM. RISER

- DENOTES POINT FOUND (AS NOTED)
- DENOTES MONUMENT FOUND

- DENOTES POINT SET THIS SURVEY
- DENOTES POINT CALCULATED

DENOTES EDGE OF EASEMENT

X X DENOTES FENCE LINE

1. BASIS OF BEARING IS FROM GSP OBSERVATION WGS84

LOT OF RECORD IS FROM
"LOT LINE ADJUSTMENT FOR HOMEWIS, TRACTS 3 AND 4"
BY RICHARD A. CHATROOP NMPLS#11011 AND FILED IN PLAT BOOK
898, PG.37, INSTR.#2004713 IN THE OFFICE OF THE SANTA FE
COUNTY CLERK.

2. THIS PLAT IS SUBJECT TO ANY EASEMENTS, RESTRICTIONS
AND COVENANTS OF RECORD.

PURPOSE STATEMENT: THE PURPOSE OF THIS
PLAT IS TO SUBDIVIDE TRACT 4 INTO 2 LOTS

LOT SPLIT
FOR
HOMEWIS, INC.
OF
TRACT 4

UPC# 1-048-096-450-107-TRACT 4

LYING WITHIN SECTION 1, T16N, R8E, NMPM,
CITY & COUNTY OF SANTA FE, NEW MEXICO.

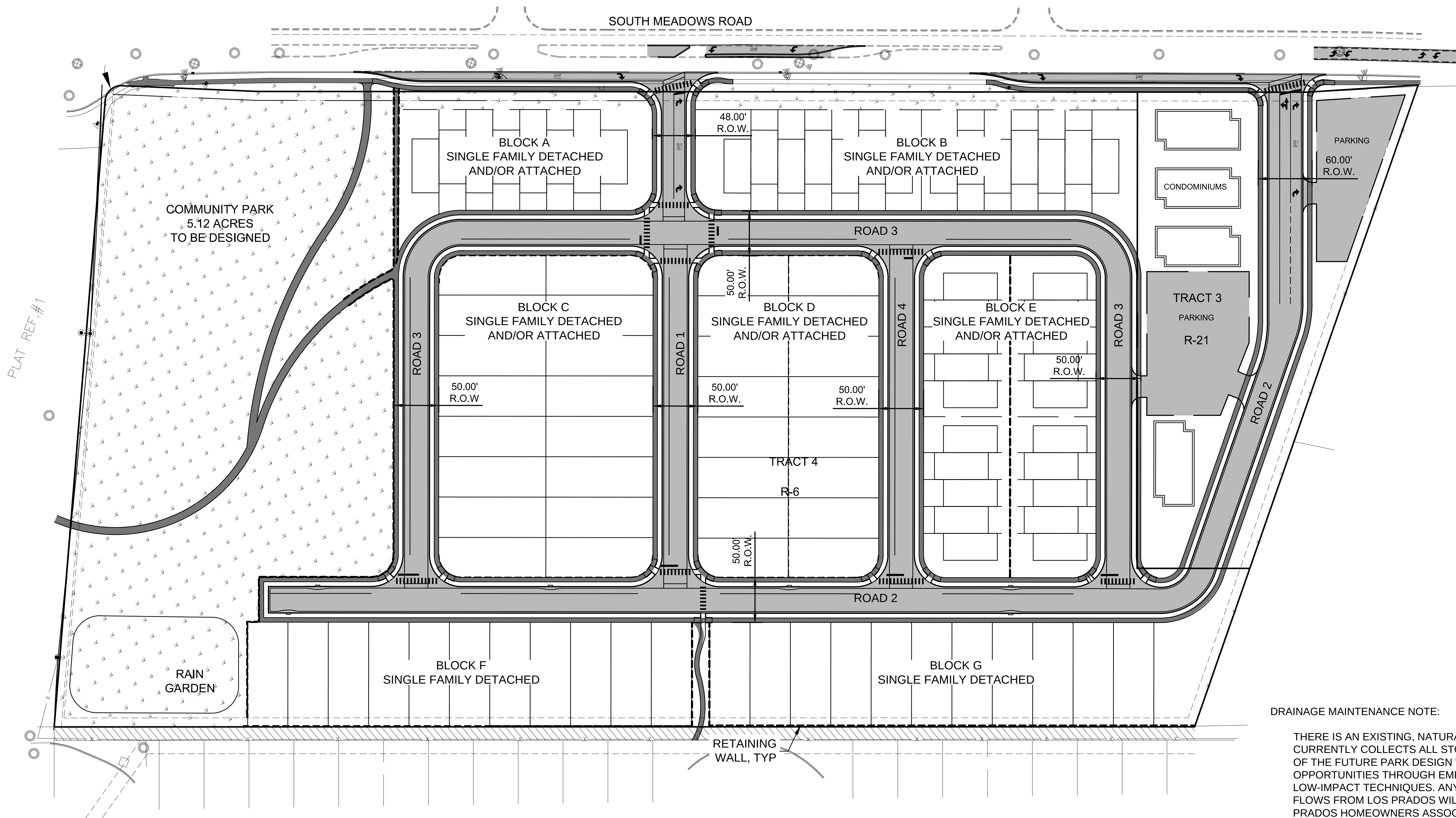
RICK CHATROOP
PROFESSIONAL LAND SURVEYOR

NEW MEXICO REGISTRATION NO. 11011
(505) 470-0037 110 WAGON TRAIL RD. CERRILLOS, NM. 87010

INDEXING INFORMATION FOR THE COUNTY CLERK

OWNER: HOMEWIS, INC.

LOCATION: LYING WITHIN SECTION 1, T16N, R8E, NMPM,
CITY & COUNTY OF SANTA FE, NEW MEXICO.



SITE DATA	
TRACT 4	
TOTAL AREA	19.54 ACRES
PROPOSED ZONING	R-6, 6 DWELLINGS/ACRE
COMMUNITY PARK	5.12 ACRES
MAXIMUM PERMISSIBLE DWELLINGS:	97 UNITS
TRACT 3	
TOTAL AREA	2.66 ACRES
PROPOSED ZONING	R-21, 21 DWELLINGS PER ACRE
MAXIMUM PERMISSIBLE DWELLINGS:	64 UNITS (INCLUSIVE OF SFHP 15% DENSITY BONUS)

DRAINAGE MAINTENANCE NOTE:

THERE IS AN EXISTING, NATURAL DEPRESSION AT THE NORTHWEST AREA OF THE SITE THAT CURRENTLY COLLECTS ALL STORMWATER FLOWS FROM THE SITE. THEREFORE, IT IS THE INTENT OF THE FUTURE PARK DESIGN TO MAXIMIZE PASSIVE STORMWATER HARVESTING OPPORTUNITIES THROUGH EMPLOYING RAIN GARDENS, PERMACULTURE, AND OTHER SIMILAR LOW-IMPACT TECHNIQUES. ANY FORMAL DRAINAGE IMPROVEMENTS ACCEPTING STORMWATER FLOWS FROM LOS PRADOS WILL BE PLACED IN A DRAINAGE EASEMENT DEDICATED TO THE LOS PRADOS HOMEOWNERS ASSOCIATION WHO WILL BE RESPONSIBLE FOR THE MAINTENANCE OF SUCH IMPROVEMENTS IN ACCORDANCE WITH THE CITY'S ADOPTED DRAINAGE MAINTENANCE REQUIREMENTS AND SCHEDULE.

GENERAL NOTES

1. PROPERTY WILL BE SUBJECT TO THE WATER ALLOCATION AND/OR WATER OFFSET RETROFIT PROVISIONS OF ORDINANCE NO. 2002-29 AND RESOLUTION 2002-55 AT THE TIME OF PERMIT APPLICATION OR WATER HOOKUP REQUEST. COMPLIANCE SHALL BE ACHIEVED BY USE OF RETROFIT CREDITS OR WATER TRANSFERS, IF APPLICABLE.

2. COMPLIANCE WITH PROVISIONS OF GUNNISON'S PRAIRIE DOG ORDINANCE SHALL BE COMPLETED PRIOR TO GRADING OPERATIONS.

3. AMERICANS WITH DISABILITIES ACT (ADA) INSPECTION SHALL BE CONDUCTED PRIOR TO ISSUANCE OF A CERTIFICATE OF OCCUPANCY. THE CONTRACTOR SHALL CONTACT CITY STAFF TO SCHEDULE AN INSPECTION.

4. FENCES, WALLS, OR OTHER OBSTRUCTIONS SHALL NOT BE PLACED OR CONSTRUCTED ACROSS PUBLIC SANITARY SEWER EASEMENTS.

5. UTILITY EXPANSION CHARGES (UEC) SHALL BE PAID AT THE TIME OF BUILDING PERMIT APPLICATION FOR EACH LOT. CONTACT THE CITY OF SANTA FE PERMIT AND DEVELOPMENT REVIEW DIVISION TO PAY THESE CHARGES.

6. ALL PROPOSED CHANGES TO THE APPROVED IMPROVEMENT PLANS SHALL RECEIVE PRIOR APPROVAL BY THE CITY OF SANTA FE.

7. COMPLY WITH THE CURRENT COSF WATER CONSERVATION STAGE.

8. LANDSCAPE DESIGN SHALL COMPLY WITH APPLICABLE PROVISIONS OF SFCC SECTION 14-8.4.

9. PARK DESIGN SHALL BE COORDINATED WITH THE PARKS DIVISION, AND THE PARK SHALL BE DEDICATED TO AND MAINTAINED BY THE CITY OF SANTA FE. THE PARK SHALL BE CONSTRUCTED BY THE DEVELOPER AS PART OF PHASE 1.

10. PROPERTY DEVELOPMENT IS SUBJECT TO THE PROVISIONS OF SECTION 14-8.4(f)(5), "PLANT MATERIAL STANDARDS," WHEREIN PRESERVATION OF SIGNIFICANT TREES IS REQUIRED.

11. PROPERTY DEVELOPMENT IS REQUIRED TO COMPLY WITH THE PROVISIONS OF COSF ORDINANCE NO. 2008-02 (IMPACT FEES).

12. PROPERTY DEVELOPMENT, BOTH PUBLIC AND PRIVATE OWNERSHIP, SHALL COMPLY WITH THE COSF ORDINANCE NO. 2002-20 (TERRAIN AND STORMWATER MANAGEMENT) AT THE TIME OF BUILDING PERMIT APPLICATION.

13. PROPERTY DEVELOPMENT IS REQUIRED TO COMPLY WITH APPLICABLE PROVISIONS OF CHAPTER 14, LAND DEVELOPMENT CODE, SFCC 1987 AND SUBSEQUENT AMENDMENTS.
14. PROPERTY DEVELOPMENT IS REQUIRED TO COMPLY WITH THE PROVISIONS OF EACH APPLICABLE CITY OF SANTA FE ORDINANCE ADOPTED PRIOR TO SUBMITTAL WITH BUILDING PERMIT APPLICATION THAT MODIFIES ANY PROVISIONS OF REQUIREMENTS CALLED FOR IN CHAPTER 14, LAND DEVELOPMENT CODE, SFCC 1987 AND SUBSEQUENT AMENDMENTS.

15. FIRE DEPARTMENT ACCESS SHALL BE MAINTAINED THROUGHOUT ALL DEVELOPMENT CONSTRUCTION PHASES AS PER IFC 1410.1.

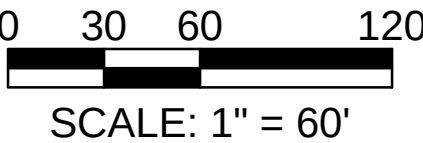
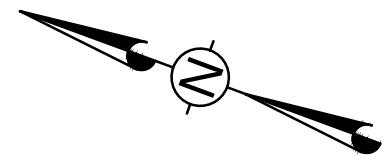
16. AN APPROVED WATER SUPPLY FOR FIRE PROTECTION, EITHER TEMPORARY OR PERMANENT, SHALL BE MADE AVAILABLE AS SOON AS COMBUSTIBLE MATERIAL ARRIVES ON THE SITE AS PER IFC 1412.1.

17. PERMANENT TRAFFIC CONTROL DEVICES SHALL BE INSTALLED PER APPROVED PLAN BY THE DEVELOPER.

18. THIS DEVELOPMENT LIES OUTSIDE OF A CITY OF SANTA FE HISTORIC DISTRICT.

19. THE LAYOUT SHOWN HEREON IS CONCEPTUAL AND MAY BE MODIFIED AS PART OF THE SUBDIVISION PLATTING AND/OR DEVELOPMENT PLAN PROCESS WITHOUT THE NEED FOR MASTER PLAN AMENDMENT.

20. PROPERTY DEVELOPMENT IS ANTICIPATED TO TAKE PLACE IN THREE PHASES, PER THE APPROVED PHASING PLAN. PHASING MAY BE MODIFIED ADMINISTRATIVELY BY THE CITY OF SANTA FE LAND USE DEPARTMENT WITHOUT THE NEED FOR A MASTER PLAN AMENDMENT.



MASTER PLAN
SCALE: 1" = 50'

Santa Fe Engineering Consultants, LLC 1599 St. Francis Drive, Suite B Santa Fe, N. M. 87505 (505) 982-2845 Fax (505) 982-2641 http://www.SFENGR.com	MASTER PLAN FOR LOS PRADOS 3600 AND 3740 SOUTH MEADOWS RD	
	LOS PRADOS MASTER PLAN	
	DATE: AUGUST 2022	SCALE: 1" = 60' SHEET: 3-1



JENKINSGAVIN
LAND USE | PROJECT MANAGEMENT

**LOS PRADOS PHASE ONE SUBDIVISION
EARLY NEIGHBORHOOD NOTIFICATION MEETING NOTES**

Date: February 21, 2024
Time: 5:30 to 7:30 pm
Location: Virtual Meeting held via Zoom
Attendees: City Land Use Department Representative (Margaret Moore, Planning Manager)
(Janice Biletnikoff, Senior Planner)
JenkinsGavin, Inc. (Jennifer Jenkins) (Angelica Reed)
Homewise, Inc. (Lisa Huval) (Jamie Jaramillo)
Santa Fe Engineering (Eric Cornelius)
Sites SW (George Radnovich) (Norma Oder)
33± Attendees

A presentation was given by Jennifer Jenkins of JenkinsGavin, Inc. regarding upcoming Development Plan and/or Master Plan and Rezone applications to the City of Santa Fe for the property located at 3730 South Meadows Road. This meeting was recorded by JenkinsGavin Inc. with participant's approval.

Following the presentation, a Question & Answer session was held. The following notes capture the questions and concerns raised and the responses by the Project Team.

Questions/Comments	Responses
Please address the pond area in the Park.	Jennifer Jenkins: We've had meetings and design reviews with Park Staff and our Engineer has been able to drill down more on the design. The southwest corner of the site is the low point so all the storm water today moves to this location. In working with the City we found this was a great opportunity for water harvesting, in terms of capturing the water, planting this area, passively irrigating these plantings and it's a great educational opportunity. We are looking into some signage to talk about the importance of water harvesting.
Concerned about the ponding not being safe for kids and the runoff and contamination that will occur from the subdivision into the pond.	Jennifer Jenkins: This is being designed in a way where you would not be seeing standing water. It will be very shallow and planted as a natural vegetated area. George Radnovich: The water harvesting area will be maintained by the Los Prados HOA. It

	is being designed in a way where the planting will serve as a natural filtration. It will have an irrigation system that will be used to irrigate the shrubs and the grasses and trees that will be in and around that pond. The plant will take up the contaminants and the nitrates from the subdivision. Will be harvesting water from within the park itself. The water harvesting area will also be useful for recreational activities. Parks Division: we see this as an amenity. It's meant to educate about contaminants in our system. We are doing this all along the river, trying to educate people on how to properly handle storm water. This is safer than having it just run across the street.
I live in Tiempo Lindo, and the back yard is facing the development. I have a wooden fence. How much easement will be between my fence and your land? Is there a plan to put up and fencing?	Jennifer Jenkins: There is an existing 15 foot easement from our development to Tiempo Lindo. Also, the backyards that will face your property will be fenced.
Is there any other drainage out of this subdivision? Are there storm drains or sewer lines? Will it all go down to the pond at the park?	Eric Cornelius: We will have a series of storm drains throughout the subdivision to collect storm water as it runs down the street, the reason for this is so we can get it off the street and prevent any flooding. Once we take it off the street we will pipe it underground with a storm/ sewer network and that will drain to the water catchment area and that's how it will be treated. In regards to the sewer system, we do not dump any of the storm water into the sewer system, we keep those separate. This goes into the piping into the City's network and that makes it to the City's Waste Water Treatment Plant.
What is the square footage for the Affordable Housing?	Jennifer Jenkins: We have not yet confirmed what our Affordable Housing program will look like in regards to various home types, I was just showing an example of what it could look like. They will likely be traditionally sized, ranging from single family 1,200- 1,800 SF and of course the townhomes will be a little smaller.
The single family homes that bump up against Tiempo Lindo will they be one story? We are	Jennifer Jenkins: We have not gotten to the point of identifying what homes will go where,

asking that no two story homes be placed there.	but this will be noted.
I live in Tiempo Lindo and it was originally an affordable housing neighborhood, and I can see where corners were cut. I want to advocate for affordable housing to make sure no corners are cut and everything is done up to code and not just as cheap as possible.	
What will the Affordable Housing look like? Dessert Sage was supposed to be 100% affordable, like this one, and it doesn't seem to be the case.	Jennifer Jenkins: We will be entering into an Affordable Housing Agreement with the City of Santa Fe, which is a contractual agreement. There is 50% which is the low price dwelling units, which is significantly subsidized pricing the other 50% is defined as below market rates. Homewise really tries to target those who don't fall within the deeply subsidized homes but still can't afford one at market rate. Lisa Huval: 50% of the homes were committed to be affordable per the City of Santa Fe's regulations, and that serves households between 50-120% AMI. The other 50% we are committed to selling below the median sales price for the City of Santa Fe so other working folks who may be above 120% AMI can still afford a home in Los Prados.
The last time I checked the lowest priced and most affordable home at Dessert Sage was over \$400,000, so when you say median, median is like \$600,000 right now, so does anything under the \$600,000 count?	Lisa Huval: There are a few elements here, yes we are committed to selling homes below the median sales price. I also want to clarify how this process works, when you look at what the sales price is for an affordable home it will be listed at the market price. We offer a subsidy to the homeowner based on their income to bring the price down to an affordable rate. And the reason we do that is so we don't lower the values of homes in the surrounding area.
In regards to the other meetings for this project will it be posted just like this one?	Jennifer Jenkins: Yes, we will send out mailings and there will be the big yellow signs posted.
What is the timeline for the Acequia Trail? That land hasn't been purchased by the city yet. Will there be meeting to discuss this as well?	Jennifer Jenkins: We have been engaging very closely with the City regarding the Acequia Trail and its status, the have hired a design team and are getting the contract finalized. Part of those efforts will be securing any real estate necessary for the trail extension. There will be meeting through the city's public works department as well.

I want to look into naming the park for a college friend of mine that passed away, is that time bound to get that dedication done.	Jennifer Jenkins: There is no time limit on that you can do it at any time.
Will there be dust control during construction?	Jennifer Jenkins: The City of Santa Fe has really strengthened their dust control requirements and are serious about enforcement. There will be water trucks on site and the site will be fenced with wind screens to serve as a better barrier for dust.
Can we get clarity on when the park will be built? Will the park and homes be built together or will the park be built first so we have access to it while the homes are being built?	Jennifer Jenkins: It will happen more parallel. We don't have the construction staging worked out yet. The vehicular access to the park in through our internal road way so to get to the park even for construction we have to have a way to get there. Also home construction close to the park for a safety stand point. We are still working through these details but I can tell you the park will not be complete before we start building homes. Infrastructure will happen first before we move to the park.
What's going to happen with all the fencing? Are you using existing fencing as construction fencing?	Jennifer Jenkins: We will put up construction fencing as the fence already in place is not tall enough. Construction fencing is usually 6 ft tall. The construction fencing will be up while under construction and as homes are completed the fencing will start coming down. It's sequential.
Will the park have ADA access and equipment for children with disabilities?	Norma Oder: we are ADA compliant in regards to access to the park and with the playground we have play equipment for kids with disabilities.
Concerned about the contamination testing that you said was done. Who can I email to get a copy of that report?	Jennifer Jenkins: You can email me, I will write my email in the chat.
If there is a 15ft easement why do you need to take down the original fencing? I rely on that fence for my backyard.	Eric Cornelius: We will not be touching your fence on your property as there is that 15 foot easement that we will not be touching. Your existing fence will remain in place.
Do you know how much will you be leveling off on Morning Side drive (south side)?	Eric Cornelius: the ground does vary it is a little high on the south side and low on the north side, we are going to try to normalize that and make it one continuous slope. So on the southern lots we will be cutting the ground down and on the northern we will be raising it up a little bit. However we won't be touching the ground in that 15 foot easement.

What is the qualifying process to get into these affordable homes?	Lisa Huval: Please reach out to Homewise, you can call the main office or go online. It's nuanced for each person. For affordable units there is a set price based on the household's income. There is quite a few things that go into the process so it's best to reach out to Homewise directly to guide you through the process.
What was the drilling going on last week?	Jamie Jaramillo: That was for Geotechnical soil testing. Which will test the soil for building homes and foundations. Once we get that report it will give us greater clarity on the soil we are working with. It was tested to a 5 foot depth.
Was the drilling testing for contamination?	Jamie Jaramillo: We did have a report done previously and it does have the soil tests that you are referring to and we would be happy to share that report. Jennifer Jenkins: There are two different tests for two different purposes. One is done in respect to contamination. And then the soils test for the purpose of construction, looking at the quality of the soils.
Since the plans have road ways and more traffic, would you consider picking materials for the fence that it is something that will help screen the lights?	Jennifer Jenkins: Typically in Homewise communities the fencing is opaque wood fencing. It will serve that purpose.
You tested the grounds for radiation already? Nothing was drilled the way you are drilling now. The environmental department translated the report sent to me and they confirmed it was just surface testing and no drilling.	Jamie Jaramillo: NV5 was our consultant and drilled to 5ft below the soil. They did not have the large rigs but they did drill down to 5ft. We had two different tests/reports done by NV5, the first one was topical. The second report was 5 feet of drilling and we can follow up with you and get you that report.
We have no lighting over here? What lighting will be in Los Prados	Jennifer Jenkins: We are still working through that with our design and the City of Santa Fe. We are advocating for as little as possible. We will need some lighting but we will have as little as possible. We want to create a safe environment but also protect the night sky.

Name (Original Name)

Jennifer Jenkins

BarbT

Janette Smith (janette Smith)

Maggie Moore, City of Santa Fe

Joanna Garcia

ASL Interpreter | Cynthia

Benjamin Rossi

Marlow Morrison

Janice Biletznikoff (Janice Biletznikoff)

cinthya and Carolina

Carolina Quiñones-Suarez Spanish Interpreter (Carolina Quiñc

Isaac Hammond-Paul

Lisa Huval, Homewise

Yesica earth care poder familiar (Yesica)

Cathleen

ASL Interpreter | Gabi C (Interpreter — Gabi C)

mgomez

Eric (SFEC)

Melissa McDonald, Parks & Open Space Division

Scott Overlie (Zoe Isaacson)

Heather Lamboy

Jaime Jaramillo

Johanna Gilligan

George Radnovich, Sites Southwest

Benjamin C. Rossi

Barbara's iPad

Norma Oder

Ellen Stone (169995)

Samsung SM-S901U

Marilyn Bennett

MarilynBennett

Allia Englert

BarbT

Heather Lamboy

User Email	Duration (Minutes)	Guest
jennifer@jenkinsgavin.com	99	No
	36	Yes
	96	Yes
	96	Yes
	96	Yes
	97	Yes
	96	Yes
	88	Yes
	95	Yes
	94	Yes
Jones-Suarez)	93	Yes
	92	Yes
	91	Yes
	91	Yes
	90	Yes
	91	Yes
	89	Yes
	89	Yes
	89	Yes
	87	Yes
	60	Yes
	87	Yes
	86	Yes
	53	Yes
	3	Yes
	84	Yes
	84	Yes
	82	Yes
	81	Yes
	62	Yes
	62	Yes
	62	Yes
	61	Yes
	28	Yes

Recording Consent

Yes

Yes

Yes

Yes

Yes

Yes

Yes

Yes

Yes

Yes

No

Yes

Yes

Yes

Yes

Yes

Yes

Yes

Yes

Yes

Yes

Yes

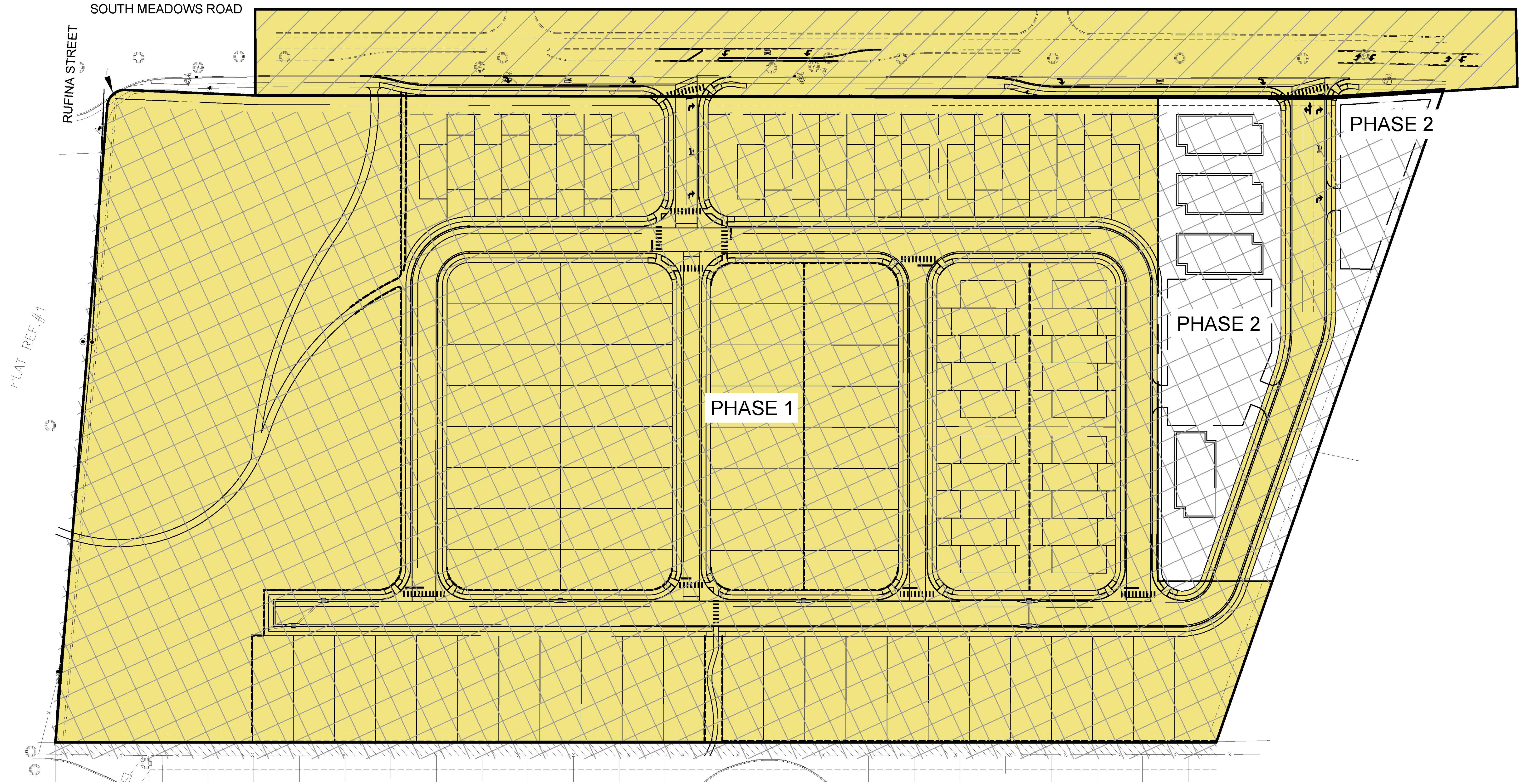
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Yes

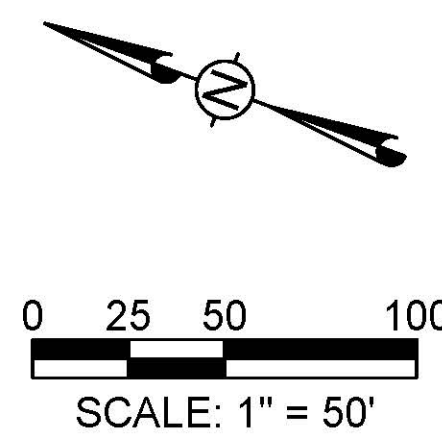
Yes

Yes

PHASE 1 - OFFSITE ROAD IMPROVEMENTS



PHASING PLAN
SCALE: 1" = 50'



THE PHASING SHOWN HEREON MAY BE MODIFIED ADMINSTRATIVELY BY THE CITY OF SANTA FE LAND USE DEPARTMENT WITHOUT THE NEED FOR A MASTER PLAN AMENDMENT.

CASE # 2021-5618
MODIFIED APRIL, 2023
PER STAFF COMMENTS

Santa Fe Engineering Consultants, LLC 1599 St. Francis Drive, Suite B Santa Fe, N. M. 87505 (505) 982-2845 Fax (505) 982-2641 http://www.SFENGR.com	MASTER PLAN SUBMITTAL FOR LOS PRADOS 3600 AND 3740 SOUTH MEADOWS RD	
	PHASING PLAN	
	DATE AUGUST 2022	SHEET 3-2

CITY OF SANTA FE
SANTA FE HOMES PROGRAM AGREEMENT
for ### SFHP Home(s) to be located at
"Los Prados Phase I", 3600 South Meadows Road, Santa Fe, New Mexico

This Santa Fe Homes Program Agreement ("Agreement") is entered into on this ____ day of _____, 2022 by and between **Homewise, Inc.** (the "SFHP Developer/Successor in Interest"), a New Mexico limited liability company, and the City of Santa Fe, New Mexico, a municipal corporation, (the "City").

WHEREAS, the City has duly adopted the Santa Fe Homes Program and Administrative Procedures (collectively "SFHP") and other regulatory structures in order to ensure that private development bears its fair share of the burden of housing affordability in the Santa Fe community; and

WHEREAS, the City's Office of Affordable Housing has determined that upon individualized assessment, the Agreement will provide, at a minimum, the amount of affordable housing necessary to offset impacts on the affordable housing market in the region of the project; and

WHEREAS, the City and the SFHP Developer/Successor in Interest have entered into a SFHP proposal ("Proposal") in accordance with the requirements of the SFHP. The Proposal is attached hereto and incorporated herein by reference; and

WHEREAS, the City and SFHP Developer/Successor in Interest now wish to satisfy the SFHP requirement for a final agreement in order to ensure SFHP compliance and a mechanism for ensuring long-term affordability.

NOW THEREFORE, the City and the SFHP Developer/Successor in Interest for themselves and their heirs, successors and assigns (including, without limitation, all persons who subsequently own a SFHP Home or any interest therein, or the Property or any portion thereof containing a SFHP Home, while this Agreement is in effect) hereby agree that the SFHP Home shall be subject to the following Agreements and restrictions for the benefit of the City.

1. Definitions. All terms capitalized herein shall have the meanings given to them in this Agreement or the meanings as defined in the Santa Fe City Code sections regarding the SFHP. In this Agreement the following words and phrases have the following meaning:

- A. "Agreement" means this SFHP Agreement.
- B. "Certificate of SFHP Homebuyer Eligibility" means a certificate issued by the City or the City's assignee or agent, which certifies that the buyer is a qualified buyer who is income eligible.
- C. "Certified Homebuyer" means a person determined, pursuant to the SFHP and such other criteria as the City or its agents may deem appropriate, to be qualified on the basis of need and preference for the purchase of a SFHP Home and who has properly obtained a "Certificate of SFHP Homebuyer Eligibility".
- D. "Fair Market Value", unless otherwise stated, means fair market value as of the day of the event in question (for example, purchase of the SFHP Home or termination of this Agreement), taking into account the restrictions on SFHP Developer/Successor in Interest and occupancy imposed by this Agreement as if such restrictions were perpetual.

E. "SFHP Developer/Successor in Interest" means each legal and equitable SFHP Developer and/or Successor in interest to the original SFHP Developer of all or any portion of a SFHP Home on the Property during the term of this Agreement, including without limitation the SFHP Developer/Successor in Interest identified above, and any subsequent SFHP Developer/Successor in Interest and/or any other Successor in Interest by sale, conveyance or other transfer of any legal or beneficial interest in a SFHP Home or the Property, as set forth in Section 2.1 herein. Unless the context otherwise requires, SFHP Developer/Successor in Interest shall mean the SFHP Developer/Successor in Interest at the time in question. SFHP Developer/Successor in Interest and SFHP Developer/Successor in Interests and Successor in Interest are used interchangeably.

F. "SFHP Home" shall mean the units of housing to be constructed, marketed and/or sold by SFHP Developer/Successor in Interest at the affordable price pursuant to the SFHP and this Agreement.

G. "SFHP Lot" means housing lot(s) housings to be constructed, developed, marketed and/or sold by SFHP Developer/Successor in Interest as affordable housing.

H. "SFHP Homebuyer" shall mean any person or entity, which purchases the SFHP Home and/or SFHP Lot from the SFHP Developer/Successor in Interest and any subsequent purchasers who buy from the initial purchaser during the term of this Agreement.

I. "Repurchaser" means the City or the City's Assignee. A general delegation of authority by the City to another person as Repurchaser shall transfer those rights, powers and obligations assigned to the Repurchaser in this Agreement or the Lien. Transfer of any rights, powers and obligations assigned to the City in this Agreement shall be effective only to the extent such rights, powers and obligations are specifically enumerated in the delegation of authority.

J. "Senior Lien" means a mortgage with respect to the Property from the Purchaser, as mortgagor, to any lender or its agent or assigns, as mortgagee which loan is solely used to purchase the SFHP Home.

2. THE PROJECT.

2.1 Property. The SFHP Developer/Successor in Interest is or will become the owner of certain property situated in the County of Santa Fe, New Mexico, located at **3600 South Meadows Road** and known as **"Los Prados Phase 1"** which property contains more or less **19.5** acres and is more particularly described in Exhibit A attached hereto (The "Property").

2.2. Intended Conveyance of Interest. The Property is to be developed and the units thereon marketed as for sale housing.

2.3 SFHP Plan. The SFHP Developer/Successor in Interest agrees to construct a **total of ## SFHP Homes** as indicated on the attached Exhibit B (SFHP Plan). The SFHP Plan shall include the number of bedrooms and bathrooms, the minimum square footage and the minimum household size, the income range to be served and the current SFHP Home price for each SFHP Home as set forth in Section 8.2 of the SFHP Administrative Procedures. If the development is constructed in phases, a SFHP Plan for each phase shall be reviewed and approved by the City and recorded at the County Clerk's office prior to issuance of a building permit by the City for that phase.

2.4 Agreement to Sell at SFHP Home Price. The SFHP Developer/Successor in Interest agrees to sell the SFHP Homes at the current SFHP Home Price or such SFHP Home Price as may be current at the time of sale by the SFHP Developer/Successor in Interest.

A. The affordable home price shall be reduced in order to limit the impact on SFHP home buyers of fees assessed by condominium, common area, or homeowner associations. The affordable home price shall be reduced so that the buyer's mortgage or manufactured home loan principal amount and, accordingly, the buyer's monthly mortgage payments, are reduced by an amount equal to the assessed fee in excess of seventy-five dollars (\$75.00). Additionally, the SFHP Developer/Successor in Interest agrees that the association's bylaws will identify limits to annual fee increases for the owners of affordable units so that increases do not exceed 5% annually or 10% over any three-year time frame.

B. The affordable home price may be increased at the request of the SFHP home buyer by the price of allowable option upgrades, not to exceed the maximum option upgrade allowance, as established pursuant to administrative procedures.

C. The affordable home price may be increased by the amount of the approved energy efficiency adjustment pursuant to administrative procedures.

2.5 Agreement for Payments for Fraction Portions of a SFHP Home. SFHP Developer/Successor in Interest **does/does not owe** any fractional portion of a SFHP Home as calculated pursuant to SFHP to the City or its agent prior to recording this Agreement.

2.6 Marketing Sequence. The SFHP Homes shall be marketed at the same time as all other units on the Property or at a time, and in a proportion, equal to the number of non-SFHP Homes being marketed for sale during a given phase of development.

2.7 Development Incentives. The City agrees to the following incentives: Development review and building permit fees shall be waived or reduced proportionately to the number of SFHP Homes and capital impact fees and utility expansion charges shall be waived or reduced as set forth in section 13.1 of the Administrative Procedures. SFHP Developer/Successor in Interest also requests a 15% density bonus as set forth in section 13.2 of the Administrative Procedures.

2.8 Agreements Regarding Transfers of Non-SFHP Homes on the Property. SFHP Developer/Successor in Interest acknowledges and hereby agrees that compliance with this Agreement shall be a precondition for any releases or express termination of SFHP Developer/Successor in Interest's SFHP Agreements and all future City approvals of building permits, certificates of occupancy and utility connections, and such other development approvals regarding the Property as may be required by law for development, construction, occupation and use of the Property.

3. SFHP LIEN.

3.1 Creation of SFHP Lien. The SFHP Developer/Successor in Interest/Successor in Interest hereby creates a lien upon each SFHP Home for the benefit of the City, its agents, heirs, successors and assigns (the "Lien"). The Lien shall be for the sole purpose of complying with the SFHP and this Agreement and for the purpose of retaining long-term affordability of SFHP Homes.

3.2 Subordination. The Lien shall be subject and subordinate in all respects to liens, terms, covenants and conditions of a Senior Lien including all sums advanced for the purpose of (a) protecting or further securing the lien of the Senior Lien, curing defaults by the SFHP Homebuyer, its successors and assigns under the Senior Lien or for any other purpose expressly permitted by the Senior Lien or (b) constructing, renovating, repairing, furnishing, fixturing or equipping the Property. The terms and provisions of the Senior Lien are paramount and controlling, and they supersede any other terms and provisions hereof in conflict therewith. In the event of a foreclosure or deed in lieu of foreclosure of the

Senior Lien, any provisions wherein or in any other collateral agreement restricting the use of the Property to income eligible households or otherwise restricting the SFHP Homebuyer, its successor's or assign's ability to sell the Property, shall have no effect on subsequent purchasers of the Property. Any purchaser following foreclosure, including his successors or assigns (other than the SFHP Homebuyer or a related entity of the SFHP Homebuyer), receiving title to the Property through a foreclosure or deed in lieu of foreclosure of the Senior Lien shall receive the title to the Property free and clear from such restrictions.

Further, if the holder of the Senior Lien acquires title to the Property pursuant to a deed in lieu of foreclosure, this Lien shall automatically terminate upon acquisition of the title by the holder of the Senior Lien, provided that (a) the City has been given written notice of default under the Senior Lien and (b) the City shall not have cured the default under the Senior Lien, or diligently pursued curing the default as determined by the holder of the Senior Lien, within the 60-day period provided in such notice sent to the City.

In no event shall the SFHP Developer/Successor in Interest or any of its officers, principals, or representatives, or employees, or affiliated entities, contract to purchase, purchase, accept or otherwise have transferred to it an SFHP Homebuyer's or Repurchaser's statutory right of redemption of the SFHP Home or SFHP Lot, and in no event shall the aforementioned persons exercise any right of redemption or foreclosure for that SFHP Home or SFHP Lot.

The Lien and/or deed restriction must be shown on Schedule B-II of the lender's title policy as a subordinate item(s) to the first mortgage that is to be sold to Fannie Mae.

Notwithstanding any terms to the contrary, herein, the City and its agent's rights to collect and apply the insurance proceeds of hazard or property insurance or other insurance proceeds shall be subject and subordinate to the rights of the Senior Lien holder to collect and apply such proceeds in accordance with the Senior Lien. Likewise, the proceeds of any award or claim for damages, direct or consequential, in connection with any condemnation or other taking of any part of the property subject to the Lien, or for conveyance in lieu of condemnation, are hereby assigned and shall be paid to the Senior Lien holder, subject to the terms of the Senior Lien.

3.3 Lien Amount. The Lien amount shall be the difference between the current SFHP Home price at the time of closing and 95% of the Fair Market Value as determined by a certified appraiser. The Lien amount shall be calculated after construction of the SFHP Home but prior to purchase by the SFHP Homebuyer. SFHP Developer/Successor in Interest agrees to execute such additional documents as are necessary to record the SFHP Lien with the Lien amount.

3.4 Term of Lien. The Lien shall run from the date of purchase of the SFHP Home by the SFHP Homebuyer or SFHP Developer/Successor in Interest until such time as the City exercises its right of first refusal or purchase under this Agreement. During this term, so long as the SFHP Homebuyer or SFHP Developer/Successor in Interest complies with SFHP and the terms of this Agreement, no payments will be due on this Lien.

3.5 Transfers of Lien. The Lien may be assumed by subsequent SFHP Homebuyers of the SFHP Home, including those acquiring the SFHP Home through testate or intestate succession so long as a Notice of Intent to Sell is received by the City and the City gives written approval of the assumption of the Lien. The Lien shall not be assigned by the City or its agent without the Senior Lien holder's prior written consent so long as the Senior Lien remains outstanding and the obligation to make payment on the mortgage note has not been satisfied. Subsequent SFHP Homebuyers or SFHP Developer/Successor in Interest shall also obtain a "Certificate of SFHP Homebuyer Eligibility".

3.6 Collection of the Lien Amount. The City and its agent agree that they will not commence foreclosure proceedings or accept a deed in lieu of foreclosure, or exercise any other rights or remedies hereunder until it has given the Senior Lien holder at least 60 days prior written notice. The subordinate lien holder (the City or its agent) will give the Senior Lien holder written notice of default and prior written notice of any acceleration under the subordinate mortgage (the Lien). However, it is understood that the subordinate lien holder is only holding the Lien for purposes of retaining long-term affordability and, therefore, will not accelerate the payment of the Lien amount.

In the event of a foreclosure on the Senior Lien the City hereby agrees to accept a note for any deficiency in the Lien amount and to forgive, release and forgo collection upon the Lien.

4. CLOSING SALES AND CONVEYANCES OF THE SFHP HOMES.

4.1 Certification of Homebuyer. SFHP Developer/Successor in Interest agrees to sell the SFHP Homes only to Certified Homebuyers who qualify for the purchase of a SFHP Home under the SFHP. The City hereby represents and warrants that Santa Fe Community Housing Trust or Homewise are its authorized agents under the most current agreements between the City and Santa Fe Community Housing Trust or Homewise, for the purpose of qualifying buyers under this Agreement and issuing Certificates of SFHP Homebuyer Eligibility.

4.2 SFHP Developer/Successor in Interest's or SFHP Homebuyer's Notice of Intent to Sell or Transfer the Property. Any time the SFHP Developer/Successor in Interest or any subsequent SFHP Homebuyer who is subject to this Lien intends to sell or otherwise transfer the Property or any interest in the SFHP Home, whether by deed, real estate contract, intestacy, testate succession or otherwise, said transferor shall give written notice to the City which shall state the transferor's intention to sell or otherwise transfer said Property or interest in the SFHP Home. (The "Notice of Intent to Sell").

4.3 Notice to Prospective SFHP Homebuyer of Lien. The SFHP Developer/Successor in Interest agrees to provide actual notice to any prospective purchaser of the covenants, conditions, and encumbrances, which are or will be placed upon the SFHP Homes to be sold by SFHP Developer/Successor in Interest to the Certified SFHP Homebuyer. Any purchase agreement containing the foregoing language or accompanied by a signed disclosure statement containing the foregoing language shall be deemed acceptable to the City pursuant to Section 26-1.18 SFCC 1987. The notice shall, at a minimum, state:

A. The SFHP Home and/or SFHP Lot is subject to a Lien which is intended to retain the long-term affordability of the SFHP Home.

B. The Lien may limit the ability of the SFHP Homebuyer to get subsequent mortgage or equity loans.

C. The Lien gives the City and its agents a right to exercise its right of first refusal and repurchase of the SFHP Home and/or SFHP Lot at the fair market value less the amount of the lien.

D. The Right of First Refusal granted to the City can be assigned by the City to an agent or third party Certified SFHP Homebuyer.

E. The Lien will limit the SFHP Homebuyer's ability to sell the SFHP Home in the future and shall require the City or its agents to agree to any additional use of the SFHP Home as collateral or security.

F. The Lien will reduce the total amount payable to the SFHP Homebuyer upon resale by the Lien amount if the SFHP Home is sold or transferred during the Lien term.

4.4 Escrow Instructions Regarding Initial SFHP Compliance. SFHP Developer/Successor in Interest agrees to sell the SFHP Homes through an escrow agent. The escrow agent shall be required by SFHP Developer/Successor in Interest as a precondition for closing to certify to the City that:

A. The SFHP Homebuyer has been notified of and has indicated their understanding of the Lien and its affect.

B. The total sales price at closing does not exceed the SFHP Home Price plus such transactional costs of closing as are usual and customary.

C. The SFHP Home is being sold to a buyer who has agreed that the SFHP Home shall be their principal residence and who has provided the escrow agent with a Certificate of SFHP Homebuyer Eligibility.

4.5 SFHP Developer/Successor in Interest to Reference Agreement and Lien. The SFHP Developer/Successor in Interest shall include a reference to this Agreement in any and all deeds or other instruments conveying any interest in the SFHP Home and/or SFHP Lot or any part thereof or interest therein, although neither the validity nor enforceability of this Agreement shall be affected in any manner by failure to do so.

5. RIGHT OF FIRST REFUSAL OR PURCHASE.

5.1 Grant of Right of First Refusal. To maintain the SFHP Home as affordable the City and its agents shall have, and SFHP Developer/Successor in Interest, its heirs, successors and assigns, hereby grant the City and its agent the right (but without obligation) to purchase the SFHP Home and SFHP Lot in any of the following circumstances (the "Rights of First Refusal"):

A. The SFHP Developer/Successor in Interest has given the City Notice of Intent to Sell; or

B. The SFHP Home is no longer the principal residence of any person purchasing the SFHP Home from the SFHP Developer/Successor in Interest or any subsequent SFHP Homebuyer, their heirs, successors or assigns whose occupancy does not comply with the covenant to maintain the SFHP Home only as a principle residence and not as a trade or business, as an investment property, (income/rental property) or as a recreational second home, or non-principal residence so long as said SFHP Homebuyer of the SFHP Home has been given written notice identifying the violations and has failed to cure them; or

C. Any legal or beneficial interest in the SFHP Home and/or SFHP Lot is conveyed or otherwise transferred (for example, by inheritance) without both Notice of Intent to Sell and Second Notice of Intent to Sell as required in this Section having been given, unless the City has waived the Right of First Refusal in writing; or

D. The City has notice of a pending mortgage or other lien foreclosure or similar proceeding (for instance, a sheriff's sale) against the SFHP Home and/or SFHP Lot or of a civil action or equivalent proceeding for unpaid condominium common expenses; or

E. The City has notice that the SFHP Home and/or SFHP Lot is being taken for unpaid taxes; or

F. The SFHP Homebuyer made material misrepresentations in applying to acquire the SFHP Home and/or SFHP Lot; or

G. The SFHP Homebuyer has failed to observe and perform the SFHP Homebuyer's obligations under this Agreement (other than as stated in (H.) below), has been given written notice identifying the violations and has failed to cure them; or

H. The SFHP Homebuyer has failed to observe and perform the SFHP Homebuyer's obligations under this Covenant in a manner, which constitutes criminal conduct, or in the City's judgment constitutes other willful, egregious and continuing violation of such obligations.

The City shall be obligated to give notice and an opportunity to cure only for events under subsections (B.) or (G.); and for those events, the SFHP Homebuyer and any holder of a Senior Lien shall have a reasonable time to cure which shall not exceed six months. In all cases other than sale or other transfer of the SFHP Home under subsection (A.), the City's right to buy the SFHP Home shall continue only while the event giving rise to exercise of the Right of First Refusal continues un-remedied.

5.2 City's Exercise of Right of First Refusal. In the event that the City has received a Notice of Intent to Sell pursuant to this Agreement, the City shall have 30 days in which to exercise its Right of First Refusal. During this 30-day period, the City and its agent shall have the exclusive right to market the SFHP Home and the SFHP Homebuyer shall not list the SFHP Home with a realtor or any other selling agent.

5.3 Duration of Right of First Refusal. The City and the SFHP Homebuyer intend that the Right of First Refusal are for the purpose of promoting and enforcing this Agreement in its goal of ensuring continuance of the SFHP Home as affordable housing as permitted under New Mexico law and accordingly are intended to be exercisable throughout the term of this Agreement (or such shorter period as is the longest permitted under law).

5.4 Purchase Price. The purchase price of the SFHP Home and/or SFHP Lot under the Right of First Refusal shall be equal to the appraised price at the time of sale less the Lien amount. In the event that there is a dispute as to the purchase price, the seller shall be entitled, at its own expense, to have a separate appraisal performed and if the seller and City or its agent still can not agree as to the purchase price, the seller and City shall resolve the matter through binding arbitration.

6. MONITORING AND ENFORCEMENT.

6.1 Notice. The City agrees to provide notice pursuant to the SFHP of any violation or alleged violation of the terms and conditions of this Agreement and further agrees to give SFHP Developer/Successor in Interest 30 days to cure and violation under this Agreement. SFHP Developer/Successor in Interest agrees to provide the City with notice and 30 days opportunity to cure any violation of this Agreement.

6.2 Term. This Agreement shall become effective when signed by the SFHP Developer/Successor in Interest and the City, and shall remain in effect: (1) as to the SFHP Developer/Successor in Interest, until all SFHP Homes on the Property covered by this Agreement have been sold and all other obligations on the part of the SFHP Developer/Successor in Interest under this

Agreement have been satisfactorily discharged; and (2) as to the terms and conditions which apply to the SFHP Homebuyers, for as long as the Lien on the SFHP Home remains in effect under this Agreement, including any renewals.

6.3 Enforcement. It is understood and agreed by SFHP Developer/Successor in Interest that the remedies provided under this Agreement are additional remedies, not exclusive of any remedy under the law.

6.4 Release by City. City hereby agrees to release SFHP Developer/Successor in Interest from this Agreement upon fulfillment of all terms of this Agreement and to issue upon request a certificate of termination within a reasonable time of receipt of written request. The City will issue the certificate in recordable form stating (if such be the case) that this Agreement has been terminated, and any such certificate, when recorded with the Santa Fe County Clerk, shall be binding and conclusive on the City and all persons relying thereon.

7. MISCELLANEOUS PROVISIONS.

7.1 Recordation. SFHP Developer/Successor in Interest agrees to record this Agreement and thus the Lien prior to closing the sale of the SFHP Home so as to legally attach, run with the land and ensure that the purposes of the SFHP are met.

7.2 Successors and Assigns; Covenants to Run. This Agreement shall be legally binding on, as the obligations of, the parties and their respective successors and assigns, including without limitation, successors in title or interest to the Property, SFHP Home or this Agreement, who by their acceptance of any interest in the Property, SFHP Home or this Agreement shall be deemed to have agreed to perform and observe all the SFHP Developer/Successor in Interest's obligations under, and to be bound by all the terms and conditions of, this Agreement. Furthermore, all the agreements, rights and restrictions set forth in this Agreement shall run with the Property for the purpose of maintaining the SFHP Home as affordable housing throughout the term of this Agreement and the Liens entered into pursuant to this Agreement.

7.3 Reporting and Verification.

A. SFHP Developer/Successor in Interest agrees to sign an affidavit declaring that the sale price did not exceed the amount specified in this Agreement.

B. SFHP Developer/Successor in Interest agrees to provide such information and documentation as the City may reasonably require in order to ensure that the actual sale was in compliance with this Agreement.

C. SFHP Developer/Successor in Interest agrees to provide income verification in selling the SFHP Home for certification by the City or its agent as complying with the SFHP Ordinance.

D. SFHP Developer/Successor in Interest agrees to grant access to the City, or its agent, to inspect the records of SFHP Developer/Successor in Interest for the SFHP Home in order to determine compliance with the SFHP Ordinance and this Agreement.

7.4 Further Assurances. The parties agree to execute such documents as may be required to show the satisfactory compliance with SFHP and this Agreement. The City may certify current compliance with the SFHP on a unit by unit basis as may be reasonably requested by SFHP Developer/Successor in Interest, escrow agents, realtors, lenders or any other interested party.

7.5 City's Right to Enter. Each SFHP Developer/Successor in Interest hereby grants to the City the right to enter upon the SFHP Home upon reasonable notice for the purpose of inspection and enforcement of the SFHP, the Santa Fe City Code and this Agreement.

7.6 Survival of Enforcement Rights. Notwithstanding the definition of SFHP Developer/Successor in Interest contained in this Agreement, the rights of enforcement for violations of this Agreement shall survive any subsequent sale or transfer of the SFHP Home.

7.7 City's Zoning Authority Unimpaired. The City's rights to enforce any and all provisions of the Santa Fe City Code shall be the same as its rights generally to enforce any other ordinance, which shall in no way be diminished by this Agreement. The SFHP Developer/Successor in Interest acknowledges that this Agreement is included within the zoning authority and charter authority of the City, which is, and is hereby, accepted by the SFHP Developer/Successor in Interest as a sufficient but additional basis for this Agreement.

7.8 Notices. Any demand, notice or request by either party to the other shall be sufficiently given if in writing delivered to the party intended to receive the same, or if mailed by certified mail, return receipt requested, or delivered to a recognized national courier, or if given in a manner sufficient for legal process. Each notice to the SFHP Developer/Successor in Interest named above shall be addressed to such party, or to such other address as may be stated in a notice given as herein provided. Each notice to subsequent SFHP Developer/Successor in Interests shall be sufficiently given if addressed to or given at the SFHP Home.

7.9 Public Purpose. The City declares, and the SFHP Developer/Successor in Interest and each other person, including mortgagees, hereafter holding any interest in the Property or a SFHP Home acknowledges, that the Lien and this Agreement as well as all restrictions contained in this Agreement are for public purposes.

7.10 Sanctions. The City has the right to impose sanctions or take other actions as set forth in the SFHP Ordinance after notice of violation has been given and not complied with.

7.11 Headings Not Part of Agreement. Section headings have been inserted for convenient reference only and are not to be construed as part of this Agreement.

7.12 Severability. If any provision of this Agreement or the application thereof to any person or circumstances is held to be invalid or unenforceable by any decision of any court of competent jurisdiction, such decision shall not impair or otherwise affect any other provision of this Agreement, or the application of such provision to persons or circumstances other than those as to which it is held invalid or unenforceable.

7.13 Interpretation. This Agreement shall be enforceable according to its terms and New Mexico law. This Agreement is subject to the general principles of equity, fairness and reasonableness irrespective of whether such enforcement or interpretation is considered in a proceeding at equity or in law and shall be construed according to its purpose of fostering and preserving affordable housing. Any benefit to private parties arising under this Agreement is purely incidental to the purpose of creating affordable housing.

7.14 No Additional Payments. Nothing herein shall be deemed a requirement to pay to the City or its agent additional payments for amounts already paid to the Senior Lien holder. The borrower shall not be obligated to make such payments of funds to the City or its agent to the extent that

the borrower under the Senior Lien makes such payments to the Senior Lien holder in accordance with the Senior Lien.

7.15 No Waiver. No actions taken by the parties following a breach of any of the terms contained in this Agreement shall be construed to be a waiver of any claim or consent to any succeeding breach of the same or any other term.

7.16 Numbers and Genders. Whenever used herein, unless the context shall otherwise provide, the singular number shall include the plural, the plural the singular, and the use of any gender shall include all genders.

7.17 Agreement. This Agreement states the entire agreement of the parties. This Agreement shall not relieve SFHP Developer/Successor in Interest from complying with present or future City ordinances, duly adopted resolutions or regulations applicable to the development.

7.18 Amendments. This Agreement shall not be altered, changed or amended except by instrument in writing executed by the parties hereto. Any amendments to this Agreement shall be reviewed by the Office of Affordable Housing and sent, if applicable, to the appropriate review body as set forth in the SFHP Ordinance and then to the City Manager for approval.

WHEREFORE, the parties set their hands and seals this ____ day of _____, _____.

ATTEST:

CITY OF SANTA FE:

KRISTINE MIHELIC, CITY CLERK

JOHN W. BLAIR
CITY MANAGER

APPROVED AS TO FORM:

ERIN MCSHERRY, CITY ATTORNEY

SFHP DEVELOPER/SUCCESSOR IN INTEREST:

By: _____
Signature

Printed Name

Its: _____
Title

ACKNOWLEDGEMENTS

STATE OF NEW MEXICO)
) ss.
COUNTY OF SANTA FE)

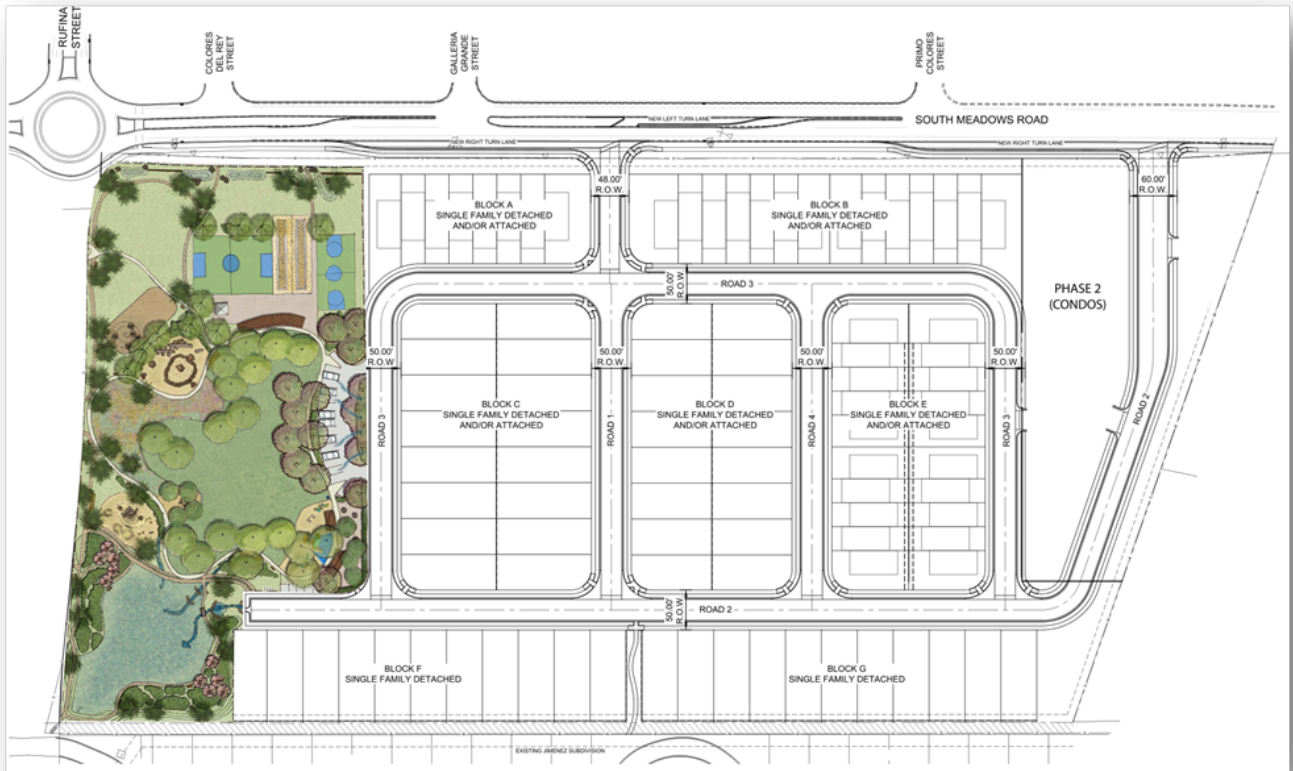
The foregoing instrument was acknowledged before me this ____ day of _____, _____,
by _____.

NOTARY PUBLIC

My Commission Expires:

Attachments: Exhibit A: The Property
 Exhibit B: SFHP Plan

INSERT SITE PLAN, LOCATION MAP OR OTHER GRAPHIC REPRESENTATION OF PROJECT SITE/AREA



SANTA FE HOMES PROGRAM SFHP PLAN AND HOME SALES PRICING SCHEDULE

Effective June 15, 2023*

(without green code adjustment pricing, unless noted)

<i>Income Range</i>	<i>Two Bedrooms</i> <i>1-2 person HH</i> <i>(900 sq min)</i>	<i>Three Bedrooms</i> <i>3-4 person HH</i> <i>(1,150 sq ft min)</i>	<i>Four Bedrooms</i> <i>4-5 person HH</i> <i>(1,250 sq ft min)</i>
2 (50-65%AMI)	Max. Price \$161,250 2 Units	Max. Price \$181,500 2 Unit	Max. Price \$201,500 1 Units
3 (65-80%AMI)	Max. Price \$209,750 3 Unit	Max. Price \$236,000 3 Unit	Max. Price \$262,000 3 Units
4 (80-100%AMI)	Max. Price \$258,000 1 Unit	Max. Price \$290,250 2 Units	Max. Price \$322,500 2 Units

Prices reflect June 2023 HUD Area Median Incomes for Santa Fe. Refer to Section 26-1.16 (B) and the SFHP Administrative Procedures. For specific requirements, contact The Office of Affordable Housing.

PHASING/AFFORDABILITY PLAN

Phase 1 Affordable Lot #s: 49

Phase 2 Affordable Lot #s: 31

AFFORDABILITY CALCULATION

Single Family/Condo Building Lots

The project has an area of approximately 19.5 acres, zoned R-6, permitting 6 dwelling homes per acre. The required number of SFHP units is 20% of the total units, 5% each in Income Ranges 2 and 4 and 10% in Income Range 3. The project proposes **97 of total lots** for development as single family homes.

CALCULATION for the SFHP requirement:

= Total number of units multiplied by (0.2) = # of Units Required

= **97 total units** x 0.2 = **19.4 SFHP unit(s)** are required

= No fractional fee is due because the Developer proposes to sell to income-qualified homebuyers **29 additional** Low Priced Dwelling Units (LPDUs), as priced according to the schedule below.

= **48 TOTAL Affordable Homes**

[INSERT LPDU SCHEDULE]

NOTE: The home prices and fractional fee schedule are modified by the City according to Section 8.7.3 of the SFHP Administrative Procedures to reflect annual changes in the median income levels. The SFHP Home prices shown in this SFHP Agreement are the prices in effect at the time this Agreement is made. The current SFHP prices that are in effect at the time the SFHP Home is made available for sale or the fractional fees are paid, determines the actual SFHP Home Price and/or amount of fractional fee. The prices are updated annually.

LOS PRADOS SUBDIVISION
PRELIMINARY WATER BUDGET
March 11, 2024

97	LOTS	
0.15	AFY	PER LOT <6,000 SF
0.17	AFY	PER LOT 6,000 - 10,890 SF
0.25	AFY	PER LOT >10,890 SF

48	MARKET LOTS <6,000 SF	7.20	AFY
1	MARKET LOTS 6,000 - 10,890 SF	0.17	AFY
19	SFHP LOTS <6,000 SF	2.85	AFY
29	LOW PRICED DWELLING UNITS <6,000 SF	4.35	AFY
97	<i>SUBTOTAL DOMESTIC WATER BUDGET</i>	14.57	AFY
	<i>COMMON AREA IRRIGATION (Average of Years 1-3)</i>	0.20	AFY
TOTAL WATER BUDGET		14.77	AFY
	AFFORDABLE LOTS <6,000 SF	2.85	AFY
	LOW PRICED DWELLING UNITS <6,000 SF	4.35	AFY
	<i>SUBTOTAL AFFORDABLE LOTS</i>	7.20	AFY
DEVELOPMENT WATER BUDGET - MARKET LOTS & IRRIGATION		7.57	AFY
	9.8% LINE LOSS CONTINGENCY	0.74	AFY
TOTAL WATER DEMAND OFFSET		8.31	AFY

LANDSCAPE IRRIGATION BUDGET				
	GPW	GPY	AFY	
YEAR 1	2,503	137,665	0.42	
YEAR 2	2,503	55,066	0.17	
YEAR 3	0.0	0.0	0.00	
<i>3 YEAR AVERAGE IRRIGATION BUDGET</i>	<i>1668.7</i>	<i>64243.7</i>	<i>0.20</i>	

Los Brios Subdivision Water Budget per Lot Submittal 4/5/22

Lot #	Lot Size	Water Budget	Market or Affordable	Lot Size in acres
2	5,625	0.15 afy	AFFORDABLE	0.13
5	5,584	0.15 afy	AFFORDABLE	0.13
15	5,375	0.15 afy	AFFORDABLE	0.12
31	5,375	0.15 afy	AFFORDABLE	0.12
35	5,375	0.15 afy	AFFORDABLE	0.12
40	5,375	0.15 afy	AFFORDABLE	0.12

0.15	AFY	PER LOT <6,000 SF
0.17	AFY	PER LOT 6,000 - 10,890 SF

Los Brios Subdivision Water Budget per Lot Submittal 4/5/22

Lot #	Lot Size	Water Budget	Market or Affordable	Lot Size in acres
5	5,584	0.15 afy	AFFORDABLE	0.13
14	5,710	0.15 afy	AFFORDABLE	0.13
16	5,280	0.15 afy	AFFORDABLE	0.12
31	5,375	0.17 afy	AFFORDABLE	0.12
35	5,375	0.17 afy	AFFORDABLE	0.12
40	5,375	0.17 afy	AFFORDABLE	0.12

Open Space Tract	SF	Acre
A	8,711	0.20
B	3,307	0.08
C	29,977	0.69
D	12,250	0.28
E	99,094	2.27
F	2,874	0.07
G	40,087	0.92
Los Brios North Landscape Strip	19,990	0.46
Los Brios South Landscape Strip	23,889	0.55
Total	240,179	5.51
Site Area		17.15
Provided Open Space		5.51
Required open space 30%		5.15
Developed		1.44
Undeveloped		4.07
Required Open Space		30%
Provided Open Space		32%
Developed Open Space		8%

LOS BRIOS SUBDIVISION
CITY WATER BUDGET
April 5, 2022

EXHIBIT F2

		56	LOTS	
		0.15	AFY	PER LOT <6,000 SF
		0.17	AFY	PER LOT 6,000 - 10,890 SF
		0.25	AFY	PER LOT >10,890 SF
46	MARKET LOTS 6,000 - 10,890 SF	7.82	AFY	
2	MARKET LOTS >10,890 SF	0.50	AFY	
6	AFFORDABLE LOTS <6,000 SF	0.90	AFY	
5	AFFORDABLE LOTS 6,000 - 10,890 SF	0.85	AFY	
TOTAL		59	10.07	AFY
SUBTOTAL ANNUAL WATER BUDGET		10.07	AFY	
		0.17	AFY	PER LOT
LESS AFFORDABLE LOTS		1.75	AFY	
TOTAL DOMESTIC WATER BUDGET		8.32	AFY	
COMMON AREA IRRIGATION		0.13	AFY	
DEVELOPMENT WATER BUDGET - MARKET LOTS		8.45	AFY	

9.8% LINE LOSS CONTINGENCY 0.83 AFY

TOTAL DEVELOPMENT WATER BUDGET 9.27 AFY

**Santa
Fe
Engineering
Consultants, LLC**

Civil and Traffic Engineering

Construction Management

Land Development

**TERRAIN MANAGEMENT REPORT
SOILS, GRADING, AND DRAINAGE
SUBDIVISION PLAT SUBMITTAL FOR LOS PRADOS
3600 AND 3740 SOUTH MEADOWS ROAD
SANTA FE, NEW MEXICO**

Prepared For:

**Homewise, Inc.
1301 Siler Road, Bldg D
Santa Fe, NM 87507**

Prepared By:

**Santa Fe Engineering Consultants, LLC
1599 S. St. Francis Drive, Suite B
Santa Fe, New Mexico**

**November 2023
Revised June 2024**

**TERRAIN MANAGEMENT REPORT
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**TERRAIN MANAGEMENT REPORT
SOILS, GRADING, AND DRAINAGE
SUBDIVISION PLAT SUBMITTAL FOR LOS PRADOS
3600 AND 3740 SOUTH MEADOWS ROAD
SANTA FE, NEW MEXICO**

I. INTRODUCTION

Homewise Inc. is proposing to construct a new residential development named Los Prados to be located southwest of the intersection of South Meadows Road / Rufina Street in Santa Fe, NM. The 22.2 ± AC site is currently vacant. It is proposed to develop a mix of single-family lots, townhouses, and condominiums.

The proposed development is located on a vacant parcel west of South Meadows Road. The site is located within the City of Santa Fe within Section 1, Township 16 North, Range 8 East, N.M.P.M. The coordinates are Latitude 35°38'30.75" North and Longitude 106°2'19.43" West. Access to the development is provided by two new roadways off of South Meadows Road. The Vicinity Map is presented in Figure 1, Appendix A. The legal lot of record is presented in Figure 2, Appendix A.

The proposed Civil Site Plan is presented in Figure 3, Appendix A. The Subdivision Plat shows a mix of uses as presented in Table 1:

TABLE 1 PROPOSED DEVELOPMENT MAXIMUM DENSITIES ALLOWED BY ZONING		
Description	Area (Ac)	Dwelling Units
TRACTS 4A and 4B (Proposed Zoning – R-6, 6 Dwelling Units Per Acre)	19.54 Acres	
4-B Community Park	5.12 Acres	
4-A Unit Count (Inclusive of 15% Density Bonus)	19.54 Acres	134.8 Units
4-A Single Family Lots	43%	58
4-A Townhouses	57%	77
Subtotal		135 (Rounded)
TRACT 3 (Proposed Zoning – R-21, 21 Dwelling Units Per Acre)	2.66 Acres	
Unit Count - 2.66 Acres (Inclusive of 15% Density Bonus)	2.66 Acres	64.23
Condominiums (future, by others)		64 (Rounded)
TOTAL	22.2 Acres	199

The purpose of this report is to evaluate soils and drainage conditions and to provide recommendations for site grading and runoff control to meet City of Santa Fe Terrain Management Regulations.

The central residential portion of the development in Tract 4-A will consist of 55 single family lots and 42 townhouse lots. On lot ponding will not be required for these lots.

The community park on the north end of the development in Tract 4-B will be dedicated to the City of Santa Fe. Homewise has committed to construct the park improvements. The park is being designed in conjunction with the City Parks and Recreation department, the neighborhood, and Homewise.

The condominium area at the south end of the development in Tract 3 will not be built at this time. However, this area will still be rough graded and used as the contractor's designated staging area during construction. This report accounts for rough grading of these areas. Any future development in this area will require a new Terrain Management Report to account for any additional runoff.

There is a natural low spot on the west end of the property that causes shallow ponding. Overflow from this low spot is currently directed behind the existing Jimenez residential subdivision on Morning Drive. The proposed development will mitigate the existing deficient drainage patterns in the area. The streets within the development are to be dedicated to the City of Santa Fe. In order to maintain the existing hydrologic conditions, it is proposed that runoff from the public streets be allowed to enter the water catchments areas west of the park to be used as a rain garden and water harvesting amenity for storm water attenuation and educating the community on water harvesting techniques. The Los Prados Homeowners Association will be responsible for the maintenance of the Water Catchment Area. A drainage easement for this is shown on the preliminary Subdivision Plat in Figure 4, Appendix A.

II. EXISTING CONDITIONS

The Los Prados development will be located on land that is currently vacant. The property is undeveloped and is mainly in its natural condition. Existing ground cover consists of herbaceous land with a fair native grass cover.

Existing land uses in the area vary from commercial to residential to vacant land. The site is located west of South Meadows Road; the Colores del Sol Subdivision borders the site on the north and east; Eberline Instrument Corp, Country Club mobile home park, and Airport Road are located to the south. The Jimenez subdivision is located on the east. The Existing Conditions Map is presented in Figure 5, Appendix A

Topographic mapping at a scale of 1 inch = 100 feet, with 1-foot contour intervals was prepared for this project by Rick Chatroop, N.M.L.S. 11011. The certified topographic map is presented in Figure 6, Appendix A.

A slope analysis was prepared using one-foot contours. The Slope Analysis Map is presented on the project plans (full size) and Figure 7, Appendix A. The slope analysis map indicates that the majority of the project is relatively flat with less than 20% slopes. The development is situated outside of any natural 30% slopes. Any 30% and greater slopes within the project limits are manmade.

III. SOILS

Soil classifications were obtained from the Natural Resource Conservation Service, "Soil Survey of Santa Fe Area, N.M.," by the U.S.D.A., dated July 14, 2022. The survey in this area is the result of a high intensity soil survey. The Soils Map is presented in Figure 8, Appendix A.

The following soils are to be found onsite:

TABLE 1 ONSITE SOILS				
No.	Soil	Natural Drainage Class	Capacity of Transmit Water	Hydrologic Soil Group
103	Zepol silt loam, 0 to 2 percent slopes, flooded	Well drained	Moderately high to high (0.60 to 2.00 in/hr)	B
114	Devargas-Urban land complex, 1 to 3 percent slopes	Well drained	Moderately high (0.20 to 0.57 in/hr)	C
115	Panky-Urban land complex, 1 to 4 percent slopes	Well drained	Moderately low to moderately high (0.06 to 0.20 in/hr)	C

The NCRS has classified the soils into four hydrologic soils groups. The hydrologic soils groups, according to their infiltration and transmission rates, are defined as follows:

- A. (Low runoff potential). Soils having high infiltration rates even thoroughly wetted. These consist chiefly of deep, well to excessively drained, sands or gravel. These soils have a high rate of water transmission in that water readily passes through them.
- B. Soils having moderate infiltration rates when thoroughly wetted. These consist chiefly of moderately deep to deep, moderately well to well drained soils with moderately fine to moderately coarse textures such as loamy sands and course sandy loams. These soils have a moderate rate of water transmission.
- C. Soils having slow infiltration rates when thoroughly wetted. These consist chiefly of soils with a layer that impedes downward movement of water or soils with moderately fine-to-fine textures such as loams and fine sandy loams. These soils have slow rate of water transmission.

- D. (High Runoff Potential). Soils having very slow infiltration rates when thoroughly wetted. These consist chiefly of clay soils with a high swelling potential, soils with a permanent high-water table, soils with a claypan or clay layer at or near the surface, and shallow soils over nearly impervious material. These soils have a very slow rate of water transmission.

For the onsite development, it is assumed that the onsite soils are Type C with herbaceous fair cover.

IV. DRAINAGE

According to maps prepared by the United States Department of Commerce, presented in NOAA Atlas 14 Volume 1, Version 5, dated 2004, revised 2017, the precipitation data in Table 2 should be used for the project:

TABLE 2 PRECIPITATION DATA		
Frequency	Duration	Precipitation
100 Year	24 Hour	3.23"
50 Year	24 Hour	2.91"
25 Year	24 Hour	2.60"

A. Existing Conditions

Runoff at this location flows mostly from the south to the north. Drainage areas were delineated using the on-site certified topographic mapping by Rick Chatroop, Professional Land Surveyor. Offsite topographic mapping was obtained using USGS LiDAR data.

Drainage areas were measured in AutoCAD Civil 3D. Drainage area and channel lengths were measured several times and average numbers used for this report. The Existing Conditions Basin Map is presented in Figure 9, Appendix A. Offsite drainage enters the site from the north and from the south. It should be noted that the Eberline Instrument Corp building and most of their property drains to the south to Airport Road. A small portion of the Eberline property drains northerly into the site. There is a natural low spot within the park that causes shallow ponding with an overflow directed to the rear yards of existing residencies in the Jimenez Subdivision on Moring Drive. This existing drainage deficiency will be mitigated by this project. All runoff that enters the site will be discharged to a more appropriate location at the northwest corner of the property.

The drainage basins which impact the site are presented in Table 3.

TABLE 3 EXISTING DRAINAGE BASIN CHARACTERISTICS		
Basin	Description	Acres
Q1	Offsite basin entering the site from the south (Eberline property)	1.04
Q2	Onsite basin exiting the site on the north	24.4
Q3	Offsite basin entering the site from the north	1.27
Q4	Onsite basin exiting the site on the east	0.46

The WinTR-55 software by the NRCS, version 1.00.10 dated April 2011, was used to calculate weighted runoff number, time of concentration, and peak flow for the existing drainage basins. TR-55 takes into account soil type, time of concentration, and cover conditions to estimate runoff. The graphical discharge method was used. The 25, 50, and 100-year frequency storms were calculated.

Existing drainage calculations are presented in Appendix B. The calculated peak flows by basin are presented in Table 4.

TABLE 4 PEAK FLOW QUANTITIES EXISTING CONDITIONS					
Basin	Description	CN	Q25 (CFS)	Q50 (CFS)	Q100 (CFS)
Q1	Offsite basin entering the site from the south (Eberline property)	81	1.03	1.28	1.54
Q2	Onsite basin exiting the site on the north	81	21.21	26.37	31.83
Q3	Offsite basin entering the site from the north	81	1.61	1.98	2.39
Q4	Onsite basin exiting the site on the east	81	0.53	0.65	0.79

B. Developed Conditions

The proposed development will alter existing drainage patterns. Drainage areas and channel flow lines were delineated on the grading plan. The Developed Conditions Basin Map is presented in Figure 10, Appendix A. Drainage areas were measured in AutoCAD Civil 3D. The basins, basin descriptions, and areas are presented in Table 5.

TABLE 5 DEVELOPED DRAINAGE BASIN CHARACTERISTICS		
Basin	Description	Acres
Q1	Offsite basin entering the site from the south	1.04
QD2A	Flow to DI on Calle Campo	1.50
QD2B	Flow to DI on Calle Campo	0.83
QD2C	Flow to DI on Camino Del Llano	0.54
QD2D	Flow to DI on Camino Del Llano	1.45
QD2E	Flow to DI on Calle Del Girasol	1.05
QD2F	Flow to DI on Camino Del Llano	0.56
QD2G	Flow to DI on Camino Del Llano	0.76
QD2H	Flow to DI on Camino Del Llano	0.03
QD2I	Flow to DI on Camino Del Llano	0.28
QD2J	Flow to DI on Camino Del Llano	0.60
QD2K	Flow to DI on Camino Del Llano	0.17
QD2L	Flow to DI on Camino Del Llano	0.30
QD2M	Flow to DI on Camino Del Llano	0.90
QD2N	Flow to DI on Camino Del Llano	0.09
QD2O	Flow to DI on Camino Del Llano	0.67
QD2P	Flow to DI on Calle Del Girasol	0.94
QD2Q	Flow to DI on Avenida De Los Prados	1.13
QD2R	Flow to DI on Avenida De Los Prados	0.86
QD2S	Flow to DI on Calle Campo	1.31
QD2T	Flow to DI on Calle Campo	3.13
QD2U	Flow on South Meadows Road	0.18
QD2V	Flow on South Meadows Road	0.10
QD2W	Flow on South Meadows Road	0.09
QD2X	Outflow from Pond	5.05
QD3	Offsite basin entering the site from the north	0.75

The WinTR-55 software by the NRCS, version 1.00.10 dated April 2011, was used to calculate weighted runoff number, time of concentration, and peak flow for the existing standalone drainage basins. The graphical discharge method was used. Runoff curve numbers were chosen based on the land uses that make up each basin. The majority of the land use is “Residential districts (1/8 acre)” with hydrologic soil group C. The 25, 50, and 100-year frequency storms were calculated. Developed conditions drainage calculations are presented in Appendix C. The calculated peak flows by basin are presented in Table 6.

TABLE 6
PEAK FLOW QUANTITIES
DEVELOPED CONDITIONS

Basin	Description	CN	Q25 (CFS)	Q50 (CFS)	Q100 (CFS)
Q1	Offsite basin entering the site from the south	81	1.03	1.28	1.54
QD2A	Flow to DI on Calle Campo	92	3.34	3.86	4.38
QD2B	Flow to DI on Calle Campo	90	1.59	1.89	2.14
QD2C	Flow to DI on Camino Del Llano	95	1.48	1.68	1.98
QD2D	Flow to DI on Camino Del Llano	92	2.92	3.37	3.83
QD2E	Flow to DI on Calle Del Girasol	90	2.25	2.63	3.02
QD2F	Flow to DI on Camino Del Llano	90	1.40	1.63	1.88
QD2G	Flow to DI on Camino Del Llano	90	1.60	1.87	2.15
QD2H	Flow to DI on Camino Del Llano	98	0.10	0.12	0.13
QD2I	Flow to DI on Camino Del Llano	90	0.60	0.70	0.80
QD2J	Flow to DI on Camino Del Llano	90	1.28	1.50	1.72
QD2K	Flow to DI on Camino Del Llano	90	0.35	0.42	0.48
QD2L	Flow to DI on Camino Del Llano	90	0.61	0.71	0.82
QD2M	Flow to DI on Camino Del Llano	90	1.93	2.26	2.59
QD2N	Flow to DI on Camino Del Llano	96	0.28	0.32	0.35
QD2O	Flow to DI on Camino Del Llano	90	1.60	1.87	2.15
QD2P	Flow to DI on Calle Del Girasol	90	1.58	1.85	2.13
QD2Q	Flow to DI on Avenida De Los Prados	90	2.16	2.53	2.91
QD2R	Flow to DI on Avenida De Los Prados	90	1.79	2.09	2.40
QD2S	Flow to DI on Calle Campo	90	2.17	2.54	2.92
QD2T	Flow to DI on Calle Campo	91	5.29	6.16	7.04
QD2U	Flow on South Meadows Road	97	0.57	0.65	0.72
QD2V	Flow on South Meadows Road	97	0.33	0.37	0.41
QD2W	Flow on South Meadows Road	93	0.24	0.28	0.32
QD2X	Outflow from Pond	74	2.93	3.90	4.97
QD3	Offsite basin entering the site from the north	81	0.95	1.17	1.41

All storm water from the development will flow to the north and west into and out of a series of rain gardens along the roadways before draining into drop inlets at the bottom most basins. The rain gardens were modeled in the Developed Conditions draining calculations by lowering the Manning's "n" roughness coefficient of a portion of each subbasin to increase its time of concentration. This runoff then spills into the Water Catchment Area via an energy dissipating landscape feature at the end of the road. Energy dissipator sizing and calculations are presented in Appendix D.

The developed conditions peak flows exceed the pre-developed condition peak flows. A comparison of Table 4 and Table 6 indicates that without additional runoff mitigating measures, the development of the project will have an impact on stormwater runoff due to the grading of the site and the additional impervious areas. In order to control this excess runoff, the storm water management plan provides for additional absorption and attenuation features on top of the roadside rain gardens via a Water Catchment Area north of the site and west of the park.

The Water Catchment Area has been designed to temporarily store and attenuate runoff as well as serve as a landscaped raingarden and bioretention area. Runoff in this area will first enter the energy dissipating landscape feature to slow down runoff to non-erosive velocities. After the runoff is dissipated, it will then spread to the raingarden plantings and pasture grass areas to the north and west. This is the first stage of the Water Catchment Area. As these areas fill up to a depth of 6", excess stormwater will then flow over a weir into the second stage of the pond. This weir is strategically located to keep velocities in the raingarden area near zero. The second stage on the Water Catchment Area consists of bioretention seeding and will fill to a depth of 12" before spilling over a second weir into the third stage of the pond. The third stage consists of another bioretention seeding area, a controlled outlet structure, and an emergency overflow weir. The overall Water Catchment Area is less than 3 feet deep and provides a hydraulic storage capacity of 43,588 cubic feet. All stages of the Water Catchment Area allow for infiltration based on the percolation rates in the geotechnical report prepared for this development. The controlled outlet structure is designed to attenuate the peak flows to pre-developed conditions. The Water Catchment Area is shown in Figure 11, Appendix A.

The flow routing calculations are presented in Appendix E. A comparison of developed and pre-developed condition peak flow values for each of the analyzed storm frequencies is presented in Table 7.

TABLE 7 PEAK RUNOFF FROM SITE		
Frequency	Pre-developed Condition (CFS)	Routed Developed Condition (CFS)
Q25	22.20	24.51
Q50	27.78	26.68
Q100	33.54	27.60

With the exception of a minor increase for the 25-year storm frequency, the total flow exiting the site is reduced in the routed developed condition for all storm frequencies, with a substantial reduction for the 100- year storm frequency, in accordance with the City of Santa Fe Terrain Management Requirements. The total hydraulic volume required to attenuate the 100-year storm is 35,861 cubic feet. As a result of changes in precipitation due to climate change, a 1,000-year check storm was modeled to verify the Water Catchment Area 's hydraulic capacity. The Water Catchment Area does not overtop during a 1,000-year event.

C. Floodplains

The flooding situation in the City of Santa Fe has been investigated by the Federal Insurance Administration (FIA). Their studies are contained in a report titled "Flood Insurance Study, Santa Fe County, New Mexico and Incorporated Areas." The FEMA Flood Plain Map Number 35049C0393D, dated June 17, 2008, indicates that all development is outside of the 1% Annual Chance Flood of being equaled or exceeded in any given year (base flood). The Flood Insurance Map Study (FIS) was revised December 4, 2012. Not all Flood Insurance Rate Maps were updated. According to the FEMA Flood Map Center, Map Number 35049C0393D, from the 2008 FIS, is the most current. The Flood Insurance Rate Map (FIRM) is shown in Figure 12, Appendix A.

V. GRADING AND EROSION CONTROL PLAN

Grading plans have been prepared for this development. Included within these plans are existing road profiles, proposed profiles, and typical roadway cross-sections. Roadway construction methods to be utilized for this development will be pursuant to the 2019 New Mexico Department of Transportation Specifications for Road and Bridge Construction.

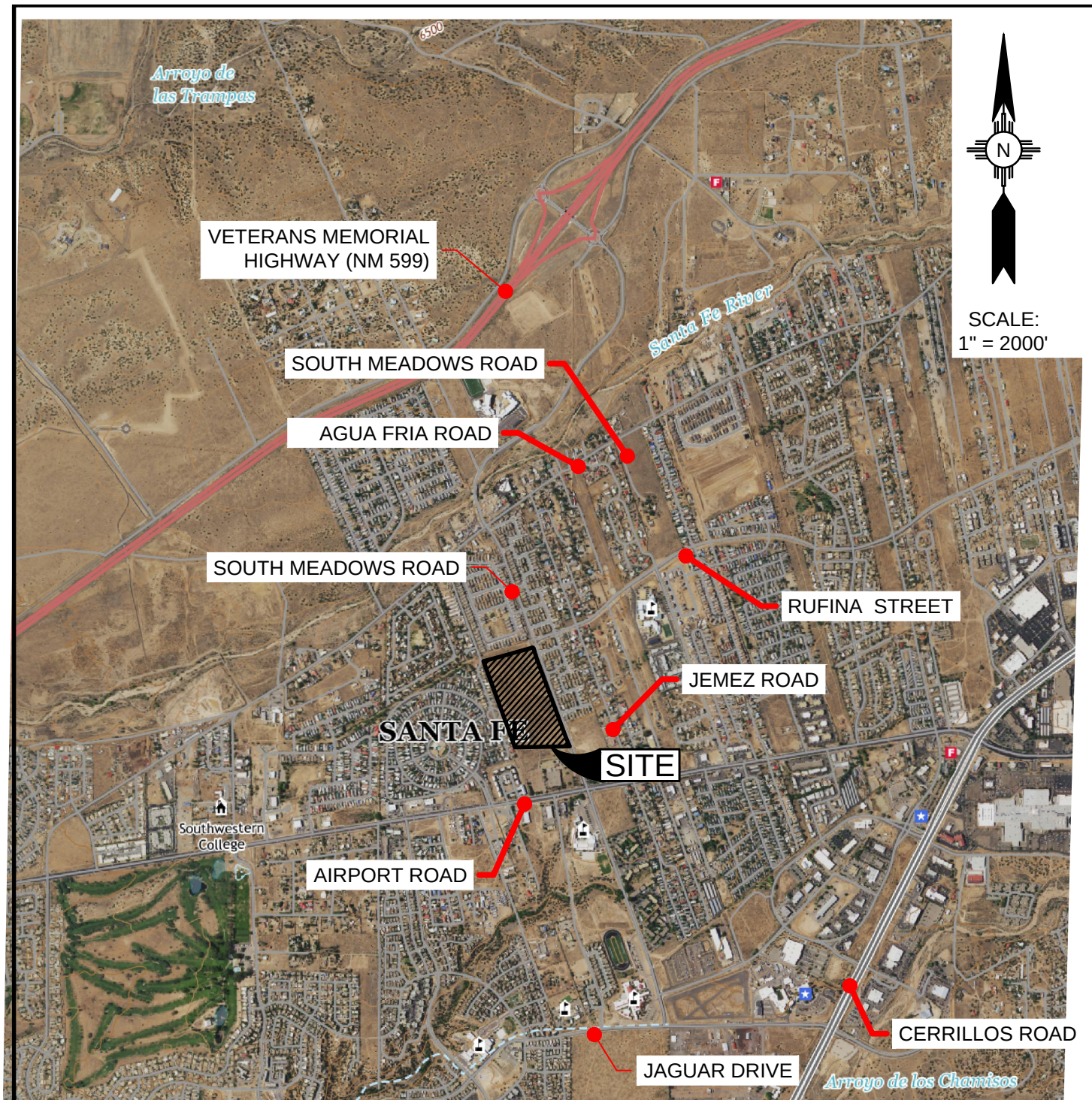
The grading activity at the site will require cut and fill operations. Utility installation will be scheduled to coincide with roadway grading and will be located with the roadway section and parkway.

The following plans will be utilized for controlling erosion and drainage through construction:

- A. The limits of construction will be clearly delineated to avoid unnecessary disturbance of adjacent areas prior to construction.
- B. Cut and fill slopes will be no steeper than:
 - Fills - 1 vertical on 3 horizontals
 - Cuts - 1 vertical on 2 horizontals
- C. Roadway drainage structures (culverts, rain gardens, ponds, check dams, and repairs to existing structures) will be constructed in conjunction with grading. No changes in the drainage ways or floodplain will be permitted except as shown on the drainage plans.
- D. Stabilization of cut or fills will begin soon after construction at the optimum time for plant survival, according to an approved landscape plan.
- E. The project will be inspected on a regular basis and additional measures such as silt fences, borrow ditches, etc., will be utilized as required to meet field conditions and to control erosion.

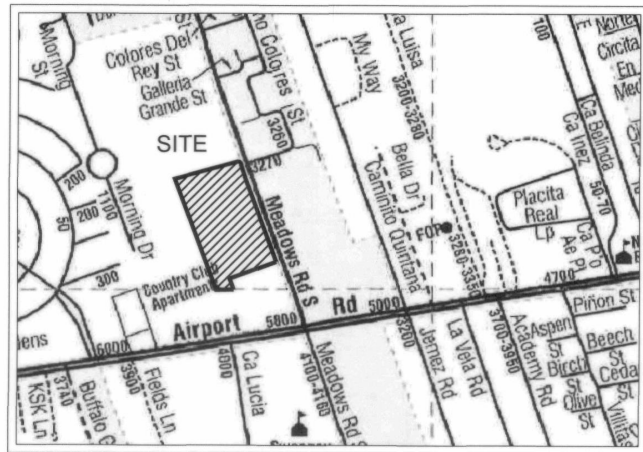
APPENDIX A

FIGURES



REFERENCE:
U.S.G.S. QUADRANGLE MAPS ENTITLED
"AGUA FRIA, NM" DATED 2020

VICINITY MAP
FIGURE 1



VICINITY MAP (NTS)

ADJOINER INFORMATION

LETTER	OWNER	DOC#-BK/PG
A	SANTA FE COUNTY	
B	MORRISON	1763014
C	WUNNICKE	1292/707A
D	ALDERETE	1024/371
E	WINN	1779532
F	TORREZ	698/877
G	RUCKEL	2121/401
H	MOUTON	638/554
I	VRABEL	714/185
J	MENIKTOS	1887191
K	MONTOYA	1919071
L	RABI REV. TRUST	1518940
M	CITY OF SANTA FE	1310905
N	MICHAEL	1742279
O	MARYOL	1312187
P	OKEEFE	953/522
Q	LOPEZ	1785159
R	DEWITT	1048/350
S	ENGLERT	1851668
T	SHERILL	1893963
U	PACHECO	1821934

PLAT REFERENCE

PLAT REFERENCE#1

"BOUNDARY SURVEY PREPARED FOR PAUL & JOANNE PEPPARD" BY GARY E. DAWSON NMPLS#7014 AND FILED IN PLAT BOOK 478, PG. 32 IN THE OFFICE OF THE SANTA FE COUNTY CLERK RECORD DATA SHOWN IN ().

PLAT REFERENCE#2

"BOUNDARY SURVEY PREPARED FOR PAUL & JOANNE PEPPARD" BY GARY E. DAWSON NMPLS#7014 AND FILED IN PLAT BOOK 402, PG. 14 IN THE OFFICE OF THE SANTA FE COUNTY CLERK RECORD DATA SHOWN IN ().

PLAT REFERENCE#3

"REVISED BOUNDARY SURVEY PREPARED FOR PAUL & JOANNE PEPPARD" BY GARY E. DAWSON NMPLS#7014 AND FILED IN PLAT BOOK 296, PG. 14 IN THE OFFICE OF THE SANTA FE COUNTY CLERK RECORD DATA SHOWN IN ().

PLAT REFERENCE#4

"SOUTH MEADOWS LOT SPLIT" BY GARY E. DAWSON NMPLS#7014 AND FILED IN PLAT BOOK 148, PG. 40 IN THE OFFICE OF THE SANTA FE COUNTY CLERK RECORD DATA SHOWN IN ().

PLAT REFERENCE#5

"CITY OF SANTA FE RUFINA STREET ROW PROJECT PHASE II C.I.P. NO.826" BY ALARID & ASSOCIATES, INC. NMPLS#5338 AND NOT RECORDED. RECORD DATA SHOWN IN ().

PLAT REFERENCE#6

"AMENDED PLAT OF THE JIMENEZ SUBDIVISION TRACT C & D" BY MORRIS A. APODACA NMPLS#5300 AND FILED IN PLAT BOOK 146, PG. 23 IN THE OFFICE OF THE SANTA FE COUNTY CLERK RECORD DATA SHOWN IN ().

SURVEYORS CERTIFICATE

I HEREBY CERTIFY THAT THIS PLAT AND THE NOTES HEREON ARE AN ACCURATE DELINEATION OF A FIELD SURVEY COMPLETED BY ME OR UNDER MY DIRECTION ON JUNE 20TH, 2023, AND ARE TRUE AND CORRECT TO THE BEST OF MY KNOWLEDGE AND BELIEF, AND MEET THE MINIMUM STANDARDS FOR PROFESSIONAL LAND SURVEYORS PRACTICING IN NEW MEXICO.

RICHARD A. CHATROOP

N.M.P.L.S.#11011



DEDICATION AND AFFIDAVIT

KNOW ALL MEN BY THESE PRESENTS THAT THE UNDERSIGNED OWNER(S), HAVE CAUSED TO BE DIVIDED AND DEDICATED THOSE LANDS SHOWN HEREON THIS DIVISION AND DEDICATION IS MADE WITH THE FREE CONSENT AND IN ACCORDANCE WITH THE WISHES AND DESIRES OF SAID OWNER(S).

THE AREA DESIGNATED AS CITY OF SANTA FE PARK (5.12 AC.±) IS HEREBY DEDICATED TO THE CITY OF SANTA FE FEE SIMPLE.

THIS DIVISION AND DEDICATION CONTAINS 19.53 AC.±, AND LIES WITHIN THE PLANNING AND PLATTING JURISDICTION OF THE CITY OF SANTA FE, NEW MEXICO.

EASEMENTS ARE VACATED AS SHOWN.

by Samie Saramillo
MICHAEL D. LOFTIN, EXECUTIVE DIRECTOR HOMEWISE INC.

STATE OF NEW MEXICO

SS

COUNTY OF SANTA FE

THE FOREGOING INSTRUMENT WAS SWORN, ACKNOWLEDGED AND

SUBSCRIBED BEFORE ME BY

MICHAEL D. LOFTIN, EXECUTIVE DIRECTOR HOMEWISE INC.

THIS 1 DAY OF November, 2023, Samie Saramillo
MY COMMISSION EXPIRES 7/1/2024 NOTARY PUBLIC

CROSSHATCHED AREA
8' WIDE UTILITY &
SIDEWALK EASEMENT
PL.BK.478, PG.032
PLAT REF.#1
TO BE VACATED
THIS DOCUMENT

R=18.00'
L=27.56'
DELTA=87°42'55"
CH=N 61°41'32" W, 24.94

5338 STA.42+11.84
5338 STA.38+49.42

14735 STA.74+25.31' E

S 20°01'05" E 18.68

1/2" RBR.

14735 STA.38+49.42

N 20°14'33" W 204.81

LOT 1 (B)

LOT 2 (C)

LOT 3 (D)

LOT 4 (E)

LOT 5 (F)

LOT 6 (G)

LOT 7 (H)

LOT 8 (I)

LOT 9 (J)

LOT 10 (K)

LOT 11 (L)

LOT 1 (N)

LOT 2 (O)

LOT 3 (P)

LOT 4 (Q)

LOT 5 (R)

LOT 6 (S)

LOT 7 (T)

LOT 8 (U)

ROAD, UTILITY & DRAINAGE EASEMENT
PL.BK.146, PG.023
PLAT REF.#6

15' WIDE UTILITY & EFFLUENT DRAINAGE EASEMENT
PL.BK.146, PG.023
PLAT REF.#6

ROAD, UTILITY & DRAINAGE EASEMENT
PL.BK.146, PG.023
PLAT REF.#6

15' WIDE UTILITY & EFFLUENT DRAINAGE EASEMENT
PL.BK.146, PG.023
PLAT REF.#6

CROSSHATCHED AREA
N/F NAGLE OR TRACT 2
PEPPARD FAMILY TRUST
15' WIDE PARCEL AS PER
WARRANTY DEED BK.254,
PAGE 844, (NAGLE) &
PLAT BK.574, PGS. 39/40
(PEPPARD FAMILY TRUST)
DENTONES DISCREPANCY OF
ADJOINER REFERENCES
PLAT REFS.#1, #2 & #3
WITH PLAT REFS.#4 & #5

TRACT 4-B
CITY OF SANTA FE PARK
DEDICATED THIS DOCUMENT
5.12 AC.±
3706 South Meadows Road

TRACT 4-A
14.41 AC.±
3730 South Meadows Road

10' WIDE UTILITY EASEMENT
PL.BK.478, PG.032
PLAT REF.#1

10' WIDE UTILITY EASEMENT
PL.BK.478, PG.032
PLAT REF.#1

CROSSHATCHED AREA
10' WIDE UTILITY EASEMENT
AS SHOWN
PL.BK.478, PG.032
PLAT REF.#1
TO BE VACATED
BY THIS INSTRUMENT

CROSSHATCHED AREA
N/F NAGLE OR TRACT 2
PEPPARD FAMILY TRUST
15' WIDE PARCEL AS PER
WARRANTY DEED BK.254,
PAGE 844, (NAGLE) &
PLAT BK.574, PGS. 39/40
(PEPPARD FAMILY TRUST)
DENTONES DISCREPANCY OF
ADJOINER REFERENCES
PLAT REFS.#1, #2 & #3
WITH PLAT REFS.#4 & #5

N/F HOMEWISE INC.
TRACT 3

N/F EBERLINE INSTRUMENT CORP
TRACT A1
BK.254, PG.844

VACATION OF EASEMENT OF WAY

KNOW ALL MEN BY THESE PRESENTS THAT THE UNDERSIGNED OWNER(S), HAVE CAUSED TO BE VACATED A CERTAIN EASEMENT OF WAY FOR A 15' WIDE MAINTENANCE EASEMENT FOR SEWER INTERCEPTOR LINES GRANTED BY DEED FOR EASEMENT OF WAY IN BOOK 175 MISC., PAGE 497, AS RECORDED IN THE OFFICE OF THE SANTA FE COUNTY CLERK ON FEB. 20TH 1981. EASEMENT VACATED ONLY WITHIN THE LIMITS OF TRACTS 3, 4A & 4B SHOWN HEREON.

THIS VACATION OF EASEMENT LIES WITHIN THE PLANNING AND PLATTING JURISDICTION OF THE CITY OF SANTA FE, NEW MEXICO.

PUBLIC UTILITY EASEMENTS

PUBLIC UTILITY EASEMENTS shown on this plat are granted for the common and joint use of:

A. Public Service Company of New Mexico

corporation, (PNM Electric) for installation, maintenance, and service of overhead and underground electrical lines, transformers, and other equipment and related facilities reasonably necessary to provide electrical services.

B. New Mexico Gas Company for installation, maintenance, and service of natural gas lines, valves and other equipment and facilities reasonably necessary to provide natural gas services.

C. West Corporation d/b/a CenturyLink QC for the installation, maintenance, and service of such lines, cable, and other related equipment and facilities reasonably necessary to provide ground communication services, including but not limited to, above ground pedestals and closures. This plat has been approved for easement purposes only. The signing of this plat does not in any way guarantee telephone services to the subdivision.

D. Cable TV for the installation, maintenance, and service of such lines, cable, and other related equipment and facilities reasonably necessary to provide Cable services.

Included, is the right to build, rebuild, construct, reconstruct, locate, relocate, change, remove, replace, modify, renew, operate and maintain facilities for purposes described above, together with free access to, from, and over said easements, with the right and privilege of going upon, over and across adjoining lands of Grantor for the purposes set forth herein and with the right to utilize the right of way and easements to extend services to customers of Grantee, including sufficient working area space for electric transformers, with the right and privilege to trim and remove trees, shrubs or bushes which interfere with the purposes set forth herein.

No building, sign, pool (aboveground or subsurface), hot tub, concrete or wood pool decking, or other structure shall be erected or constructed on said easements, nor shall any well be drilled or operated thereon. Property owners shall be solely responsible for correcting any violations of National Electrical Safety Code by construction of pools, decking, or any structures adjacent to or near easements shown on this plat.

Easements for electric transformer/switchgears, as installed, shall extend ten (10) feet in front of transformer/switchgear doors and five (5) feet on each side.

Disclaimer: In approving this plat, Public Service Company of New Mexico (PNM), New Mexico Gas Company (NMGC) and West Corporation d/b/a CenturyLink (QWEST) did not conduct a Title Search of the properties shown hereon. Consequently, PNM, NMGC and QWEST do not waive or release any easement or easement rights which may have been granted by prior plat, replat or other document and which are not shown on this plat.

Easement Vacation: The utility companies listed below hereby release, waive, quitclaim and discharge their right, title and interest to the present owner(s), and their assigns, in the Public Utility Easement as shown to be vacated on this plat. As to all property covered by such easement, which is not specifically described herein, the right, title, and interest of the following companies shall not be affected by this release.

UTILITIES APPROVAL

Frank A. Guyon 10/24/2023
NEW MEXICO GAS COMPANY DATE

Charlette Diana Gonzalez 10/30/23
PNM ELECTRIC SERVICES DATE

Dan Dule 10-24-2023
QWEST CORP. d/b/a CENTURYLINK QC DATE

S. R. R. 10-31-23
COMCAST DATE

COUNTY OF SANTA FE
STATE OF NEW MEXICO
I hereby certify that this instrument was filed
for record on the 16 day of November A.D.
2023 at 10:31 o'clock A.M.
and was duly recorded in book 1038
page 15 of the records of
Santa Fe County.

Witness my Hand and Seal of Office
Katharine Clark
County Clerk, Santa Fe County, N.M.



FIGURE 2

COUNTY OF SANTA FE REVIEW

Nov. 6, 2023
COUNTY TREASURER DATE

CITY OF SANTA FE APPROVAL,

NOTES AND CONDITIONS:

Dee Bennesser 11/13/23
CITY ENGINEER FOR LAND USE DATE

Thye Th 11/16/2023
CITY PLANNER DATE

NOTES

- PROPERTY DEVELOPMENT IS REQUIRED TO COMPLY WITH APPLICABLE PROVISIONS OF CHAPTER 14, LAND DEVELOPMENT CODE, SFCC 1987 AND SUBSEQUENT AMENDMENTS.
- WASTEWATER UTILITY EXPANSION CHARGES (UEC) SHALL BE PAID AT THE TIME OF CONSTRUCTION PERMIT APPLICATION.
- NEW DEVELOPMENT SHALL HAVE WATER SUPPLY THAT MEETS FIRE FLOW REQUIREMENTS AS PER IFC OR INSTALL AN AUTOMATIC SPRINKLER SYSTEM.
- FIRE DEPARTMENT SHALL HAVE 150 FEET DISTANCE TO ANY PORTION OF THE BUILDING ON ANY NEW CONSTRUCTION.
- PRIOR TO ANY NEW CONSTRUCTION OR REMODELING ALL FIRE DEPARTMENT REQUIREMENTS MUST BE MET OR AUTOMATIC SPRINKLER SYSTEMS MAY BE REQUIRED.
- TRACT 4 LIES WITHIN ZONE "X" AREAS DETERMINED TO BE OUTSIDE OF THE 0.2% ANNUAL CHANCE FLOODPLAIN AS PER F.I.R.M. PANEL# 35049C03930 DATED 6/17/08.
- CITY APPROVAL OF THIS PLAT DOES NOT ALTER OR REMOVE ANY EASEMENTS, RESTRICTIONS, OR RESERVATIONS OF RECORD PERTAINING TO THE LAND HEREIN DESCRIBED.
- PARCELS 4-A AND 4-B ARE BOUND BY THE MASTER PLAN CONDITIONS AS OUTLINED IN LAND USE CASE #2022-5618, TITLED "3600 & 3740 SOUTH MEADOWS MASTER PLAN". THESE CONDITIONS WERE PROPOSED BY THE PLANNING COMMISSION TO THE GOVERNING BODY ON NOVEMBER 3, 2022 AND SUBSEQUENTLY ADOPTED AS CONDITIONS OF APPROVAL BY THE GOVERNING BODY ON FEBRUARY 1, 2023.
- ANY FORMAL DRAINAGE IMPROVEMENTS ACCEPTING STORMWATER FLOWS FROM THE FUTURE DEVELOPMENT OF TRACT 4-A WILL BE PLACED IN A DRAINAGE EASEMENT DEDICATED TO THE TRACT 4-A HOMEOWNERS ASSOCIATION WHO WILL BE RESPONSIBLE FOR THE MAINTENANCE OF SUCH IMPROVEMENTS IN ACCORDANCE WITH THE CITY'S ADOPTED DRAINAGE MAINTENANCE REQUIREMENTS AND SCHEDULE.

LEGEND AND NOTES

- DENOTES ELEC. TRANSFORMER
- DENOTES SANITARY SEWER MANHOLE
- DENOTES STORM SEWER MANHOLE
- ▲ DENOTES WATER MARKER
- ▲ DENOTES FIRE HYDRANT
- ▲ DENOTES GAS TEST
- △ DENOTES TEL./COM. RISER

- DENOTES POINT FOUND (AS NOTED)
- DENOTES MONUMENT FOUND

- DENOTES POINT SET THIS SURVEY
- DENOTES POINT CALCULATED

- X — X — DENOTES EDGE OF EASEMENT
- X — X — DENOTES FENCE LINE

1. BASIS OF BEARING IS FROM GSP OBSERVATION WGS84

LOT OF RECORD IS FROM
"LOT LINE ADJUSTMENT FOR HOMEWISE, TRACTS 3 AND 4"
BY RICHARD A. CHATROOP NMPLS#11011 AND FILED IN PLAT BOOK
898, PG.37, INSTR.#2004713 IN THE OFFICE OF THE SANTA FE
COUNTY CLERK.

2. THIS PLAT IS SUBJECT TO ANY EASEMENTS, RESTRICTIONS
AND COVENANTS OF RECORD.

PURPOSE STATEMENT: THE PURPOSE OF THIS
PLAT IS TO SUBDIVIDE TRACT 4 INTO 2 LOTS

LOT SPLIT
FOR
HOMEWISE, INC.
OF
TRACT 4

UPC# 1-048-096-450-107-TRACT 4

LYING WITHIN SECTION 1, T16N, R8E, NMPM,
CITY & COUNTY OF SANTA FE, NEW MEXICO.

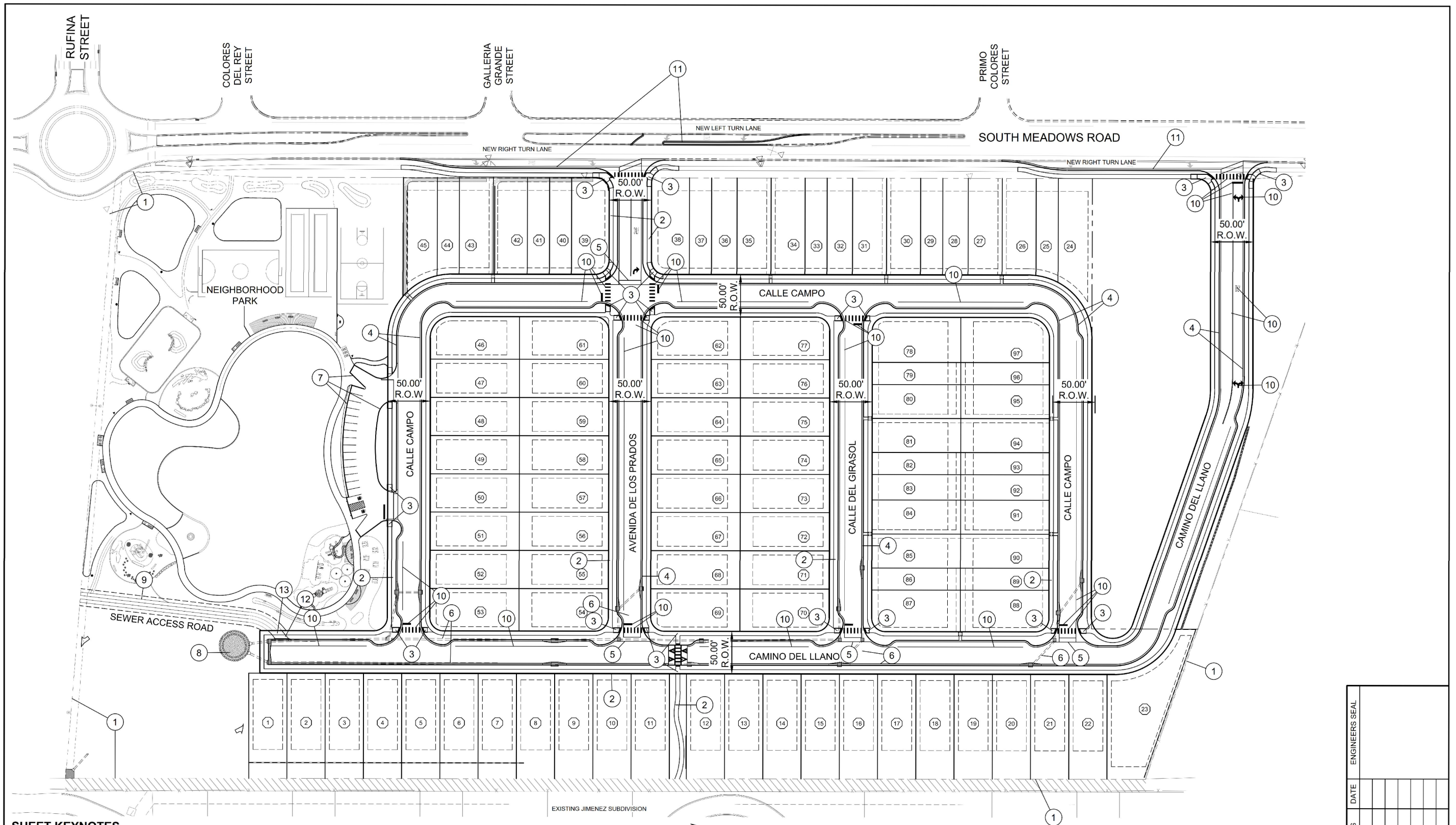
RICK CHATROOP
PROFESSIONAL LAND SURVEYOR

NEW MEXICO REGISTRATION NO. 11011
(505) 470-0037 110 WAGON TRAIL RD. CERRILLOS, NM. 87010

INDEXING INFORMATION FOR THE COUNTY CLERK

OWNER: HOMEWISE, INC.

LYING WITHIN SECTION 1, T16N, R8E, NMPM,
CITY & COUNTY OF SANTA FE, NEW MEXICO.



SHEET KEYNOTES

- | | | |
|--|--|--|
| 1 PROPERTY LINE | 6 PROPOSED STORM SEWER
SEE SHEETS 8-10 TO 8-13 | 11 PROPOSED OFFSITE IMPROVEMENTS
SEE 12 SERIES SHEETS |
| 2 PROPOSED CONCRETE SIDEWALK
SEE SHEET 2-6 | 7 PROPOSED 6" STAND-UP CURB
SEE SHEET 2-6 | 12 MOUNTABLE CURB AND GUTTER
SEE SHEET 2-6 |
| 3 PROPOSED ADA ACCESSIBLE CURB RAMP
SEE SHEETS 13-6 TO 13-8 | 8 PROPOSED WATER CATCHMENT
SEE SHEET 8-13 | 13 THICKENED SIDEWALK,
SEE SHEET 2-6 |
| 4 PROPOSED 2' STANDARD CURB AND GUTTER,
SEE SHEET 2-6 | 9 PROPOSED SEWER ACCESS ROAD
SEE LANDSCAPING SHEETS | |
| 5 PROPOSED 5' TYPE II VALLEY GUTTER,
SEE SHEET 2-6 | 10 PROPOSED STRIPING
SEE 7 SERIES SHEETS | |

CIVIL SITE PLAN
SCALE: 1" = 50'

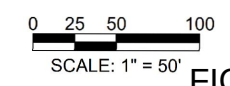


FIGURE 3

CITY REVIEW		
DEPARTMENT	SIGN-OFF	DATE
WASTEWATER MANAGEMENT DIV.	SIGNATURE ON FILE	
WATER SERVICES		
STREETS DIV.		
TRAFFIC DIV.		
FIRE		
LANDSCAPE		
SUBDIVISION		
ENGINEER		
CITY	USE	ONLY

SFE Santa Fe Engineering Consultants, LLC

1599 St. Francis Drive, Suite B
Santa Fe, N. M. 87505
(505) 982-2845 Fax (505) 982-2641
<http://www.SFENGR.com>

PRELIMINARY SUBDIVISION PLAT SUBMITTAL FOR LOS PRADOS 3600 AND 3740 SOUTH MEADOWS ROAD		
CIVIL SITE PLAN		
DATE: MARCH 2024	SCALE: 1" = 50'	SHEET: 1-3

REVISIONS	DATE	ENGINEERS SEAL

ADJOINER INFORMATION

LETTER	OWNER	DOC#-BK/PG
A	SANTA FE COUNTY	
B	MORRISON	1763014
C	WUNNICKE	1292/707A
D	ALDERETE	1024/371
E	WINN	1779532
F	TORREZ	698/877
G	RUCKEL	2121/401
H	MOUTON	638/554
I	VRABEL	714/185
J	MENIKTOS	1887191
K	MONTOYA	1919071
L	RABI REV.TRUST	1518940
M	SANTA FE COUNTY	1310905
N	MICHAEL	1742279
O	MARYOL	1312187
P	OKEFFE	953/522
Q	LOPEZ	1785159
R	DEWITT	1048/350
S	ENGLERT	1851668
T	SHERRILL	1893963
U	PACHECO	1821934

LOT, OPEN SPACE & RIGHT-OF-WAY AREA

LOT#	ADDRESS	AREA	LOT#	ADDRESS	AREA
1		5574 SQ.FT.±	50		4725 SQ.FT.±
2		5581 SQ.FT.±	51		4725 SQ.FT.±
3		5579 SQ.FT.±	52		4725 SQ.FT.±
4		5578 SQ.FT.±	53		5195 SQ.FT.±
5		5576 SQ.FT.±	54		5195 SQ.FT.±
6		5575 SQ.FT.±	55		4725 SQ.FT.±
7		5573 SQ.FT.±	56		4725 SQ.FT.±
8		5572 SQ.FT.±	57		4725 SQ.FT.±
9		5571 SQ.FT.±	58		4725 SQ.FT.±
10		5570 SQ.FT.±	59		4725 SQ.FT.±
11		5568 SQ.FT.±	60		4725 SQ.FT.±
12		5566 SQ.FT.±	61		5195 SQ.FT.±
13		5565 SQ.FT.±	62		5195 SQ.FT.±
14		5564 SQ.FT.±	63		4725 SQ.FT.±
15		5562 SQ.FT.±	64		4725 SQ.FT.±
16		5561 SQ.FT.±	65		4725 SQ.FT.±
17		5560 SQ.FT.±	66		4725 SQ.FT.±
18		5559 SQ.FT.±	67		4725 SQ.FT.±
19		5557 SQ.FT.±	68		4725 SQ.FT.±
20		5556 SQ.FT.±	69		5195 SQ.FT.±
21		5555 SQ.FT.±	70		5195 SQ.FT.±
22		5553 SQ.FT.±	71		4725 SQ.FT.±
23		10167 SQ.FT.±	72		4725 SQ.FT.±
24		5657 SQ.FT.±	73		4725 SQ.FT.±
25		3054 SQ.FT.±	74		4725 SQ.FT.±
26		4551 SQ.FT.±	75		4725 SQ.FT.±
27		4549 SQ.FT.±	76		4725 SQ.FT.±
28		3412 SQ.FT.±	77		5195 SQ.FT.±
29		2957 SQ.FT.±	78		5471 SQ.FT.±
30		4549 SQ.FT.±	79		2711 SQ.FT.±
31		4549 SQ.FT.±	80		4170 SQ.FT.±
32		3412 SQ.FT.±	81		4170 SQ.FT.±
33		2957 SQ.FT.±	82		2711 SQ.FT.±
34		4549 SQ.FT.±	83		2881 SQ.FT.±
35		4549 SQ.FT.±	84		4170 SQ.FT.±
36		3412 SQ.FT.±	85		4170 SQ.FT.±
37		2957 SQ.FT.±	86		2711 SQ.FT.±
38		5004 SQ.FT.±	87		5054 SQ.FT.±
39		5015 SQ.FT.±	88		5054 SQ.FT.±
40		2957 SQ.FT.±	89		2711 SQ.FT.±
41		2957 SQ.FT.±	90		4170 SQ.FT.±
42		4549 SQ.FT.±	91		4170 SQ.FT.±
43		4549 SQ.FT.±	92		2711 SQ.FT.±
44		2970 SQ.FT.±	93		2497 SQ.FT.±
45		4897 SQ.FT.±	94		4170 SQ.FT.±
46		5116 SQ.FT.±	95		4170 SQ.FT.±
47		4725 SQ.FT.±	96		2711 SQ.FT.±
48		4725 SQ.FT.±	97		5391 SQ.FT.±
49		4725 SQ.FT.±			

RUFINA STREET
VARIABLE WIDTH R/W

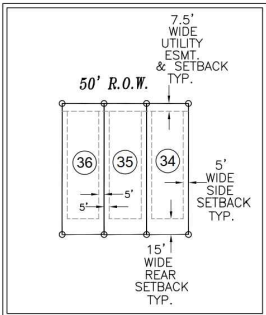
TRACT 4-B
CITY OF SANTA FE PARK
5.12 AC.±

CURVE	ARC	RADIUS	CHORD	LENGTH	DELTA
C1	78.54	50.00	N65°11'45"W	70.71	90°00'00"
C2	78.54	50.00	N24°48'15"E	70.71	90°00'00"
C3	56.46	50.00	S52°32'50"E	53.51	64°42'10"
C4	40.16	120.00	N79°23'34"E	39.98	19°10'38"
C5	25.13	16.00	S24°48'15"W	22.63	90°00'00"
C6	39.27	25.00	N65°11'45"W	35.36	90°00'00"
C7	46.76	75.00	N51°56'31"W	46.00	35°43'10"
C8	18.18	75.00	N27°08'20"W	18.13	13°53'12"
C9	25.13	16.00	S65°11'45"E	22.63	90°00'00"
C10	25.13	16.00	N24°48'15"E	22.63	90°00'00"
C11	25.13	16.00	S65°11'45"E	22.63	90°00'00"
C12	25.13	16.00	S24°48'15"W	22.63	90°00'00"
C13	25.13	16.00	N65°11'45"W	22.63	90°00'00"
C14	20.85	16.00	N32°28'21"E	19.41	74°39'48"
C15	5.55	16.00	S79°44'15"W	5.52	19°51'59"
C16	25.13	16.00	S24°48'15"W	22.63	90°00'00"
C17	25.13	16.00	N24°48'15"E	22.63	90°00'00"
C18	25.13	16.00	S65°11'45"E	22.63	90°00'00"
C19	25.13	16.00	S24°48'15"W	22.63	90°00'00"
C20	25.13	16.00	N65°11'45"W	22.63	90°00'00"
C21	39.27	25.00	N24°48'15"E	35.36	90°00'00"
C22	25.13	16.00	S65°11'45"E	22.63	90°00'00"
C23	9.03	75.00	N16°44'49"W	9.02	6°53'51"
C24	27.39	75.00	N02°50'16"W	27.23	20°55'14"
C25	81.40	75.00	N38°42'48"E	77.46	62°10'55"
C26	70.17	25.00	S10°36'26"E	49.30	160°49'22"
C27	92.71	75.00	S55°36'26"E	86.92	70°49'22"
C28	31.80	95.00	N79°23'34"E	31.65	19°10'38"
C29	48.53	145.00	N79°23'34"E	48.31	19°10'38"
C30	25.13	16.00	N24°48'15"E	22.63	90°00'00"
C31	5.34	50.00	S87°57'31"E	5.34	6°07'12"

LEGEND AND NOTES

- DENOTES ELEC. TRANSFORMER
- DENOTES SANITARY SEWER MANHOLE
- DENOTES STORM SEWER MANHOLE
- ▲ DENOTES WATER MARKER
- ▲ DENOTES FIRE HYDRANT
- ▲ DENOTES GAS TEST
- △ DENOTES TEL./COM. RISER
- DENOTES POINT FOUND (AS NOTED)
- ⊙ DENOTES MONUMENT FOUND
- DENOTES POINT SET THIS SURVEY
- DENOTES POINT CALCULATED
- DENOTES EDGE OF EASEMENT
- X---X--- DENOTES FENCE LINE

1. BASIS OF BEARING IS FROM GSP OBSERVATION WGS84
LOT OF RECORD IS FROM
"LOT SPLIT FOR HOMEWISE, INC. OF TRACT 4"
BY RICHARD A. CHATROOP NMPLS#11011 AND FILED IN PLAT BOOK
908, PG.15, IN THE OFFICE OF THE SANTA FE COUNTY CLERK.
2. THIS PLAT IS SUBJECT TO ANY EASEMENTS, RESTRICTIONS
AND COVENANTS OF RECORD.



TYPICAL LOT
SETBACKS & EASEMENTS

NORTH
SCALE 1"=80'

COUNTY OF SANTA FE } SS
STATE OF NEW MEXICO }
I hereby certify that this instrument was filed
for record on the _____ day of _____ A.D.
20____, at _____ o'clock _____ m.
and was duly recorded in book _____ of the records of
page _____ of Santa Fe County.
Witness my Hand and Seal of Office
Katharine Clark
County Clerk, Santa Fe County, N.M.

Deputy

PRELIMINARY PLAT
LOS PRADOS SUBDIVISION

UPC# 1-048-096-450-107-TRACT 4

LYING WITHIN SECTION 1, T16N, R8E, NMPM,
CITY & COUNTY OF SANTA FE, NEW MEXICO.

RICK CHATROOP
PROFESSIONAL LAND SURVEYOR
NEW MEXICO REGISTRATION NO. 11011
(505) 470-0037 110 WAGON TRAIL RD. CERRILLOS, NM. 87010

INDEXING INFORMATION FOR THE COUNTY CLERK

OWNER: HOMEWISE, INC.
LOCATION: LYING WITHIN SECTION 1, T16N, R8E, NMPM,
CITY & COUNTY OF SANTA FE, NEW MEXICO.

SHEET 2

FIGURE 4

CASE# 2024-

HOMESIDE SOUTH MEADOWS

N/F EBERLINE INSTRUMENT CORP
TRACT A1
BK.254, PG.844



EXISTING CONDITIONS
FIGURE 5

MANHOLE & STORM DRAIN
INFORMATION

LETTER	POSITION	ELEVATION/SIZE/DIR.
A	SAN. SEWER RIM	6477.3'
	INVERT	6471.0'-30"E-W
B	SAN. SEWER RIM	6473.8'
	INVERT	6466.7'-30"E-24"W
C	SAN. SEWER RIM	6473.3'
	INVERT	6466.2'-24"E-W
D	STORM GRATE TOP	6467.7'-8"N
	INVERT	6481.8"
E	STORM GRATE TOP	6484.0"
	INVERT	6480.0'-20"N
F	STORM GRATE TOP	6484.0"
	INVERT	6476.0'-16"E
G	STORM SEWER RIM	6484.4'
	INVERT	6474.0'-30"N-S
H	STORM SEWER RIM	6487.5'
	INVERT	6475.0'-6"E-W
I	STORM SEWER RIM	6485.9'
	INVERT	6475.6'-30"N-S
	INVERT	6477.6'-30"N-S
	STORM GRATE TOP	6478.0'-18"SE-SW
	INVERT	6485.2'
	INVERT	6479.2'-16"NE
	INVERT	6484.6'-4"E

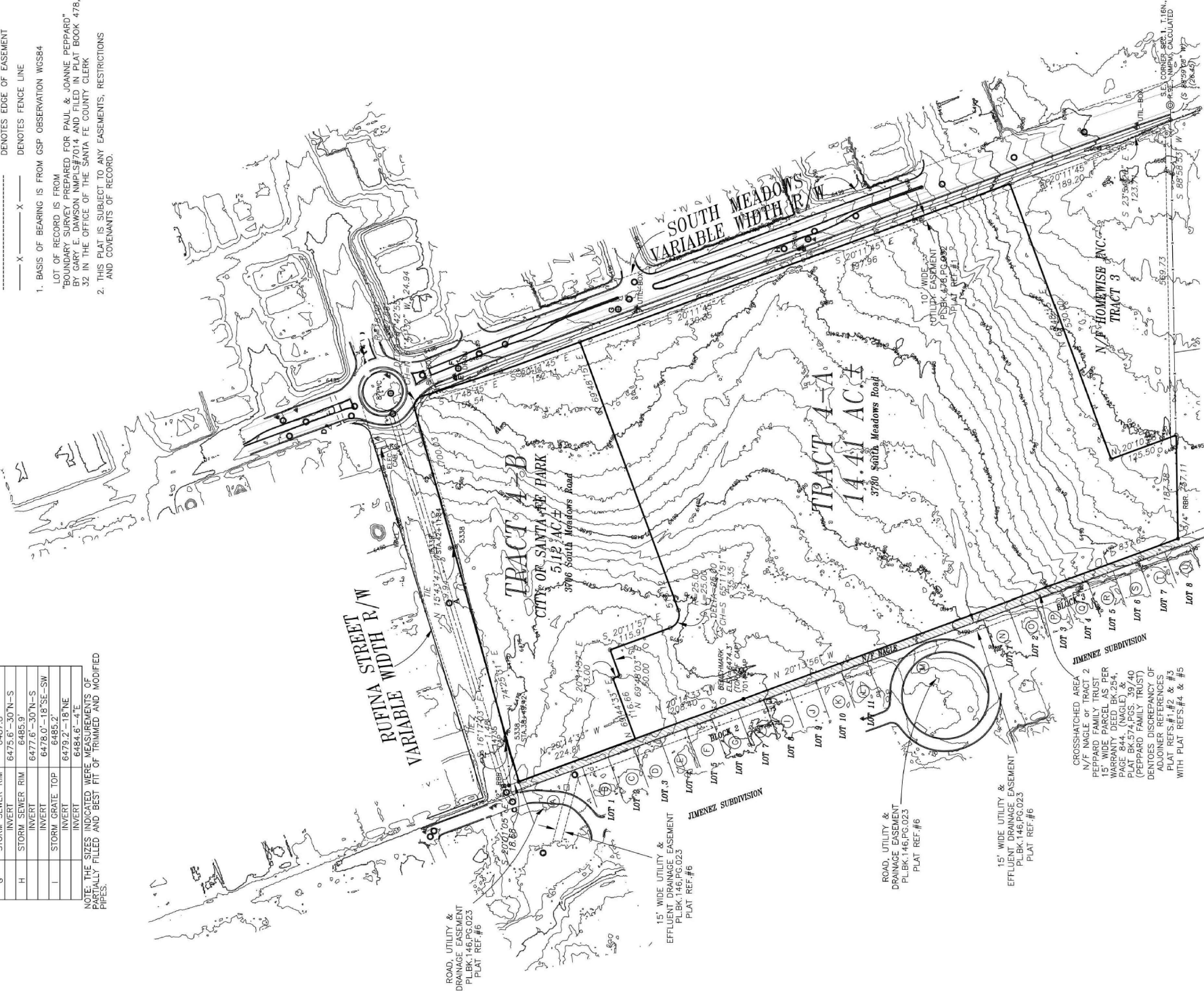
NOTE: THE SIZES INDICATED WERE MEASUREMENTS OF
PARTIALLY FILLED AND BEST FIT OF TRIMMED AND MODIFIED
PIPES.

TOPOGRAPHIC NOTES

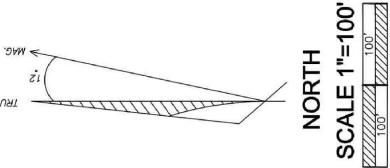
1. TOPOGRAPHIC INFORMATION PURCHASED DIRECTLY FROM
SANTA FE COUNTY GIS DEPARTMENT AND FIELD VERIFIED
TO MEET NATIONAL MAP ACCURACY STANDARDS.
2. ELEVATION DATUM IS AS SHOWN (BENCHMARK)
OPUS SOLUTION NAVD88 (COMPUTED USING GEOID18)
3. CONTOUR INTERVALS = 1'

LEGEND AND NOTES

- DENOTES ELEC. TRANSFORMER
 - DENOTES SANITARY SEWER MANHOLE
 - DENOTES STORM SEWER MANHOLE
 - ▲ DENOTES WATER MARKER
 - ▲ DENOTES FIRE HYDRANT
 - ▲ DENOTES GAS TEST
 - ▲ DENOTES TEL./COM. RISER
 - DENOTES POINT FOUND
(AS NOTED)
 - DENOTES MONUMENT FOUND
 - DENOTES POINT SET THIS SURVEY
 - DENOTES POINT CALCULATED
 - X --- DENOTES EDGE OF EASEMENT
 - X --- DENOTES FENCE LINE
1. BASIS OF BEARING IS FROM GSP OBSERVATION WCS84
LOT OF RECORD IS FROM
"BOUNDARY SURVEY PREPARED FOR PAUL & JOANNE PERPARD"
BY GARY E. DAWSON NMP/LS/7014 AND FILED IN PLAT BOOK 478, PG.
32 IN THE OFFICE OF THE SANTA FE COUNTY CLERK
2. THIS PLAT IS SUBJECT TO ANY EASEMENTS, RESTRICTIONS
AND COVENANTS OF RECORD.



N/F EBBELINE INSTRUMENT CORP
TRACT 41
BK.254, PG.84



SURVEYORS CERTIFICATE

I HEREBY CERTIFY THAT THIS PLAT AND THE NOTES HEREON
ARE AN ACCURATE DELINEATION OF A FIELD SURVEY COMPLETED
UNDER MY DIRECT SUPERVISION ON NOV 15TH, 2024, AND
TO REPRESENT THE INTERESTS OF THE PARTIES TO THE SURVEY.
OF TRACT 4" AS RECORDED IN PLAT BK. 908, PG. 015 ON 2/23/24, &
ARE TRUE AND CORRECT TO THE BEST OF MY KNOWLEDGE AND BELIEF,
AND MEET THE MINIMUM STANDARDS FOR PROFESSIONAL LAND SURVEYORS
PRACTICING IN NEW MEXICO.

RICHARD A. CHATROOP
N.M.P.L.S. #11011

COUNTY OF SANTA FE
STATE OF NEW MEXICO
I hereby certify that this instrument was filed
20 or record on the _____ day of _____ A.D. m.
and was duly recorded in book _____ of the records of
page _____ of the records of
Santa Fe County.
Witness my hand and Seal of Office
County Clerk, Santa Fe County, N.M.

Deputy

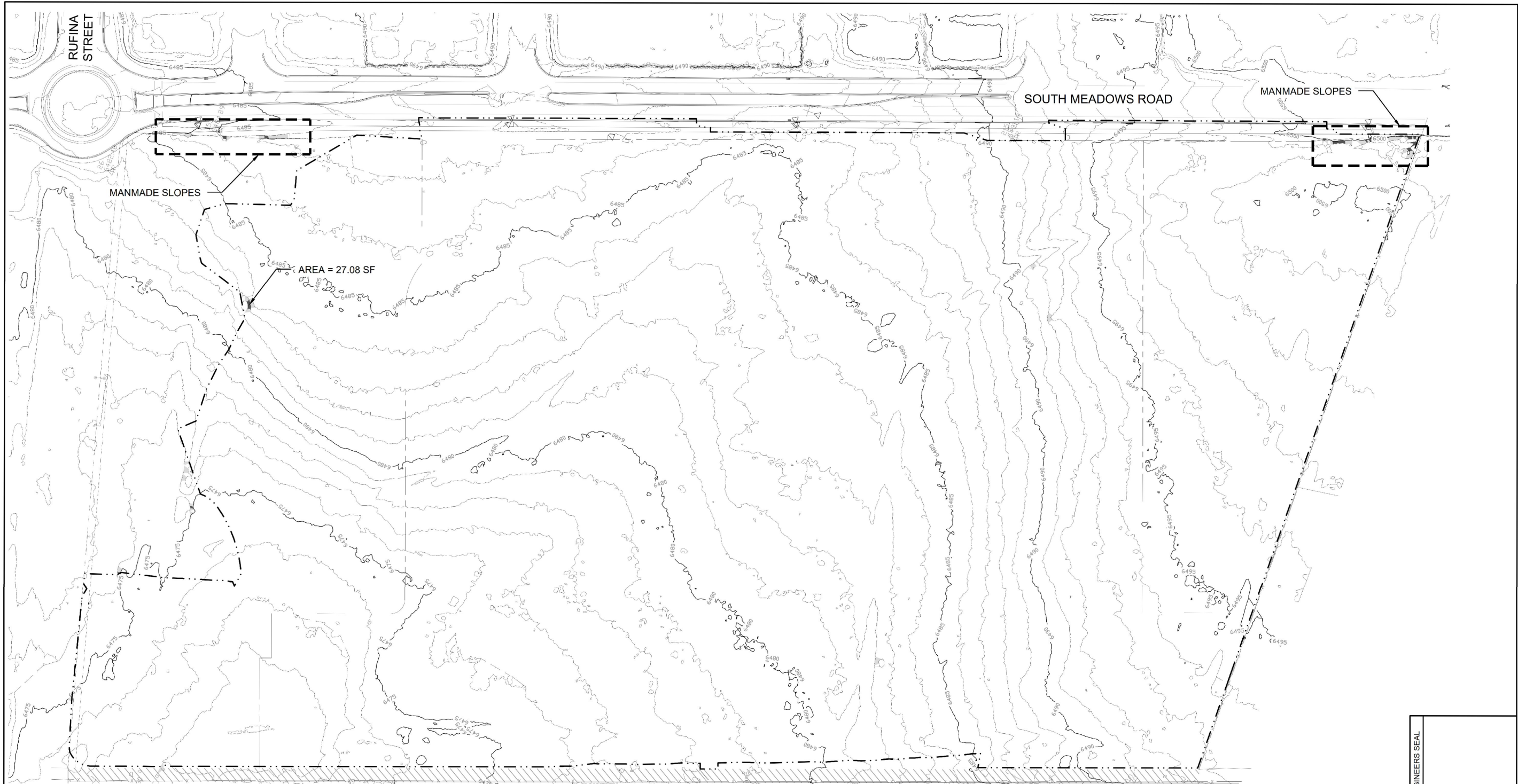
TOPOGRAPHIC PLAN
FOR
HOMEWISE
TRACTS 4A & 4B

LYING WITHIN SECTION 1, T16N, R8E, NMPM,
CITY & COUNTY OF SANTA FE, NEW MEXICO.

RICK CHATROOP
PROFESSIONAL LAND SURVEYOR
NEW MEXICO REGISTRATION NO. 11011
(505) 470-0037 110 WAGON TRAIL RD. CERRILLOS, NM. 87010

INDEXING INFORMATION FOR THE COUNTY CLERK
OWNER: HOMEWISE
LYING WITHIN SECTION 1, T16N, R8E, NMPM,
CITY & COUNTY OF SANTA FE, NEW MEXICO.

FIGURE 6



LEGEND

AREA OF 20% TO 30% SLOPE

AREA OF 30% AND GREATER SLOPE

GRADING LIMITS

ALL TOPOGRAPHIC INFORMATION IS PROVIDED BY
RICK CHATROOP

THIS SLOPE ANALYSIS IS BASED UPON 1 FOOT
CONTOUR INTERVALS

AREA OF 30% AND GREATER SLOPES DISTURBED
BY THIS PROJECT = 27.08 SF

SLOPE ANALYSIS MAP
SCALE: 1" = 50'
CONTOUR INTERVAL = 1'

CITY REVIEW		
DEPARTMENT	SIGN-OFF	DATE
WASTEWATER MANAGEMENT DIV.	SIGNATURE	ON FILE
WATER SERVICES		
STREETS DIV.		
TRAFFIC DIV.		
FIRE		
LANDSCAPE		
SUBDIVISION ENGINEER		
CITY	USE	ONLY

REVISED JUNE 2024
PER STAFF COMMENTS
CASE # 2024-7998

S F E C
**Santa Fe Engineering
Consultants, LLC**
1599 St. Francis Drive, Suite B
Santa Fe, N. M. 87505
(505) 982-2845 Fax (505) 982-2641
<http://www.SFENGR.com>

ENGINEERS SEAL

DATE

REVISIONS

PRELIMINARY SUBDIVISION PLAT
SUBMITTAL FOR LOS PRADOS
3600 AND 3740 SOUTH MEADOWS ROAD

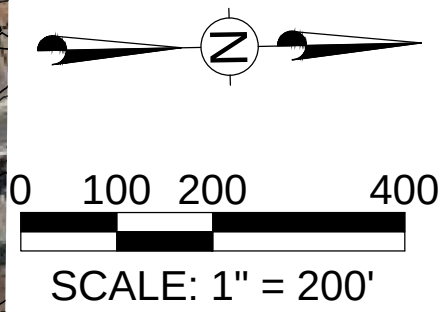
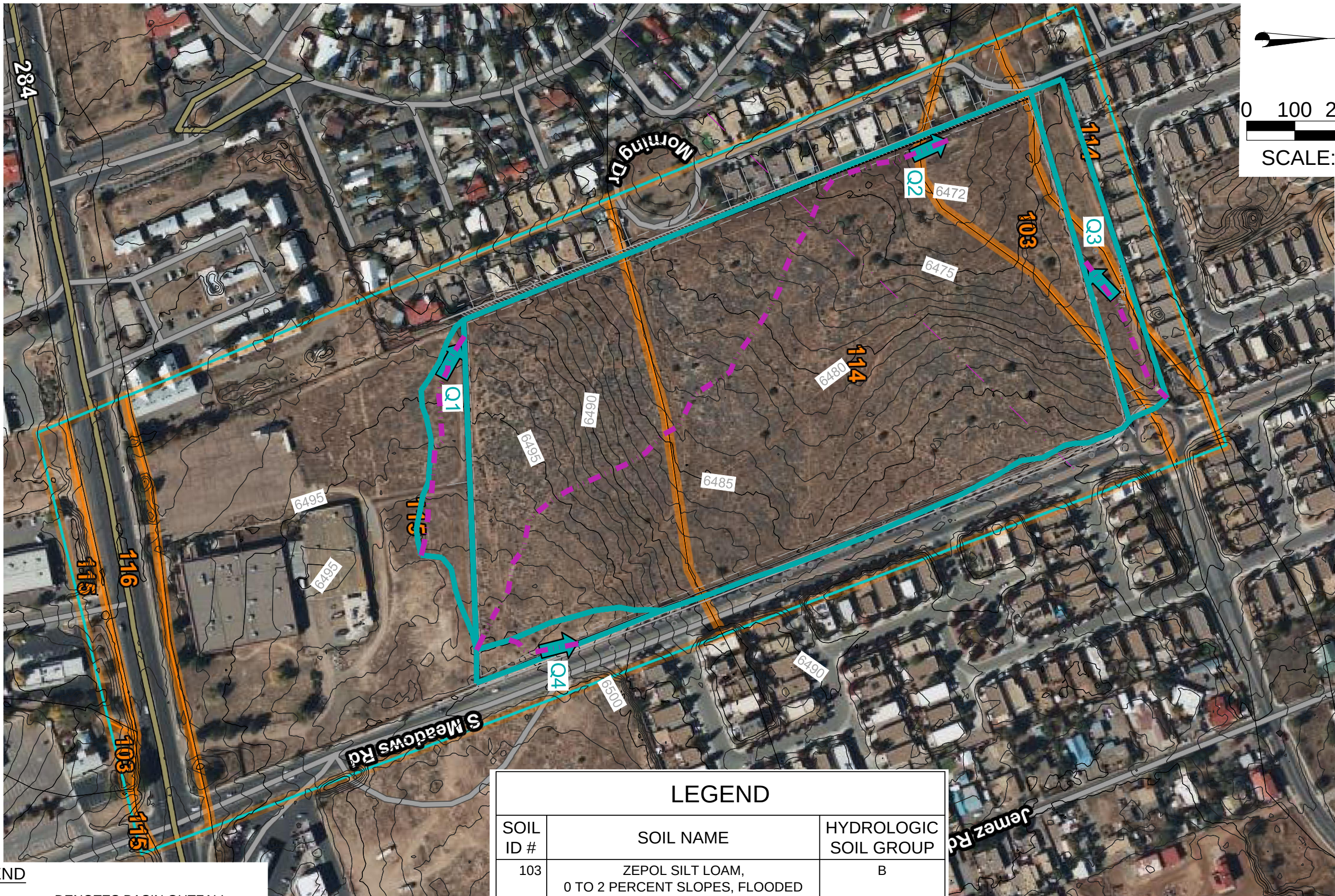
SLOPE ANALYSIS MAP

DATE:
MARCH 2024

SCALE:
1" = 50'

SHEET:
1-9

FIGURE 7

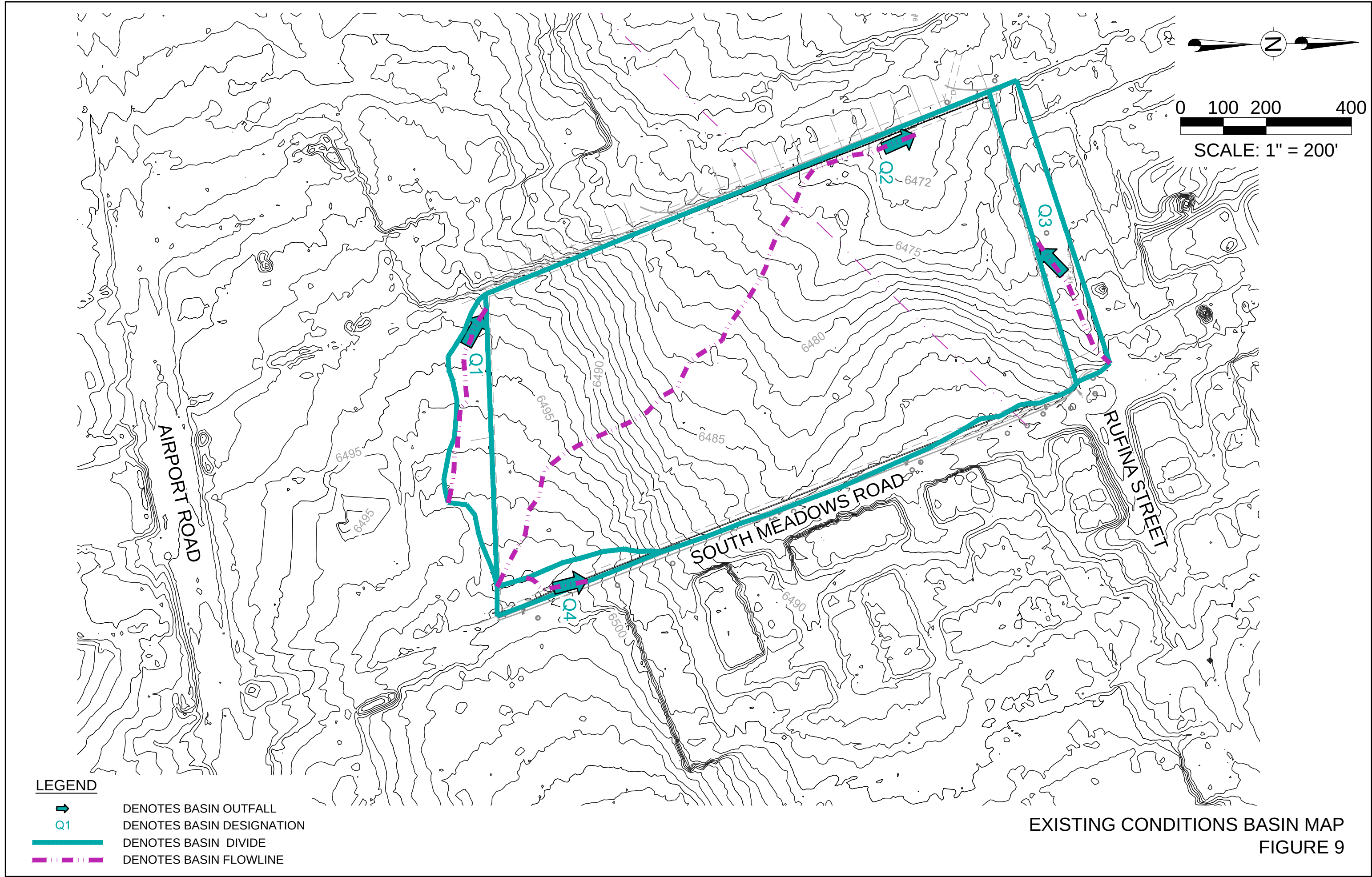


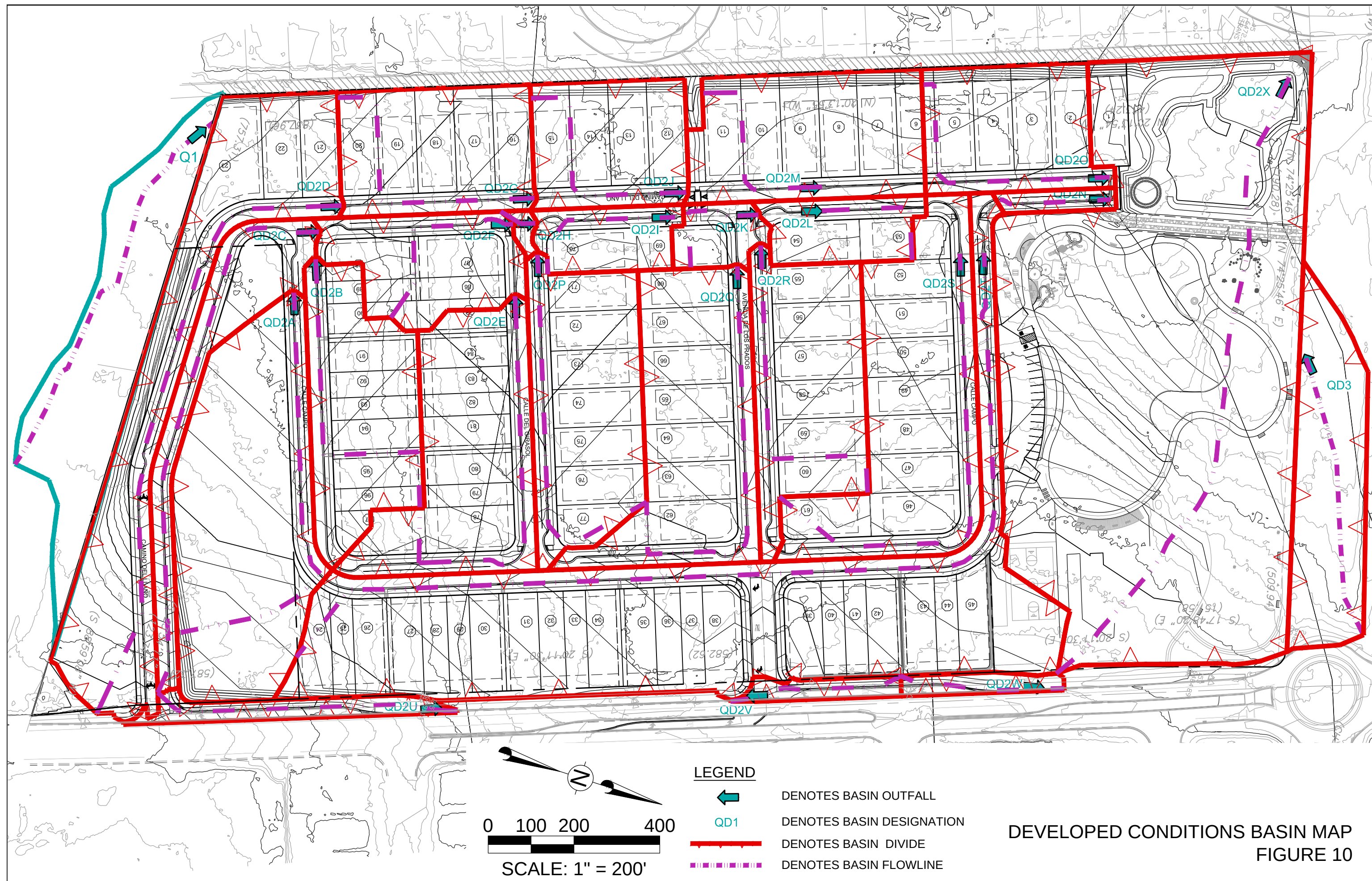
LEGEND

-  DENOTES BASIN OUTFALL
-  DENOTES BASIN DESIGNATION
-  DENOTES BASIN DIVIDE
-  DENOTES BASIN FLOWLINE

LEGEND		
SOIL ID #	SOIL NAME	HYDROLOGIC SOIL GROUP
103	ZEPOL SILT LOAM, 0 TO 2 PERCENT SLOPES, FLOODED	B
114	DEVARGAS-URBAN LAND COMPLEX, 1 TO 3 PERCENT SLOPES	C
115	PANKY-URBAN LAND COMPLEX, 1 TO 4 PERCENT SLOPES	C

SOILS MAP
FIGURE 8







PLANT SCHEDULE

SYMBOL	BOTANICAL / COMMON NAME	SYMBOL	BOTANICAL / COMMON NAME
TREES			
	EXISTING TREE TO REMAIN		DECIDUOUS SHRUBS
DECIDUOUS TREES			ARTEMISIA FRIGIDA FRINGED WORMWOOD
	ACER GRANDIDENTATUM 'MESA 6' MESA GLOW MAPLE		BAPTISIA AUSTRALIS FALSE INDIGO
	FORESTIERA NEOMEXICANA DESERT OLIVE		BOUTELOUA GRACILIS 'BLONDE AMBITION' BLONDE AMBITION BLUE GRAMA
	PLATANUS OCCIDENTALIS 'BISMARCK' NORTHERN ADVANCE® AMERICAN SYCAMORE		BUDDLEJA DAVIDII 'NANHO BLUE' NANHO BLUE BUTTERFLY BUSH
	PRUNUS ARMENIACA 'HARGLOW' HARGLOW SEMI-DWARF APRICOT		CALAMAGROSTIS X ACUTIFLORA KARL FOERSTER GRASS
	PRUNUS ARMENIACA 'SUNGOLD' SUNGOLD APRICOT		CARYOPTERIS X GLANDONENSIS 'BLUE MIST' BLUE MIST BLUEBEARD
	PRUNUS AVIUM 'STELLA' STELLA SWEET CHERRY		CHAMAEBATIARIA MILLEFOLIUM FERNBUSH
	PRUNUS CERASUS 'MONTMORENCY' MONTMORENCY CHERRY		ERICAMERIA NAUSEOSA RUBBER RABBITBRUSH
	QUERCUS GAMBELII VAR. GILA GILA GAMBEL OAK		MUHLENBERGIA RIGENS DEER GRASS
	QUERCUS TEXANA TEXAS RED OAK		PRUNUS BESSEYI 'POLIS' PAWNEE BUTTES® SAND CHERRY
	ULMUS PARVIFLORA 'EMER II' ALEE LACEBARK ELM		RHUS AROMATICA 'GRO-LOW' GRO-LOW FRAGRANT SUMAC
	ULMUS X 'FRONTIER' FRONTIER ELM		RHUS TRILOBATA THREE LEAF SUMAC
EVERGREEN TREES			RIBES AUREUM GOLDEN CURRANT
	JUNIPERUS SCOPULORUM ROCKY MOUNTAIN JUNIPER		RIBES RUBRUM 'RED LAKE' RED LAKE RED CURRANT
	PINUS FLEXILIS LIMBER PINE		SALVIA X SYLVESTRIS 'MAY NIGHT' MAY NIGHT SAGE
	PINUS NIGRA AUSTRIAN PINE	EVERGREEN SHRUBS	
	PINUS STROBUS WHITE PINE		ARCTOSTAPHYLOS X COLORADENSIS 'CHIEFTAIN' CHIEFTAIN MANZANITA
			ARTEMISIA TRIDENTATA BIG SAGEBRUSH
			CERCOCARPUS INTRICATUS LITTLELEAF MOUNTAIN MAHOGANY
			FALLUGIA PARADOXA APACHE PLUME
			NOLINA MICROCARPA BEARGRASS
SOD/SEED			
	TURFGRASS SOD: POA SEE FOR MORE DETAIL SHEET 4-00		
	BIORETENTION POND SEED MIX SEE SEED LIST ON SHEET 4-00.		
	REVEGETATION SEED MIX SEE SEED LIST ON SHEET 4-00.		
	RECLAMATION STEEP SLOPE SEEDING SEE SEED LIST ON SHEET 4-00.		
	WILDFLOWER MEADOW SEED MIX. SEE SEED LIST ON SHEET 4-00.		
	PASTURE SEED MIX. SEE SEED LIST ON SHEET 4-00.		

WATER CATCHMENT AREA
FIGURE 11

NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The community map repository should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where **Base Flood Elevations** (BFEs) and/or **floodways** have been determined, users are encouraged to consult the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables contained within the Flood Insurance Study (FIS) report that accompanies this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood elevation data presented in the FIS report should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

Coastal Base Flood Elevations shown on this map apply only landward of 0.0' North American Vertical Datum of 1988 (NAVD 88). Users of this FIRM should be aware that coastal flood elevations are also provided in the summary of Stillwater Elevations table in the Flood Insurance Study Report for this jurisdiction. Elevations shown in the Summary of Stillwater Elevations table should be used for construction and/or floodplain management purposes when they are higher than the elevations shown on this FIRM.

Boundaries of the **floodways** were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study report for this jurisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by flood control structures. Refer to Section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures in this jurisdiction.

The **projection** used in the preparation of this map was New Mexico State Plane, Central zone. The **horizontal datum** was NAD 83, GR80 spheroid. Differences in datum, spheroid, projection or State Plane zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the North American Vertical Datum of 1988. These flood elevations must be compared to structure and ground elevations referenced to the same **vertical datum**. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at www.ngs.noaa.gov or contact the National Geodetic Survey at the following address:

Spatial Reference System Division
National Geodetic Survey, NOAA
Silver Spring Metro Center
1315 East-West Highway
Silver Spring, Maryland 20910
(301) 713-3191

To obtain current elevation, description and/or location information for **bench marks** shown on this map, please contact the Information Services Branch of the National Geodetic Survey at (301) 713-3242, or visit their website at www.ngs.noaa.gov.

Base map information shown on this FIRM was provided by the U.S. Geological Survey and Bohannon-Huston Incorporated (BHI). Digital Orthophoto Quadrangles were produced at a scale of 1:12,000 from photography dated 1997. BHI orthophotos were produced at a scale of 1:12,000 from photography dated 2001.

Based on updated topographic information, this map reflects more detailed and up-to-date **stream channel configurations and floodplain delineations** than those shown on the previous FIRM for this jurisdiction. As a result, the Flood Profiles and Floodway Data tables for the Flood Insurance Study report may reflect stream channel distances that differ from what is shown on the map. Also, the road to floodplain relationships for unreviewed streams may differ from what is shown on previous maps.

Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

Please refer to the separately printed **Map Index** for an overview map of the county showing the layout of map panels; community map repository addresses; and a Listing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is located.

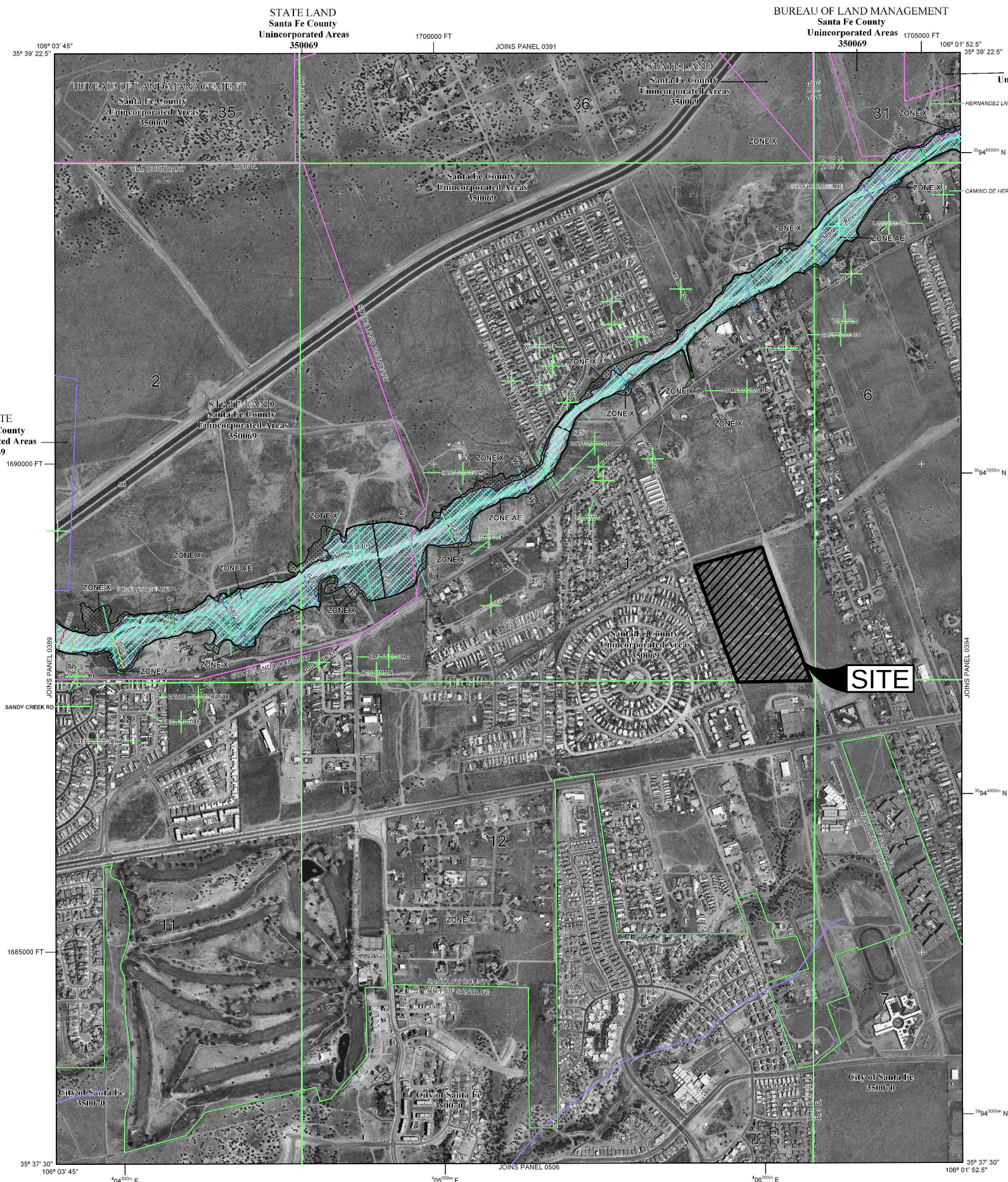
Contact the **FEMA Map Service Center** at 1-800-358-9616 for information on available products associated with this FIRM. Available products may include previously issued Letters of Map Change, a Flood Insurance Study report, and/or digital versions of this map. The FEMA Map Service Center may also be reached by Fax at 1-800-358-9620 and their website at www.msc.fema.gov.

If you have **questions about this map** or questions concerning the National Flood Insurance Program in general, please call 1-877-FEMA MAP (1-877-336-2627) or visit the FEMA website at www.fema.gov.

NOTE:

THE FLOOD INSURANCE STUDY (FIS) WAS REVISED DECEMBER 4, 2012. NOT ALL FLOOD INSURANCE RATE MAPS WERE UPDATED. ACCORDING TO THE FEMA FLOOD MAP CENTER, MAP NUMBER 35049C0393D WAS NOT UPDATED.

PRIVATE
Santa Fe County
Unincorporated Areas
350069



LEGEND

SPECIAL FLOOD HAZARD AREAS (SFHAs) SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD

The 1% annual chance flood (100-year flood), also known as the base flood, is the flood that has a 1% chance of being equaled or exceeded in any given year. The Special Flood Hazard Area is the area subject to flooding by the 1% annual chance flood. Areas of Special Flood Hazard include Zones A, AE, AH, AO, AR, AV, V, and VE. The Base Flood Elevation is the water-surface elevation of the 1% annual chance flood.

ZONE A No Base Flood Elevations determined.

ZONE AE Base Flood Elevations determined.

ZONE AH Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood Elevations determined.

ZONE AO Flood depths of 2 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also determined.

ZONE AR Special Flood Hazard Area formerly protected from the 1% annual chance flood by a flood control system that was subsequently identified. Zone AR indicates that the former flood control system is being restored to provide protection from the 1% annual chance or greater flood.

ZONE ASS Area to be protected from 1% annual chance flood by a Federal flood protection system under construction; no Base Flood Elevations determined.

ZONE VE Coastal flood zone with velocity hazard (wave action); Base Flood Elevations determined.

FLOODWAY AREAS IN ZONE AE

The floodway is the channel of a stream plus any adjacent floodplain areas that must be kept free of encroachment so that the 1% annual chance flood can be carried without substantial increases in flood heights.

OTHER FLOOD AREAS

ZONE X Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood.

OTHER AREAS

ZONE X Areas determined to be outside the 0.2% annual chance floodplain.

ZONE D Areas in which flood hazards are undetermined, but possible.

COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS

OTHERWISE PROTECTED AREAS (OPAs)

CBRS areas and OPAs are normally located within or adjacent to Special Flood Hazard Areas.

1% annual chance floodplain boundary

0.2% annual chance floodplain boundary

Floodway boundary

Zone D boundary

CBRS and OPA boundary

Boundary dividing Special Flood Hazard Areas of different Base Flood Elevations, flood depths or flood velocities.

Base Flood Elevation line and value; elevation in feet*

Base Flood Elevation value where uniform within zone; elevation in feet*

*Referenced to the North American Vertical Datum of 1988

Cross section line

Transect line

Geographic coordinates referenced to the North American Datum of 1983 (NAD 83), Western Hemisphere

1000-meter Universal Transverse Mercator grid values, zone 13

5000-foot grid ticks: New Mexico State Plane coordinate system, Central zone (FIPSZONE 1002), Transverse Mercator

Bench mark (see explanation in Notes to Users section of this FIRM panel).

ML5 River Mile

MAP REPOSITORIES

Refer to Map Repositories list on Map Index.

EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP PANEL

JUNE 17, 2008

EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL

For community map revision history prior to countywide mapping, refer to the Community Map History table located in the Flood Insurance Study report for this jurisdiction.

To determine if flood insurance is available in this community, contact your insurance agent or call the National Flood Insurance Program at 1-800-638-6620.



MAP SCALE 1" = 500'



NATIONAL FLOOD INSURANCE PROGRAM

PANEL 0393D

FIRM

FLOOD INSURANCE RATE MAP
SANTA FE COUNTY,
NEW MEXICO
AND INCORPORATED AREAS

PANEL 393 OF 1100

(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

CONTAINS:

COMMUNITY	NUMBER	PANEL	SUFFIX
SANTA FE, CITY OF	350070	0393	D
SANTA FE COUNTY (UNINCORPORATED AREAS)	350069	0393	D

Notice to User: The Map Number shown below should be used when citing map across the Community Number shown above should be used on insurance applications for the subject community.



MAP NUMBER

35049C0393D

EFFECTIVE DATE

JUNE 17, 2008

Federal Emergency Management Agency

FIGURE 12

APPENDIX B

EXISTING CONDITIONS DRAINAGE CALCULATIONS

WinTR-55 Current Data Description

--- Identification Data ---

User: CPC Date: 9/7/2022
 Project: LOS PRADOS Units: English
 SubTitle: Q1 EXISTING CONDITIONS Areal Units: Acres
 State: New Mexico
 County: Santa Fe
 Filename: K:\HOMEWISE SOUTH MEADOWS\Drainage\TR55\Q1 EX.w55

--- Sub-Area Data ---

Name	Description	Reach	Area(ac)	RCN	Tc
Q1		Outlet	1.04	81	.405

Total area: 1.04 (ac)

--- Storm Data --

Rainfall Depth by Rainfall Return Period

2-Yr (in)	-Yr (in)	-Yr (in)	25-Yr (in)	50-Yr (in)	100-Yr (in)	-Yr (in)
1.52	.0	.0	2.6	2.91	3.23	.0

Storm Data Source: User-provided custom storm data
 Rainfall Distribution Type: Type II
 Dimensionless Unit Hydrograph: <standard>

Watershed Peak Table

Sub-Area or Reach Identifier	25-Yr (cfs)	50-Yr (cfs)	100-Yr (cfs)
------------------------------------	----------------	----------------	-----------------

SUBAREAS

Q1	1.03	1.28	1.54
----	------	------	------

REACHES

OUTLET	1.03	1.28	1.54
--------	------	------	------

Hydrograph Peak/Peak Time Table

Sub-Area or Reach Identifier	25-Yr (cfs) (hr)	50-Yr (cfs) (hr)	100-Yr (cfs) (hr)
------------------------------------	------------------------	------------------------	-------------------------

SUBAREAS

Q1	1.03 12.13	1.28 12.13	1.54 12.14
----	---------------	---------------	---------------

REACHES

OUTLET	1.03	1.28	1.54
--------	------	------	------

CPC

LOS PRADOS
Q1 EXISTING CONDITIONS
Santa Fe County, New Mexico

Sub-Area Summary Table

Sub-Area Identifier	Drainage Area (ac)	Time of Concentration (hr)	Curve Number	Receiving Reach	Sub-Area Description
Q1	1.04	0.405	81	Outlet	

Total Area: 1.04 (ac)

Sub-Area Time of Concentration Details

Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wetted Perimeter (ft)	Velocity (ft/sec)	Travel Time (hr)
Q1							
SHEET	100	0.0079	0.150				0.344
SHALLOW	378	0.0115	0.050				0.061
Time of Concentration							.405
							=====

Sub-Area Land Use and Curve Number Details

Sub-Area Identifier	Land Use	Hydrologic Soil Group	Sub-Area Area (ac)	Curve Number
Q1	Herbaceous	(fair) C	1.04	81
Total Area / Weighted Curve Number			1.04	81
			====	==

WinTR-55 Current Data Description

--- Identification Data ---

User: CPC Date: 9/22/2022
 Project: LOS PRADOS Units: English
 SubTitle: Q2 EXISTING CONDITIONS Areal Units: Acres
 State: New Mexico
 County: Santa Fe
 Filename: K:\HOMEWISE SOUTH MEADOWS\Drainage\TR55\Q2 EX.w55

--- Sub-Area Data ---

Name	Description	Reach	Area(ac)	RCN	Tc
Q2		Outlet	24.4	81	.5

Total area: 24.40 (ac)

--- Storm Data --

Rainfall Depth by Rainfall Return Period

2-Yr (in)	-Yr (in)	-Yr (in)	25-Yr (in)	50-Yr (in)	100-Yr (in)	-Yr (in)
1.52	.0	.0	2.6	2.91	3.23	.0

Storm Data Source: User-provided custom storm data
 Rainfall Distribution Type: Type II
 Dimensionless Unit Hydrograph: <standard>

Watershed Peak Table

Sub-Area or Reach Identifier	25-Yr (cfs)	50-Yr (cfs)	100-Yr (cfs)
------------------------------------	----------------	----------------	-----------------

SUBAREAS
 Q2 21.21 26.37 31.83

REACHES

OUTLET 21.21 26.37 31.83

Hydrograph Peak/Peak Time Table

Sub-Area or Reach Identifier	25-Yr (cfs) (hr)	50-Yr (cfs) (hr)	100-Yr (cfs) (hr)
------------------------------------	------------------------	------------------------	-------------------------

SUBAREAS
 Q2 21.21 26.37 31.83
 12.20 12.19 12.20

REACHES

OUTLET 21.21 26.37 31.83

LOS PRADOS
Q2 EXISTING CONDITIONS
Santa Fe County, New Mexico

Sub-Area Identifier	Drainage Area (ac)	Time of Concentration (hr)	Curve Number	Receiving Reach	Sub-Area Description
Q2	24.40	0.500	81	Outlet	
Total Area:		24.40 (ac)			

Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wetted Perimeter (ft)	Velocity (ft/sec)	Travel Time (hr)

Q2							
SHEET	100	0.0100	0.150				0.313
SHALLOW	1000	0.0218	0.050				0.117
SHALLOW	442	0.0117	0.050				0.070
Time of Concentration							.5
							=====

Sub-Area Identifier	Land Use	Hydrologic Soil Group	Sub-Area Area (ac)	Curve Number
Q2	Herbaceous	(fair) C	24.4	81
Total Area / Weighted Curve Number			24.4	81

WinTR-55 Current Data Description

--- Identification Data ---

User: CPC Date: 9/7/2022
 Project: LOS PRADOS Units: English
 SubTitle: Q3 EXISTING CONDITIONS Areal Units: Acres
 State: New Mexico
 County: Santa Fe
 Filename: K:\HOMEWISE SOUTH MEADOWS\Drainage\TR55\Q3 EX.w55

--- Sub-Area Data ---

Name	Description	Reach	Area(ac)	RCN	Tc
Q3		Outlet	1.27	81	.238

Total area: 1.27 (ac)

--- Storm Data --

Rainfall Depth by Rainfall Return Period

2-Yr (in)	-Yr (in)	-Yr (in)	25-Yr (in)	50-Yr (in)	100-Yr (in)	-Yr (in)
1.52	.0	.0	2.6	2.91	3.23	.0

Storm Data Source: User-provided custom storm data
 Rainfall Distribution Type: Type II
 Dimensionless Unit Hydrograph: <standard>

Watershed Peak Table

Sub-Area or Reach Identifier	25-Yr (cfs)	50-Yr (cfs)	100-Yr (cfs)
------------------------------------	----------------	----------------	-----------------

SUBAREAS
 Q3 1.61 1.98 2.39

REACHES

OUTLET 1.61 1.98 2.39

Hydrograph Peak/Peak Time Table

Sub-Area or Reach Identifier	25-Yr (cfs) (hr)	50-Yr (cfs) (hr)	100-Yr (cfs) (hr)
------------------------------------	------------------------	------------------------	-------------------------

SUBAREAS
 Q3 1.61 1.98 2.39
 12.04 12.04 12.04

REACHES

OUTLET 1.61 1.98 2.39

CPC

LOS PRADOS
Q3 EXISTING CONDITIONS
Santa Fe County, New Mexico

Sub-Area Summary Table

Sub-Area Identifier	Drainage Area (ac)	Time of Concentration (hr)	Curve Number	Receiving Reach	Sub-Area Description
Q3	1.27	0.238	81	Outlet	

Total Area: 1.27 (ac)

Sub-Area Time of Concentration Details

Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wetted Perimeter (ft)	Velocity (ft/sec)	Travel Time (hr)
Q3							
SHEET	100	0.0337	0.150				0.192
SHALLOW	248	0.0086	0.050				0.046
Time of Concentration							.238
							=====

Sub-Area Land Use and Curve Number Details

Sub-Area Identifier	Land Use	Hydrologic Soil Group	Sub-Area Area (ac)	Curve Number
Q3	Herbaceous	(fair) C	1.27	81
Total Area / Weighted Curve Number			1.27	81
			=====	==

WinTR-55 Current Data Description

--- Identification Data ---

User: CPC Date: 9/7/2022
 Project: LOS PRADOS Units: English
 SubTitle: Q4 EXISTING CONDITIONS Areal Units: Acres
 State: New Mexico
 County: Santa Fe
 Filename: K:\HOMEWISE SOUTH MEADOWS\Drainage\TR55\Q4 EX.w55

--- Sub-Area Data ---

Name	Description	Reach	Area(ac)	RCN	Tc
Q4		Outlet	0.46	81	.304

Total area: .46 (ac)

--- Storm Data --

Rainfall Depth by Rainfall Return Period

2-Yr (in)	-Yr (in)	-Yr (in)	25-Yr (in)	50-Yr (in)	100-Yr (in)	-Yr (in)
1.52	.0	.0	2.6	2.91	3.23	.0

Storm Data Source: User-provided custom storm data
 Rainfall Distribution Type: Type II
 Dimensionless Unit Hydrograph: <standard>

Watershed Peak Table

Sub-Area or Reach Identifier	25-Yr (cfs)	50-Yr (cfs)	100-Yr (cfs)
------------------------------------	----------------	----------------	-----------------

SUBAREAS

Q4	0.53	0.65	0.79
----	------	------	------

REACHES

OUTLET	0.53	0.65	0.79
--------	------	------	------

Hydrograph Peak/Peak Time Table

Sub-Area or Reach Identifier	25-Yr (cfs) (hr)	50-Yr (cfs) (hr)	100-Yr (cfs) (hr)
------------------------------------	------------------------	------------------------	-------------------------

SUBAREAS

Q4	0.53 12.08	0.65 12.07	0.79 12.07
----	---------------	---------------	---------------

REACHES

OUTLET	0.53	0.65	0.79
--------	------	------	------

CPC

LOS PRADOS
Q4 EXISTING CONDITIONS
Santa Fe County, New Mexico

Sub-Area Summary Table

Sub-Area Identifier	Drainage Area (ac)	Time of Concentration (hr)	Curve Number	Receiving Reach	Sub-Area Description
Q4	.46	0.304	81	Outlet	

Total Area:	.46 (ac)				

Sub-Area Time of Concentration Details

Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wetted Perimeter (ft)	Velocity (ft/sec)	Travel Time (hr)
Q4							
SHEET	100	0.0114	0.150				0.297
SHALLOW	53	0.0160	0.050				0.007
Time of Concentration							.304
							=====

Sub-Area Land Use and Curve Number Details

Sub-Area Identifier	Land Use	Hydrologic Soil Group	Sub-Area Area (ac)	Curve Number
Q4	Herbaceous	(fair) C	.46	81
Total Area / Weighted Curve Number				
				.46 81
				==

APPENDIX C

DEVELOPED CONDITIONS DRAINAGE CALCULATIONS

WinTR-55 Current Data Description

--- Identification Data ---

User: CPC Date: 6/21/2024
 Project: LOS PRADOS Units: English
 SubTitle: QD2A PROP DI ON CALLE CAMPO Areal Units: Acres
 State: New Mexico
 County: Sante Fe
 Filename: K:\Homewise Los Prados Plat Submittal\Drainage\TR55\2024-06-21 RAIN\QD2A
 PRO.w55

--- Sub-Area Data ---

Name	Description	Reach	Area(ac)	RCN	Tc
QD2A		Outlet	1.5	92	.234

Total area: 1.50 (ac)

--- Storm Data --

Rainfall Depth by Rainfall Return Period

2-Yr (in)	-Yr (in)	-Yr (in)	25-Yr (in)	50-Yr (in)	100-Yr (in)	-Yr (in)
1.52	.0	.0	2.6	2.91	3.23	.0

Storm Data Source: User-provided custom storm data
 Rainfall Distribution Type: Type II
 Dimensionless Unit Hydrograph: <standard>

Watershed Peak Table

Sub-Area or Reach Identifier	Peak Flow by Rainfall Return Period 25-Yr (cfs)	50-Yr (cfs)	100-Yr (cfs)
SUBAREAS			
QD2A	3.34	3.86	4.39
REACHES			
OUTLET	3.34	3.86	4.39

Hydrograph Peak/Peak Time Table

Sub-Area or Reach Identifier	Peak Flow and Peak Time (hr) by Rainfall Return Period 25-Yr (cfs) (hr)	50-Yr (cfs) (hr)	100-Yr (cfs) (hr)
SUBAREAS			
QD2A	3.34 12.02	3.86 12.02	4.39 12.03
REACHES			
OUTLET	3.34	3.86	4.39

CPC

LOS PRADOS
QD2A PROP DI ON CALLE CAMPO
Sante Fe County, New Mexico

Sub-Area Summary Table

Sub-Area Identifier	Drainage Area (ac)	Time of Concentration (hr)	Curve Number	Receiving Reach	Sub-Area Description
QD2A	1.50	0.234	92	Outlet	

Total Area: 1.50 (ac)

Sub-Area Time of Concentration Details

Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wetted Perimeter (ft)	Velocity (ft/sec)	Travel Time (hr)
QD2A							
SHEET	100	0.0472	0.150				0.168
SHALLOW	81	0.0519	0.050				0.006
SHALLOW	334	0.0091	0.050				0.060

Time of Concentration .234
=====

Sub-Area Land Use and Curve Number Details

Sub-Area Identifier	Land Use	Hydrologic Soil Group	Sub-Area Area (ac)	Curve Number
QD2A	Paved parking lots, roofs, driveways	C	.03	98
	Paved, curbs and storm sewers	C	.1	98
	Residential districts (1/8 acre)	C	.07	90
	Newly graded area (pervious only)	C	1.3	91
	Total Area / Weighted Curve Number		1.5 ===	92 ==

WinTR-55 Current Data Description

--- Identification Data ---

User: CPC Date: 6/21/2024
 Project: LOS PRADOS Units: English
 SubTitle: QD2B PROP DI ON CALLE CAMPO Areal Units: Acres
 State: New Mexico
 County: Sante Fe
 Filename: K:\Homewise Los Prados Plat Submittal\Drainage\TR55\2024-06-21 RAIN\QD2B
 PRO.w55

--- Sub-Area Data ---

Name	Description	Reach	Area(ac)	RCN	Tc
QD2B		Outlet	0.83	90	.281

Total area: .83 (ac)

--- Storm Data --

Rainfall Depth by Rainfall Return Period

2-Yr (in)	-Yr (in)	-Yr (in)	25-Yr (in)	50-Yr (in)	100-Yr (in)	-Yr (in)
1.52	.0	.0	2.6	2.91	3.23	.0

Storm Data Source: User-provided custom storm data
 Rainfall Distribution Type: Type II
 Dimensionless Unit Hydrograph: <standard>

Watershed Peak Table

Sub-Area or Reach Identifier	Peak Flow by Rainfall Return Period 25-Yr (cfs)	50-Yr (cfs)	100-Yr (cfs)
SUBAREAS			
QD2B	1.59	1.86	2.14
REACHES			
OUTLET	1.59	1.86	2.14

Hydrograph Peak/Peak Time Table

Sub-Area or Reach Identifier	Peak Flow and Peak Time (hr) by Rainfall Return Period 25-Yr (cfs) (hr)	50-Yr (cfs) (hr)	100-Yr (cfs) (hr)
SUBAREAS			
QD2B	1.59 12.07	1.86 12.06	2.14 12.06
REACHES			
OUTLET	1.59	1.86	2.14

CPC

LOS PRADOS
QD2B PROP DI ON CALLE CAMPO
Sante Fe County, New Mexico

Sub-Area Summary Table

Sub-Area Identifier	Drainage Area (ac)	Time of Concentration (hr)	Curve Number	Receiving Reach	Sub-Area Description
QD2B	.83	0.281	90	Outlet	

Total Area: .83 (ac)

Sub-Area Time of Concentration Details

Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wetted Perimeter (ft)	Velocity (ft/sec)	Travel Time (hr)
QD2B							
SHEET	65	0.0100	0.150				0.222
SHALLOW	110	0.0100	0.050				0.019
SHALLOW	229	0.0099	0.050				0.040

Time of Concentration .281
=====

Sub-Area Land Use and Curve Number Details

Sub-Area Identifier	Land Use	Hydrologic Soil Group	Sub-Area Area (ac)	Curve Number
QD2B	Residential districts (1/8 acre)	C	.83	90
	Total Area / Weighted Curve Number		.83 ===	90 ==

WinTR-55 Current Data Description

--- Identification Data ---

User: CPC Date: 6/21/2024
 Project: LOS PRADOS Units: English
 SubTitle: QD2C PROP DI ON CAM DEL LLANO Areal Units: Acres
 State: New Mexico
 County: Sante Fe
 Filename: K:\Homewise Los Prados Plat Submittal\Drainage\TR55\2024-06-21 RAIN\QD2C
 PRO.w55

--- Sub-Area Data ---

Name	Description	Reach	Area(ac)	RCN	Tc
QD2C		Outlet	0.54	95	.159

Total area: .54 (ac)

--- Storm Data --

Rainfall Depth by Rainfall Return Period

2-Yr (in)	-Yr (in)	-Yr (in)	25-Yr (in)	50-Yr (in)	100-Yr (in)	-Yr (in)
1.52	.0	.0	2.6	2.91	3.23	.0

Storm Data Source: User-provided custom storm data
 Rainfall Distribution Type: Type II
 Dimensionless Unit Hydrograph: <standard>

Watershed Peak Table

Sub-Area or Reach Identifier	25-Yr (cfs)	50-Yr (cfs)	100-Yr (cfs)
SUBAREAS			
QD2C	1.48	1.68	1.89
REACHES			
OUTLET	1.48	1.68	1.89

Hydrograph Peak/Peak Time Table

Sub-Area or Reach Identifier	25-Yr (cfs) (hr)	50-Yr (cfs) (hr)	100-Yr (cfs) (hr)
SUBAREAS			
QD2C	1.48 11.97	1.68 11.96	1.89 11.97
REACHES			
OUTLET	1.48	1.68	1.89

CPC

LOS PRADOS
QD2C PROP DI ON CAM DEL LLANO
Sante Fe County, New Mexico

Sub-Area Summary Table

Sub-Area Identifier	Drainage Area (ac)	Time of Concentration (hr)	Curve Number	Receiving Reach	Sub-Area Description
QD2C	.54	0.159	95	Outlet	

Total Area: .54 (ac)

Sub-Area Time of Concentration Details

Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wetted Perimeter (ft)	Velocity (ft/sec)	Travel Time (hr)
QD2C							
SHEET	32	0.0238	0.150				0.089
SHALLOW	614	0.0229	0.050				0.070
Time of Concentration							.159
							=====

Sub-Area Land Use and Curve Number Details

Sub-Area Identifier	Land Use	Hydrologic Soil Group	Sub-Area Area (ac)	Curve Number
QD2C	Paved parking lots, roofs, driveways	C	.08	98
	Paved; curbs and storm sewers	C	.25	98
	Newly graded area (pervious only)	C	.21	91
Total Area / Weighted Curve Number			.54	95
			===	==

WinTR-55 Current Data Description

--- Identification Data ---

User: CPC Date: 6/21/2024
 Project: LOS PRADOS Units: English
 SubTitle: QD2D PROP DI ON CAM DEL LLANO Areal Units: Acres
 State: New Mexico
 County: Sante Fe
 Filename: K:\Homewise Los Prados Plat Submittal\Drainage\TR55\2024-06-21 RAIN\QD2D
 PRO.w55

--- Sub-Area Data ---

Name	Description	Reach	Area(ac)	RCN	Tc
QD2D		Outlet	1.45	92	.31

Total area: 1.45 (ac)

--- Storm Data --

Rainfall Depth by Rainfall Return Period

2-Yr (in)	-Yr (in)	-Yr (in)	25-Yr (in)	50-Yr (in)	100-Yr (in)	-Yr (in)
1.52	.0	.0	2.6	2.91	3.23	.0

Storm Data Source: User-provided custom storm data
 Rainfall Distribution Type: Type II
 Dimensionless Unit Hydrograph: <standard>

Watershed Peak Table

Sub-Area or Reach Identifier	Peak Flow by Rainfall Return Period 25-Yr (cfs)	50-Yr (cfs)	100-Yr (cfs)
SUBAREAS			
QD2D	2.92	3.37	3.83
REACHES			
OUTLET	2.92	3.37	3.83

Hydrograph Peak/Peak Time Table

Sub-Area or Reach Identifier	Peak Flow and Peak Time (hr) by Rainfall Return Period 25-Yr (cfs) (hr)	50-Yr (cfs) (hr)	100-Yr (cfs) (hr)
SUBAREAS			
QD2D	2.92 12.07	3.37 12.08	3.83 12.06
REACHES			
OUTLET	2.92	3.37	3.83

CPC

LOS PRADOS
QD2D PROP DI ON CAM DEL LLANO
Sante Fe County, New Mexico

Sub-Area Summary Table

Sub-Area Identifier	Drainage Area (ac)	Time of Concentration (hr)	Curve Number	Receiving Reach	Sub-Area Description
QD2D	1.45	0.310	92	Outlet	

Total Area:	1.45 (ac)				

Sub-Area Time of Concentration Details

Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wetted Perimeter (ft)	Velocity (ft/sec)	Travel Time (hr)
QD2D							
SHEET	100	0.0210	0.150				0.232
SHALLOW	25	0.0484	0.050				0.002
SHALLOW	623	0.0197	0.050				0.076

Time of Concentration							.31
							=====

Sub-Area Land Use and Curve Number Details

Sub-Area Identifier	Land Use	Hydrologic Soil Group	Sub-Area Area (ac)	Curve Number
QD2D	Paved parking lots, roofs, driveways	C	.07	98
	Paved, curbs and storm sewers	C	.18	98
	Residential districts (1/8 acre)	C	.58	90
	Newly graded area (pervious only)	C	.62	91
Total Area / Weighted Curve Number				
				1.45 92
				==== ==

WinTR-55 Current Data Description

--- Identification Data ---

User: CPC Date: 6/21/2024
 Project: LOS PRADOS Units: English
 SubTitle: QD2E PROP DI ON CALLE DEL GIRASOL Areal Units: Acres
 State: New Mexico
 County: Sante Fe
 Filename: K:\Homewise Los Prados Plat Submittal\Drainage\TR55\2024-06-21 RAIN\QD2E
 PRO.w55

--- Sub-Area Data ---

Name	Description	Reach	Area(ac)	RCN	Tc
QD2E		Outlet	1.05	90	.198

Total area: 1.05 (ac)

--- Storm Data --

Rainfall Depth by Rainfall Return Period

2-Yr (in)	-Yr (in)	-Yr (in)	25-Yr (in)	50-Yr (in)	100-Yr (in)	-Yr (in)
1.52	.0	.0	2.6	2.91	3.23	.0

Storm Data Source: User-provided custom storm data
 Rainfall Distribution Type: Type II
 Dimensionless Unit Hydrograph: <standard>

Watershed Peak Table

Sub-Area or Reach Identifier	25-Yr (cfs)	50-Yr (cfs)	100-Yr (cfs)
SUBAREAS			
QD2E	2.25	2.63	3.02
REACHES			
OUTLET	2.25	2.63	3.02

Hydrograph Peak/Peak Time Table

Sub-Area or Reach Identifier	25-Yr (cfs) (hr)	50-Yr (cfs) (hr)	100-Yr (cfs) (hr)
SUBAREAS			
QD2E	2.25 12.00	2.63 12.01	3.02 12.01
REACHES			
OUTLET	2.25	2.63	3.02

CPC

LOS PRADOS
QD2E PROP DI ON CALLE DEL GIRASOL
Sante Fe County, New Mexico

Sub-Area Summary Table

Sub-Area Identifier	Drainage Area (ac)	Time of Concentration (hr)	Curve Number	Receiving Reach	Sub-Area Description
QD2E	1.05	0.198	90	Outlet	

Total Area: 1.05 (ac)

Sub-Area Time of Concentration Details

Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wetted Perimeter (ft)	Velocity (ft/sec)	Travel Time (hr)
QD2E							
SHEET	63	0.0306	0.150				0.138
SHALLOW	385	0.0122	0.050				0.060
Time of Concentration							.198
							=====

Sub-Area Land Use and Curve Number Details

Sub-Area Identifier	Land Use	Hydrologic Soil Group	Sub-Area Area (ac)	Curve Number
QD2E	Residential districts (1/8 acre)	C	1.05	90
Total Area / Weighted Curve Number			1.05	90
			====	==

WinTR-55 Current Data Description

--- Identification Data ---

User: CPC Date: 6/21/2024
 Project: LOS PRADOS Units: English
 SubTitle: QD2F PROP DI ON CAM DEL LLANO Areal Units: Acres
 State: New Mexico
 County: Sante Fe
 Filename: K:\Homewise Los Prados Plat Submittal\Drainage\TR55\2024-06-21 RAIN\QD2F
 PRO.w55

--- Sub-Area Data ---

Name	Description	Reach	Area(ac)	RCN	Tc
QD2F		Outlet	0.56	90	0.1

Total area: .56 (ac)

--- Storm Data --

Rainfall Depth by Rainfall Return Period

2-Yr (in)	-Yr (in)	-Yr (in)	25-Yr (in)	50-Yr (in)	100-Yr (in)	-Yr (in)
1.52	.0	.0	2.6	2.91	3.23	.0

Storm Data Source: User-provided custom storm data
 Rainfall Distribution Type: Type II
 Dimensionless Unit Hydrograph: <standard>

Watershed Peak Table

Sub-Area or Reach Identifier	25-Yr (cfs)	50-Yr (cfs)	100-Yr (cfs)
SUBAREAS			
QD2F	1.40	1.63	1.88
REACHES			
OUTLET	1.40	1.63	1.88

Hydrograph Peak/Peak Time Table

Sub-Area or Reach Identifier	25-Yr (cfs) (hr)	50-Yr (cfs) (hr)	100-Yr (cfs) (hr)
SUBAREAS			
QD2F	1.40 11.93	1.63 11.93	1.88 11.93
REACHES			
OUTLET	1.40	1.63	1.88

CPC

LOS PRADOS
QD2F PROP DI ON CAM DEL LLANO
Sante Fe County, New Mexico

Sub-Area Summary Table

Sub-Area Identifier	Drainage Area (ac)	Time of Concentration (hr)	Curve Number	Receiving Reach	Sub-Area Description
QD2F	.56	0.100	90	Outlet	

Total Area: .56 (ac)

Sub-Area Time of Concentration Details

Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wetted Perimeter (ft)	Velocity (ft/sec)	Travel Time (hr)
QD2F							
SHEET	48	0.1000	0.150				0.069
SHALLOW	65	0.0512	0.050				0.005
SHALLOW	119	0.0104	0.050				0.020

Time of Concentration 0.1
=====

Sub-Area Land Use and Curve Number Details

Sub-Area Identifier	Land Use	Hydrologic Soil Group	Sub-Area Area (ac)	Curve Number
QD2F	Residential districts (1/8 acre)	C	.56	90
	Total Area / Weighted Curve Number		.56 ===	90 ==

WinTR-55 Current Data Description

--- Identification Data ---

User: CPC Date: 6/21/2024
 Project: LOS PRADOS Units: English
 SubTitle: QD2G PROP DI ON CAM DEL LLANO Areal Units: Acres
 State: New Mexico
 County: Sante Fe
 Filename: K:\Homewise Los Prados Plat Submittal\Drainage\TR55\2024-06-21 RAIN\QD2G
 PRO.w55

--- Sub-Area Data ---

Name	Description	Reach	Area(ac)	RCN	Tc
QD2G		Outlet	0.76	90	.213

Total area: .76 (ac)

--- Storm Data --

Rainfall Depth by Rainfall Return Period

2-Yr (in)	-Yr (in)	-Yr (in)	25-Yr (in)	50-Yr (in)	100-Yr (in)	-Yr (in)
1.52	.0	.0	2.6	2.91	3.23	.0

Storm Data Source: User-provided custom storm data
 Rainfall Distribution Type: Type II
 Dimensionless Unit Hydrograph: <standard>

Watershed Peak Table

Sub-Area or Reach Identifier	Peak Flow by Rainfall Return Period 25-Yr (cfs)	50-Yr (cfs)	100-Yr (cfs)
SUBAREAS			
QD2G	1.60	1.87	2.15
REACHES			
OUTLET	1.60	1.87	2.15

Hydrograph Peak/Peak Time Table

Sub-Area or Reach Identifier	Peak Flow and Peak Time (hr) by Rainfall Return Period 25-Yr (cfs) (hr)	50-Yr (cfs) (hr)	100-Yr (cfs) (hr)
SUBAREAS			
QD2G	1.60 12.02	1.87 12.01	2.15 12.01
REACHES			
OUTLET	1.60	1.87	2.15

CPC

LOS PRADOS
QD2G PROP DI ON CAM DEL LLANO
Sante Fe County, New Mexico

Sub-Area Summary Table

Sub-Area Identifier	Drainage Area (ac)	Time of Concentration (hr)	Curve Number	Receiving Reach	Sub-Area Description
QD2G	.76	0.213	90	Outlet	

Total Area:	.76 (ac)				

Sub-Area Time of Concentration Details

Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wetted Perimeter (ft)	Velocity (ft/sec)	Travel Time (hr)
QD2G							
SHEET	44	0.0100	0.150				0.162
SHALLOW	127	0.0100	0.050				0.022
SHALLOW	173	0.0107	0.050				0.029

Time of Concentration							.213
							=====

Sub-Area Land Use and Curve Number Details

Sub-Area Identifier	Land Use	Hydrologic Soil Group	Sub-Area Area (ac)	Curve Number
QD2G	Residential districts (1/8 acre)	C	.76	90
Total Area / Weighted Curve Number				
			.76	90
			===	==

WinTR-55 Current Data Description

--- Identification Data ---

User: CPC Date: 6/21/2024
 Project: LOS PRADOS Units: English
 SubTitle: QD2H PROP DI ON CAM DEL LLANO Areal Units: Acres
 State: New Mexico
 County: Sante Fe
 Filename: K:\Homewise Los Prados Plat Submittal\Drainage\TR55\2024-06-21 RAIN\QD2H
 PRO.w55

--- Sub-Area Data ---

Name	Description	Reach	Area(ac)	RCN	Tc
QD2H		Outlet	0.03	98	0.1

Total area: .03 (ac)

--- Storm Data --

Rainfall Depth by Rainfall Return Period

2-Yr (in)	-Yr (in)	-Yr (in)	25-Yr (in)	50-Yr (in)	100-Yr (in)	-Yr (in)
1.52	.0	.0	2.6	2.91	3.23	.0

Storm Data Source: User-provided custom storm data
 Rainfall Distribution Type: Type II
 Dimensionless Unit Hydrograph: <standard>

Watershed Peak Table

Sub-Area or Reach Identifier	Peak Flow by Rainfall Return Period 25-Yr (cfs)	50-Yr (cfs)	100-Yr (cfs)
SUBAREAS			
QD2H	0.10	0.12	0.13
REACHES			
OUTLET	0.10	0.12	0.13

Hydrograph Peak/Peak Time Table

Sub-Area or Reach Identifier	Peak Flow and Peak Time (hr) by Rainfall Return Period 25-Yr (cfs) (hr)	50-Yr (cfs) (hr)	100-Yr (cfs) (hr)
SUBAREAS			
QD2H	0.10 11.93	0.12 11.93	0.13 11.92
REACHES			
OUTLET	0.10	0.12	0.13

CPC

LOS PRADOS
QD2H PROP DI ON CAM DEL LLANO
Sante Fe County, New Mexico

Sub-Area Summary Table

Sub-Area Identifier	Drainage Area (ac)	Time of Concentration (hr)	Curve Number	Receiving Reach	Sub-Area Description
QD2H	.03	0.100	98	Outlet	

Total Area: .03 (ac)

Sub-Area Time of Concentration Details

Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wetted Perimeter (ft)	Velocity (ft/sec)	Travel Time (hr)
QD2H SHEET	43	0.0179	0.011				0.016

Time of Concentration 0.1
=====

Sub-Area Land Use and Curve Number Details

Sub-Area Identifier	Land Use	Hydrologic Soil Group	Sub-Area Area (ac)	Curve Number
QD2H	Paved; curbs and storm sewers	C	.03	98
Total Area / Weighted Curve Number			.03 ===	98 ==

WinTR-55 Current Data Description

--- Identification Data ---

User: CPC Date: 6/21/2024
 Project: LOS PRADOS Units: English
 SubTitle: QD2I PROP DI ON CAM DEL LLANO Areal Units: Acres
 State: New Mexico
 County: Sante Fe
 Filename: K:\Homewise Los Prados Plat Submittal\Drainage\TR55\2024-06-21 RAIN\QD2I
 PRO.w55

--- Sub-Area Data ---

Name	Description	Reach	Area(ac)	RCN	Tc
QD2I		Outlet	0.28	90	.205

Total area: .28 (ac)

--- Storm Data --

Rainfall Depth by Rainfall Return Period

2-Yr (in)	-Yr (in)	-Yr (in)	25-Yr (in)	50-Yr (in)	100-Yr (in)	-Yr (in)
1.52	.0	.0	2.6	2.91	3.23	.0

Storm Data Source: User-provided custom storm data
 Rainfall Distribution Type: Type II
 Dimensionless Unit Hydrograph: <standard>

Watershed Peak Table

Sub-Area or Reach Identifier	Peak Flow by Rainfall Return Period 25-Yr (cfs)	50-Yr (cfs)	100-Yr (cfs)
SUBAREAS			
QD2I	0.60	0.70	0.80
REACHES			
OUTLET	0.60	0.70	0.80

Hydrograph Peak/Peak Time Table

Sub-Area or Reach Identifier	Peak Flow and Peak Time (hr) by Rainfall Return Period 25-Yr (cfs) (hr)	50-Yr (cfs) (hr)	100-Yr (cfs) (hr)
SUBAREAS			
QD2I	0.60 12.01	0.70 12.01	0.80 12.01
REACHES			
OUTLET	0.60	0.70	0.80

CPC

LOS PRADOS
QD2I PROP DI ON CAM DEL LLANO
Sante Fe County, New Mexico

Sub-Area Summary Table

Sub-Area Identifier	Drainage Area (ac)	Time of Concentration (hr)	Curve Number	Receiving Reach	Sub-Area Description
QD2I	.28	0.205	90	Outlet	

Total Area: .28 (ac)

Sub-Area Time of Concentration Details

Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wetted Perimeter (ft)	Velocity (ft/sec)	Travel Time (hr)
QD2I							
SHEET	71	0.0196	0.150				0.182
SHALLOW	126	0.0087	0.050				0.023
Time of Concentration							.205
							=====

Sub-Area Land Use and Curve Number Details

Sub-Area Identifier	Land Use	Hydrologic Soil Group	Sub-Area Area (ac)	Curve Number
QD2I	Residential districts (1/8 acre)	C	.28	90
Total Area / Weighted Curve Number			.28	90
			===	==

WinTR-55 Current Data Description

--- Identification Data ---

User: CPC Date: 6/21/2024
 Project: LOS PRADOS Units: English
 SubTitle: QD2J PROP DI ON CAM DEL LLANO Areal Units: Acres
 State: New Mexico
 County: Sante Fe
 Filename: K:\Homewise Los Prados Plat Submittal\Drainage\TR55\2024-06-21 RAIN\QD2J
 PRO.w55

--- Sub-Area Data ---

Name	Description	Reach	Area(ac)	RCN	Tc
QD2J		Outlet	0.6	90	.203

Total area: .60 (ac)

--- Storm Data --

Rainfall Depth by Rainfall Return Period

2-Yr (in)	-Yr (in)	-Yr (in)	25-Yr (in)	50-Yr (in)	100-Yr (in)	-Yr (in)
1.52	.0	.0	2.6	2.91	3.23	.0

Storm Data Source: User-provided custom storm data
 Rainfall Distribution Type: Type II
 Dimensionless Unit Hydrograph: <standard>

Watershed Peak Table

Sub-Area or Reach Identifier	Peak Flow by Rainfall Return Period	25-Yr (cfs)	50-Yr (cfs)	100-Yr (cfs)
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SUBAREAS				
QD2J	1.28	1.50	1.72	

REACHES

OUTLET	1.28	1.50	1.72
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Hydrograph Peak/Peak Time Table

Sub-Area or Reach Identifier	Peak Flow and Peak Time (hr) by Rainfall Return Period	25-Yr (cfs) (hr)	50-Yr (cfs) (hr)	100-Yr (cfs) (hr)
------------------------------------	--	------------------------	------------------------	-------------------------

SUBAREAS				
QD2J	1.28	1.50	1.72	
	12.00	12.01	12.01	

REACHES

OUTLET	1.28	1.50	1.72
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CPC

LOS PRADOS
QD2J PROP DI ON CAM DEL LLANO
Sante Fe County, New Mexico

Sub-Area Summary Table

Sub-Area Identifier	Drainage Area (ac)	Time of Concentration (hr)	Curve Number	Receiving Reach	Sub-Area Description
QD2J	.60	0.203	90	Outlet	

Total Area:	.60 (ac)				

Sub-Area Time of Concentration Details

Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wetted Perimeter (ft)	Velocity (ft/sec)	Travel Time (hr)
QD2J							
SHEET	44	0.0100	0.150				0.162
SHALLOW	119	0.0100	0.050				0.020
SHALLOW	123	0.0103	0.050				0.021

Time of Concentration							.203
							=====

Sub-Area Land Use and Curve Number Details

Sub-Area Identifier	Land Use	Hydrologic Soil Group	Sub-Area Area (ac)	Curve Number
QD2J	Residential districts (1/8 acre)	C	.6	90
Total Area / Weighted Curve Number				
				.6 90
				== ==

WinTR-55 Current Data Description

--- Identification Data ---

User: CPC Date: 6/21/2024
 Project: LOS PRADOS Units: English
 SubTitle: QD2K PROP DI ON CAM DEL LLANO Areal Units: Acres
 State: New Mexico
 County: Sante Fe
 Filename: K:\Homewise Los Prados Plat Submittal\Drainage\TR55\2024-06-21 RAIN\QD2K
 PRO.w55

--- Sub-Area Data ---

Name	Description	Reach	Area(ac)	RCN	Tc
QD2K		Outlet	0.17	90	.23

Total area: .17 (ac)

--- Storm Data --

Rainfall Depth by Rainfall Return Period

2-Yr (in)	-Yr (in)	-Yr (in)	25-Yr (in)	50-Yr (in)	100-Yr (in)	-Yr (in)
1.52	.0	.0	2.6	2.91	3.23	.0

Storm Data Source: User-provided custom storm data
 Rainfall Distribution Type: Type II
 Dimensionless Unit Hydrograph: <standard>

Watershed Peak Table

Sub-Area or Reach Identifier	25-Yr (cfs)	50-Yr (cfs)	100-Yr (cfs)
SUBAREAS			
QD2K	0.35	0.42	0.48
REACHES			
OUTLET	0.35	0.42	0.48

Hydrograph Peak/Peak Time Table

Sub-Area or Reach Identifier	25-Yr (cfs) (hr)	50-Yr (cfs) (hr)	100-Yr (cfs) (hr)
SUBAREAS			
QD2K	0.35 12.02	0.42 12.03	0.48 12.02
REACHES			
OUTLET	0.35	0.42	0.48

CPC

LOS PRADOS
QD2K PROP DI ON CAM DEL LLANO
Sante Fe County, New Mexico

Sub-Area Summary Table

Sub-Area Identifier	Drainage Area (ac)	Time of Concentration (hr)	Curve Number	Receiving Reach	Sub-Area Description
QD2K	.17	0.230	90	Outlet	

Total Area: .17 (ac)

Sub-Area Time of Concentration Details

Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wetted Perimeter (ft)	Velocity (ft/sec)	Travel Time (hr)
QD2K							
SHEET	64	0.0100	0.150				0.219
SHALLOW	83	0.0178	0.050				0.011
Time of Concentration							.23
							=====

Sub-Area Land Use and Curve Number Details

Sub-Area Identifier	Land Use	Hydrologic Soil Group	Sub-Area Area (ac)	Curve Number
QD2K	Residential districts (1/8 acre)	C	.17	90
Total Area / Weighted Curve Number			.17	90
			===	==

WinTR-55 Current Data Description

--- Identification Data ---

User: CPC Date: 6/21/2024
 Project: LOS PRADOS Units: English
 SubTitle: QD2L PROP DI ON CAM DEL LLANO Areal Units: Acres
 State: New Mexico
 County: Sante Fe
 Filename: K:\Homewise Los Prados Plat Submittal\Drainage\TR55\2024-06-21 RAIN\QD2L
 PRO.w55

--- Sub-Area Data ---

Name	Description	Reach	Area(ac)	RCN	Tc
QD2L		Outlet	0.3	90	.239

Total area: .30 (ac)

--- Storm Data --

Rainfall Depth by Rainfall Return Period

2-Yr (in)	-Yr (in)	-Yr (in)	25-Yr (in)	50-Yr (in)	100-Yr (in)	-Yr (in)
1.52	.0	.0	2.6	2.91	3.23	.0

Storm Data Source: User-provided custom storm data
 Rainfall Distribution Type: Type II
 Dimensionless Unit Hydrograph: <standard>

Watershed Peak Table

Sub-Area or Reach Identifier	Peak Flow by Rainfall Return Period 25-Yr (cfs)	50-Yr (cfs)	100-Yr (cfs)
SUBAREAS			
QD2L	0.61	0.71	0.82
REACHES			
OUTLET	0.61	0.71	0.82

Hydrograph Peak/Peak Time Table

Sub-Area or Reach Identifier	Peak Flow and Peak Time (hr) by Rainfall Return Period 25-Yr (cfs) (hr)	50-Yr (cfs) (hr)	100-Yr (cfs) (hr)
SUBAREAS			
QD2L	0.61 12.03	0.71 12.03	0.82 12.03
REACHES			
OUTLET	0.61	0.71	0.82

CPC

LOS PRADOS
QD2L PROP DI ON CAM DEL LLANO
Sante Fe County, New Mexico

Sub-Area Summary Table

Sub-Area Identifier	Drainage Area (ac)	Time of Concentration (hr)	Curve Number	Receiving Reach	Sub-Area Description
QD2L	.30	0.239	90	Outlet	

Total Area: .30 (ac)

Sub-Area Time of Concentration Details

Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wetted Perimeter (ft)	Velocity (ft/sec)	Travel Time (hr)
QD2L							
SHEET	61	0.0100	0.150				0.211
SHALLOW	102	0.0039	0.050				0.028
Time of Concentration							.239
							=====

Sub-Area Land Use and Curve Number Details

Sub-Area Identifier	Land Use	Hydrologic Soil Group	Sub-Area Area (ac)	Curve Number
QD2L	Residential districts (1/8 acre)	C	.3	90
Total Area / Weighted Curve Number			.3	90
			==	==

WinTR-55 Current Data Description

--- Identification Data ---

User: CPC Date: 6/21/2024
 Project: LOS PRADOS Units: English
 SubTitle: QD2M PROP DI ON CAM DEL LLANO Areal Units: Acres
 State: New Mexico
 County: Sante Fe
 Filename: K:\Homewise Los Prados Plat Submittal\Drainage\TR55\2024-06-21 RAIN\QD2M
 PRO.w55

--- Sub-Area Data ---

Name	Description	Reach	Area(ac)	RCN	Tc
QD2M		Outlet	0.9	90	.199

Total area: .90 (ac)

--- Storm Data --

Rainfall Depth by Rainfall Return Period

2-Yr (in)	-Yr (in)	-Yr (in)	25-Yr (in)	50-Yr (in)	100-Yr (in)	-Yr (in)
1.52	.0	.0	2.6	2.91	3.23	.0

Storm Data Source: User-provided custom storm data
 Rainfall Distribution Type: Type II
 Dimensionless Unit Hydrograph: <standard>

Watershed Peak Table

Sub-Area or Reach Identifier	Peak Flow by Rainfall Return Period 25-Yr (cfs)	50-Yr (cfs)	100-Yr (cfs)
SUBAREAS			
QD2M	1.93	2.26	2.59
REACHES			
OUTLET	1.93	2.26	2.59

Hydrograph Peak/Peak Time Table

Sub-Area or Reach Identifier	Peak Flow and Peak Time (hr) by Rainfall Return Period 25-Yr (cfs) (hr)	50-Yr (cfs) (hr)	100-Yr (cfs) (hr)
SUBAREAS			
QD2M	1.93 12.00	2.26 12.00	2.59 12.00
REACHES			
OUTLET	1.93	2.26	2.59

CPC

LOS PRADOS
QD2M PROP DI ON CAM DEL LLANO
Sante Fe County, New Mexico

Sub-Area Summary Table

Sub-Area Identifier	Drainage Area (ac)	Time of Concentration (hr)	Curve Number	Receiving Reach	Sub-Area Description
QD2M	.90	0.199	90	Outlet	

Total Area:	.90 (ac)				

Sub-Area Time of Concentration Details

Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wetted Perimeter (ft)	Velocity (ft/sec)	Travel Time (hr)
QD2M							
SHEET	43	0.0100	0.150				0.159
SHALLOW	116	0.0100	0.050				0.020
SHALLOW	81	0.0048	0.050				0.020
Time of Concentration							.199
							=====

Sub-Area Land Use and Curve Number Details

Sub-Area Identifier	Land Use	Hydrologic Soil Group	Sub-Area Area (ac)	Curve Number
QD2M	Residential districts (1/8 acre)	C	.9	90
Total Area / Weighted Curve Number				
			.9	90
			==	==

WinTR-55 Current Data Description

--- Identification Data ---

User: CPC Date: 6/21/2024
 Project: LOS PRADOS Units: English
 SubTitle: QD2N PROP DI ON CAM DEL LLANO Areal Units: Acres
 State: New Mexico
 County: Sante Fe
 Filename: K:\Homewise Los Prados Plat Submittal\Drainage\TR55\2024-06-21 RAIN\QD2N
 PRO.w55

--- Sub-Area Data ---

Name	Description	Reach	Area(ac)	RCN	Tc
QD2N		Outlet	0.09	96	0.1

Total area: .09 (ac)

--- Storm Data --

Rainfall Depth by Rainfall Return Period

2-Yr (in)	-Yr (in)	-Yr (in)	25-Yr (in)	50-Yr (in)	100-Yr (in)	-Yr (in)
1.52	.0	.0	2.6	2.91	3.23	.0

Storm Data Source: User-provided custom storm data
 Rainfall Distribution Type: Type II
 Dimensionless Unit Hydrograph: <standard>

Watershed Peak Table

Sub-Area or Reach Identifier	25-Yr (cfs)	50-Yr (cfs)	100-Yr (cfs)
SUBAREAS			
QD2N	0.28	0.32	0.35
REACHES			
OUTLET	0.28	0.32	0.35

Hydrograph Peak/Peak Time Table

Sub-Area or Reach Identifier	25-Yr (cfs) (hr)	50-Yr (cfs) (hr)	100-Yr (cfs) (hr)
SUBAREAS			
QD2N	0.28 11.93	0.32 11.93	0.35 11.92
REACHES			
OUTLET	0.28	0.32	0.35

CPC

LOS PRADOS
QD2N PROP DI ON CAM DEL LLANO
Sante Fe County, New Mexico

Sub-Area Summary Table

Sub-Area Identifier	Drainage Area (ac)	Time of Concentration (hr)	Curve Number	Receiving Reach	Sub-Area Description
QD2N	.09	0.100	96	Outlet	

Total Area: .09 (ac)

Sub-Area Time of Concentration Details

Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wetted Perimeter (ft)	Velocity (ft/sec)	Travel Time (hr)
QD2N							
SHEET	17	0.0106	0.011				0.009
SHALLOW	112	0.0091	0.050				0.020

Time of Concentration 0.1
=====

Sub-Area Land Use and Curve Number Details

Sub-Area Identifier	Land Use	Hydrologic Soil Group	Sub-Area Area (ac)	Curve Number
QD2N	Paved parking lots, roofs, driveways	C	.02	98
	Paved; curbs and storm sewers	C	.05	98
	Newly graded area (pervious only)	C	.02	91
	Total Area / Weighted Curve Number		.09	96
			===	==

WinTR-55 Current Data Description

--- Identification Data ---

User: CPC Date: 6/21/2024
 Project: LOS PRADOS Units: English
 SubTitle: QD20 PROP DI ON CAM DEL LLANO Areal Units: Acres
 State: New Mexico
 County: Sante Fe
 Filename: K:\Homewise Los Prados Plat Submittal\Drainage\TR55\2024-06-21 RAIN\QD20
 PRO.w55

--- Sub-Area Data ---

Name	Description	Reach	Area(ac)	RCN	Tc
QD20		Outlet	0.67	90	.124

Total area: .67 (ac)

--- Storm Data --

Rainfall Depth by Rainfall Return Period

2-Yr (in)	-Yr (in)	-Yr (in)	25-Yr (in)	50-Yr (in)	100-Yr (in)	-Yr (in)
1.52	.0	.0	2.6	2.91	3.23	.0

Storm Data Source: User-provided custom storm data
 Rainfall Distribution Type: Type II
 Dimensionless Unit Hydrograph: <standard>

Watershed Peak Table

Sub-Area or Reach Identifier	25-Yr (cfs)	50-Yr (cfs)	100-Yr (cfs)
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SUBAREAS			
QD20	1.60	1.87	2.15

REACHES

OUTLET	1.60	1.87	2.15
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Hydrograph Peak/Peak Time Table

Sub-Area or Reach Identifier	25-Yr (cfs) (hr)	50-Yr (cfs) (hr)	100-Yr (cfs) (hr)
------------------------------------	------------------------	------------------------	-------------------------

SUBAREAS			
QD20	1.60 11.95	1.87 11.94	2.15 11.95

REACHES

OUTLET	1.60	1.87	2.15
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LOS PRADOS
QD20 PROP DI ON CAM DEL LLANO
Sante Fe County, New Mexico

Sub-Area Identifier	Drainage Area (ac)	Time of Concentration (hr)	Curve Number	Receiving Reach	Sub-Area Description
QD20	.67	0.124	90	Outlet	
Total Area: .67 (ac)					

Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wetted Perimeter (ft)	Velocity (ft/sec)	Travel Time (hr)

QD20							
SHEET	12	0.0100	0.150				0.057
SHALLOW	120	0.0100	0.050				0.021
SHALLOW	189	0.0049	0.050				0.046
Time of Concentration							.124
							=====

Sub-Area Identifier	Land Use	Hydrologic Soil Group	Sub-Area Area (ac)	Curve Number
QD20	Residential districts (1/8 acre)	C	.67	90
	Total Area / Weighted Curve Number		.67	90

WinTR-55 Current Data Description

--- Identification Data ---

User: CPC Date: 6/21/2024
 Project: LOS PRADOS Units: English
 SubTitle: QD2P PROP DI ON CALLE DEL GIRASOL Areal Units: Acres
 State: New Mexico
 County: Santa Fe
 Filename: K:\Homewise Los Prados Plat Submittal\Drainage\TR55\2024-06-21 RAIN\QD2P
 PRO.w55

--- Sub-Area Data ---

Name	Description	Reach	Area(ac)	RCN	Tc
QD2P		Outlet	0.94	90	.373

Total area: .94 (ac)

--- Storm Data --

Rainfall Depth by Rainfall Return Period

2-Yr (in)	-Yr (in)	-Yr (in)	25-Yr (in)	50-Yr (in)	100-Yr (in)	-Yr (in)
1.52	.0	.0	2.6	2.91	3.23	.0

Storm Data Source: User-provided custom storm data
 Rainfall Distribution Type: Type II
 Dimensionless Unit Hydrograph: <standard>

Watershed Peak Table

Sub-Area or Reach Identifier	Peak Flow by Rainfall Return Period 25-Yr (cfs)	50-Yr (cfs)	100-Yr (cfs)
SUBAREAS			
QD2P	1.58	1.85	2.13
REACHES			
OUTLET	1.58	1.85	2.13

Hydrograph Peak/Peak Time Table

Sub-Area or Reach Identifier	Peak Flow and Peak Time (hr) by Rainfall Return Period 25-Yr (cfs) (hr)	50-Yr (cfs) (hr)	100-Yr (cfs) (hr)
SUBAREAS			
QD2P	1.58 12.12	1.85 12.11	2.13 12.12
REACHES			
OUTLET	1.58	1.85	2.13

CPC

LOS PRADOS
QD2P PROP DI ON CALLE DEL GIRASOL
Santa Fe County, New Mexico

Sub-Area Summary Table

Sub-Area Identifier	Drainage Area (ac)	Time of Concentration (hr)	Curve Number	Receiving Reach	Sub-Area Description
QD2P	.94	0.373	90	Outlet	

Total Area: .94 (ac)

Sub-Area Time of Concentration Details

Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wetted Perimeter (ft)	Velocity (ft/sec)	Travel Time (hr)
QD2P							
SHEET	100	0.0100	0.150				0.313
SHALLOW	40	0.0100	0.050				0.007
SHALLOW	314	0.0103	0.050				0.053

Time of Concentration .373
=====

Sub-Area Land Use and Curve Number Details

Sub-Area Identifier	Land Use	Hydrologic Soil Group	Sub-Area Area (ac)	Curve Number
QD2P	Residential districts (1/8 acre)	C	.94	90
Total Area / Weighted Curve Number			.94 ===	90 ==

WinTR-55 Current Data Description

--- Identification Data ---

User: CPC Date: 6/21/2024
 Project: LOS PRADOS Units: English
 SubTitle: QD2Q PROP DI ON AVE DE LOS PRADOS Areal Units: Acres
 State: New Mexico
 County: Santa Fe
 Filename: K:\Homewise Los Prados Plat Submittal\Drainage\TR55\2024-06-21 RAIN\QD2Q
 PRO.w55

--- Sub-Area Data ---

Name	Description	Reach	Area(ac)	RCN	Tc
QD2Q		Outlet	1.13	90	.282

Total area: 1.13 (ac)

--- Storm Data --

Rainfall Depth by Rainfall Return Period

2-Yr (in)	-Yr (in)	-Yr (in)	25-Yr (in)	50-Yr (in)	100-Yr (in)	-Yr (in)
1.52	.0	.0	2.6	2.91	3.23	.0

Storm Data Source: User-provided custom storm data
 Rainfall Distribution Type: Type II
 Dimensionless Unit Hydrograph: <standard>

Watershed Peak Table

Sub-Area or Reach Identifier	Peak Flow by Rainfall Return Period 25-Yr (cfs)	50-Yr (cfs)	100-Yr (cfs)
SUBAREAS			
QD2Q	2.16	2.53	2.91
REACHES			
OUTLET	2.16	2.53	2.91

Hydrograph Peak/Peak Time Table

Sub-Area or Reach Identifier	Peak Flow and Peak Time (hr) by Rainfall Return Period 25-Yr (cfs) (hr)	50-Yr (cfs) (hr)	100-Yr (cfs) (hr)
SUBAREAS			
QD2Q	2.16 12.05	2.53 12.07	2.91 12.05
REACHES			
OUTLET	2.16	2.53	2.91

CPC

LOS PRADOS
QD2Q PROP DI ON AVE DE LOS PRADOS
Santa Fe County, New Mexico

Sub-Area Summary Table

Sub-Area Identifier	Drainage Area (ac)	Time of Concentration (hr)	Curve Number	Receiving Reach	Sub-Area Description
QD2Q	1.13	0.282	90	Outlet	

Total Area: 1.13 (ac)

Sub-Area Time of Concentration Details

Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wetted Perimeter (ft)	Velocity (ft/sec)	Travel Time (hr)
QD2Q							
SHEET	60	0.0100	0.150				0.208
SHALLOW	445	0.0108	0.050				0.074
Time of Concentration							.282
							=====

Sub-Area Land Use and Curve Number Details

Sub-Area Identifier	Land Use	Hydrologic Soil Group	Sub-Area Area (ac)	Curve Number
QD2Q	Residential districts (1/8 acre)	C	1.13	90
Total Area / Weighted Curve Number			1.13	90
			====	==

WinTR-55 Current Data Description

--- Identification Data ---

User: CPC Date: 6/21/2024
 Project: LOS PRADOS Units: English
 SubTitle: QD2R PROP DI ON AVE DE LOS PRADOS Areal Units: Acres
 State: New Mexico
 County: Santa Fe
 Filename: K:\Homewise Los Prados Plat Submittal\Drainage\TR55\2024-06-21 RAIN\QD2R
 PRO.w55

--- Sub-Area Data ---

Name	Description	Reach	Area(ac)	RCN	Tc
QD2R		Outlet	0.86	90	.219

Total area: .86 (ac)

--- Storm Data --

Rainfall Depth by Rainfall Return Period

2-Yr (in)	-Yr (in)	-Yr (in)	25-Yr (in)	50-Yr (in)	100-Yr (in)	-Yr (in)
1.52	.0	.0	2.6	2.91	3.23	.0

Storm Data Source: User-provided custom storm data
 Rainfall Distribution Type: Type II
 Dimensionless Unit Hydrograph: <standard>

Watershed Peak Table

Sub-Area or Reach Identifier	Peak Flow by Rainfall Return Period 25-Yr (cfs)	50-Yr (cfs)	100-Yr (cfs)
SUBAREAS			
QD2R	1.79	2.09	2.40
REACHES			
OUTLET	1.79	2.09	2.40

Hydrograph Peak/Peak Time Table

Sub-Area or Reach Identifier	Peak Flow and Peak Time (hr) by Rainfall Return Period 25-Yr (cfs) (hr)	50-Yr (cfs) (hr)	100-Yr (cfs) (hr)
SUBAREAS			
QD2R	1.79 12.03	2.09 12.02	2.40 12.02
REACHES			
OUTLET	1.79	2.09	2.40

CPC

LOS PRADOS
QD2R PROP DI ON AVE DE LOS PRADOS
Santa Fe County, New Mexico

Sub-Area Summary Table

Sub-Area Identifier	Drainage Area (ac)	Time of Concentration (hr)	Curve Number	Receiving Reach	Sub-Area Description
QD2R	.86	0.219	90	Outlet	

Total Area: .86 (ac)

Sub-Area Time of Concentration Details

Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wetted Perimeter (ft)	Velocity (ft/sec)	Travel Time (hr)
QD2R							
SHEET	43	0.0100	0.150				0.159
SHALLOW	113	0.0100	0.050				0.019
SHALLOW	249	0.0110	0.050				0.041

Time of Concentration .219
=====

Sub-Area Land Use and Curve Number Details

Sub-Area Identifier	Land Use	Hydrologic Soil Group	Sub-Area Area (ac)	Curve Number
QD2R	Residential districts (1/8 acre)	C	.86	90
	Total Area / Weighted Curve Number		.86 ===	90 ==

WinTR-55 Current Data Description

--- Identification Data ---

User: CPC Date: 6/21/2024
 Project: LOS PRADOS Units: English
 SubTitle: QD2S PROP DI ON CALLE CAMPO Areal Units: Acres
 State: New Mexico
 County: Santa Fe
 Filename: K:\Homewise Los Prados Plat Submittal\Drainage\TR55\2024-06-21 RAIN\QD2S
 PRO.w55

--- Sub-Area Data ---

Name	Description	Reach	Area(ac)	RCN	Tc
QD2S		Outlet	1.31	90	.386

Total area: 1.31 (ac)

--- Storm Data --

Rainfall Depth by Rainfall Return Period

2-Yr (in)	-Yr (in)	-Yr (in)	25-Yr (in)	50-Yr (in)	100-Yr (in)	-Yr (in)
1.52	.0	.0	2.6	2.91	3.23	.0

Storm Data Source: User-provided custom storm data
 Rainfall Distribution Type: Type II
 Dimensionless Unit Hydrograph: <standard>

Watershed Peak Table

Sub-Area or Reach Identifier	25-Yr (cfs)	50-Yr (cfs)	100-Yr (cfs)
------------------------------------	----------------	----------------	-----------------

SUBAREAS			
QD2S	2.17	2.54	2.92

REACHES

OUTLET	2.17	2.54	2.92
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Hydrograph Peak/Peak Time Table

Sub-Area or Reach Identifier	25-Yr (cfs) (hr)	50-Yr (cfs) (hr)	100-Yr (cfs) (hr)
------------------------------------	------------------------	------------------------	-------------------------

SUBAREAS			
QD2S	2.17 12.13	2.54 12.11	2.92 12.11

REACHES

OUTLET	2.17	2.54	2.92
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CPC

LOS PRADOS
QD2S PROP DI ON CALLE CAMPO
Santa Fe County, New Mexico

Sub-Area Summary Table

Sub-Area Identifier	Drainage Area (ac)	Time of Concentration (hr)	Curve Number	Receiving Reach	Sub-Area Description
QD2S	1.31	0.386	90	Outlet	

Total Area: 1.31 (ac)

Sub-Area Time of Concentration Details

Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wetted Perimeter (ft)	Velocity (ft/sec)	Travel Time (hr)
QD2S							
SHEET	92	0.0100	0.150				0.292
SHALLOW	483	0.0078	0.050				0.094
Time of Concentration							.386
							=====

Sub-Area Land Use and Curve Number Details

Sub-Area Identifier	Land Use	Hydrologic Soil Group	Sub-Area Area (ac)	Curve Number
QD2S	Residential districts (1/8 acre)	C	1.31	90
Total Area / Weighted Curve Number			1.31	90
			====	==

WinTR-55 Current Data Description

--- Identification Data ---

User: CPC Date: 6/21/2024
 Project: LOS PRADOS Units: English
 SubTitle: QD2T PROP DI ON CALLE CAMPO Areal Units: Acres
 State: New Mexico
 County: Santa Fe
 Filename: K:\Homewise Los Prados Plat Submittal\Drainage\TR55\2024-06-21 RAIN\QD2T
 PRO.w55

--- Sub-Area Data ---

Name	Description	Reach	Area(ac)	RCN	Tc
QD2T		Outlet	3.13	91	.404

Total area: 3.13 (ac)

--- Storm Data --

Rainfall Depth by Rainfall Return Period

2-Yr (in)	-Yr (in)	-Yr (in)	25-Yr (in)	50-Yr (in)	100-Yr (in)	-Yr (in)
1.52	.0	.0	2.6	2.91	3.23	.0

Storm Data Source: User-provided custom storm data
 Rainfall Distribution Type: Type II
 Dimensionless Unit Hydrograph: <standard>

Watershed Peak Table

Sub-Area or Reach Identifier	25-Yr (cfs)	50-Yr (cfs)	100-Yr (cfs)
SUBAREAS			
QD2T	5.29	6.16	7.04
REACHES			
OUTLET	5.29	6.16	7.04

Hydrograph Peak/Peak Time Table

Sub-Area or Reach Identifier	25-Yr (cfs) (hr)	50-Yr (cfs) (hr)	100-Yr (cfs) (hr)
SUBAREAS			
QD2T	5.29 12.13	6.16 12.12	7.04 12.11
REACHES			
OUTLET	5.29	6.16	7.04

CPC

LOS PRADOS
QD2T PROP DI ON CALLE CAMPO
Santa Fe County, New Mexico

Sub-Area Summary Table

Sub-Area Identifier	Drainage Area (ac)	Time of Concentration (hr)	Curve Number	Receiving Reach	Sub-Area Description
QD2T	3.13	0.404	91	Outlet	

Total Area:	3.13 (ac)				

Sub-Area Time of Concentration Details

Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wetted Perimeter (ft)	Velocity (ft/sec)	Travel Time (hr)
QD2T							
SHEET	100	0.0289	0.150				0.204
SHALLOW	480	0.0108	0.050				0.080
SHALLOW	613	0.0077	0.050				0.120

Time of Concentration							.404
							=====

Sub-Area Land Use and Curve Number Details

Sub-Area Identifier	Land Use	Hydrologic Soil Group	Sub-Area Area (ac)	Curve Number
QD2T	Paved parking lots, roofs, driveways	C	.04	98
	Paved, curbs and storm sewers	C	.16	98
	Residential districts (1/8 acre)	C	2.65	90
	Newly graded area (pervious only)	C	.28	91
Total Area / Weighted Curve Number				
				91
				====
				==

WinTR-55 Current Data Description

--- Identification Data ---

User: CPC Date: 6/17/2024
 Project: LOS PRADOS Units: English
 SubTitle: QD2U PROP FLOW ON SOUTH MEADOWS ROAD Areal Units: Acres
 State: New Mexico
 County: Santa Fe
 Filename: K:\Homewise Los Prados Plat Submittal\Drainage\TR55\2024-06-17\QD2U PRO.w55

--- Sub-Area Data ---

Name	Description	Reach	Area(ac)	RCN	Tc
QD2U		Outlet	0.18	97	0.1

Total area: .18 (ac)

--- Storm Data --

Rainfall Depth by Rainfall Return Period

2-Yr (in)	-Yr (in)	-Yr (in)	25-Yr (in)	50-Yr (in)	100-Yr (in)	-Yr (in)
1.52	.0	.0	2.6	2.91	3.23	.0

Storm Data Source: User-provided custom storm data
 Rainfall Distribution Type: Type II
 Dimensionless Unit Hydrograph: <standard>

Watershed Peak Table

Sub-Area or Reach Identifier	25-Yr (cfs)	50-Yr (cfs)	100-Yr (cfs)
SUBAREAS			
QD2U	0.57	0.65	0.72
REACHES			
OUTLET	0.57	0.65	0.72

Hydrograph Peak/Peak Time Table

Sub-Area or Reach Identifier	25-Yr (cfs) (hr)	50-Yr (cfs) (hr)	100-Yr (cfs) (hr)
SUBAREAS			
QD2U	0.57 11.92	0.65 11.93	0.72 11.93
REACHES			
OUTLET	0.57	0.65	0.72

CPC

LOS PRADOS
QD2U PROP FLOW ON SOUTH MEADOWS ROAD
Santa Fe County, New Mexico

Sub-Area Summary Table

Sub-Area Identifier	Drainage Area (ac)	Time of Concentration (hr)	Curve Number	Receiving Reach	Sub-Area Description
QD2U	.18	0.100	97	Outlet	

Total Area: .18 (ac)

Sub-Area Time of Concentration Details

Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wetted Perimeter (ft)	Velocity (ft/sec)	Travel Time (hr)
QD2U							
SHEET	29	0.0245	0.011				0.010
SHALLOW	328	0.0289	0.025				0.026

Time of Concentration 0.1
=====

Sub-Area Land Use and Curve Number Details

Sub-Area Identifier	Land Use	Hydrologic Soil Group	Sub-Area Area (ac)	Curve Number
QD2U	Paved parking lots, roofs, driveways	C	.04	98
	Paved; curbs and storm sewers	C	.11	98
	Newly graded area (pervious only)	C	.03	91
	Total Area / Weighted Curve Number		.18	97
			===	==

WinTR-55 Current Data Description

--- Identification Data ---

User: CPC Date: 6/17/2024
 Project: LOS PRADOS Units: English
 SubTitle: QD2V PROP FLOW ON SOUTH MEADOWS ROAD Areal Units: Acres
 State: New Mexico
 County: Santa Fe
 Filename: K:\Homewise Los Prados Plat Submittal\Drainage\TR55\2024-06-17\QD2V PRO.w55

--- Sub-Area Data ---

Name	Description	Reach	Area(ac)	RCN	Tc
QD2V		Outlet	0.1	97	.103

Total area: .10 (ac)

--- Storm Data --

Rainfall Depth by Rainfall Return Period

2-Yr (in)	-Yr (in)	-Yr (in)	25-Yr (in)	50-Yr (in)	100-Yr (in)	-Yr (in)
1.52	.0	.0	2.6	2.91	3.23	.0

Storm Data Source: User-provided custom storm data
 Rainfall Distribution Type: Type II
 Dimensionless Unit Hydrograph: <standard>

Watershed Peak Table

Sub-Area or Reach Identifier	25-Yr (cfs)	50-Yr (cfs)	100-Yr (cfs)
SUBAREAS			
QD2V	0.33	0.37	0.41
REACHES			
OUTLET	0.33	0.37	0.41

Hydrograph Peak/Peak Time Table

Sub-Area or Reach Identifier	25-Yr (cfs) (hr)	50-Yr (cfs) (hr)	100-Yr (cfs) (hr)
SUBAREAS			
QD2V	0.33 11.93	0.37 11.93	0.41 11.93
REACHES			
OUTLET	0.33	0.37	0.41

CPC

LOS PRADOS
QD2V PROP FLOW ON SOUTH MEADOWS ROAD
Santa Fe County, New Mexico

Sub-Area Summary Table

Sub-Area Identifier	Drainage Area (ac)	Time of Concentration (hr)	Curve Number	Receiving Reach	Sub-Area Description
QD2V	.10	0.103	97	Outlet	

Total Area:	.10 (ac)				

Sub-Area Time of Concentration Details

Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wetted Perimeter (ft)	Velocity (ft/sec)	Travel Time (hr)
QD2V							
SHEET	26	0.0265	0.150				0.072
SHALLOW	177	0.0059	0.025				0.031

Time of Concentration							.103
							=====

Sub-Area Land Use and Curve Number Details

Sub-Area Identifier	Land Use	Hydrologic Soil Group	Sub-Area Area (ac)	Curve Number
QD2V	Paved parking lots, roofs, driveways	C	.02	98
	Paved; curbs and storm sewers	C	.07	98
	Newly graded area (pervious only)	C	.01	91
Total Area / Weighted Curve Number				
				.1
				97
				==
				==

WinTR-55 Current Data Description

--- Identification Data ---

User: CPC Date: 6/17/2024
 Project: LOS PRADOS Units: English
 SubTitle: QD2W PROP FLOW ON SOUTH MEADOWS ROAD Areal Units: Acres
 State: New Mexico
 County: Santa Fe
 Filename: K:\Homewise Los Prados Plat Submittal\Drainage\TR55\2024-06-17\QD2W PRO.w55

--- Sub-Area Data ---

Name	Description	Reach	Area(ac)	RCN	Tc
QD2W		Outlet	0.09	93	.121

Total area: .09 (ac)

--- Storm Data --

Rainfall Depth by Rainfall Return Period

2-Yr (in)	-Yr (in)	-Yr (in)	25-Yr (in)	50-Yr (in)	100-Yr (in)	-Yr (in)
1.52	.0	.0	2.6	2.91	3.23	.0

Storm Data Source: User-provided custom storm data
 Rainfall Distribution Type: Type II
 Dimensionless Unit Hydrograph: <standard>

Watershed Peak Table

Sub-Area or Reach Identifier	25-Yr (cfs)	50-Yr (cfs)	100-Yr (cfs)
SUBAREAS			
QD2W	0.24	0.28	0.32
REACHES			
OUTLET	0.24	0.28	0.32

Hydrograph Peak/Peak Time Table

Sub-Area or Reach Identifier	25-Yr (cfs) (hr)	50-Yr (cfs) (hr)	100-Yr (cfs) (hr)
SUBAREAS			
QD2W	0.24 11.94	0.28 11.94	0.32 11.94
REACHES			
OUTLET	0.24	0.28	0.32

CPC

LOS PRADOS
QD2W PROP FLOW ON SOUTH MEADOWS ROAD
Santa Fe County, New Mexico

Sub-Area Summary Table

Sub-Area Identifier	Drainage Area (ac)	Time of Concentration (hr)	Curve Number	Receiving Reach	Sub-Area Description
QD2W	.09	0.121	93	Outlet	

Total Area:	.09 (ac)				

Sub-Area Time of Concentration Details

Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wetted Perimeter (ft)	Velocity (ft/sec)	Travel Time (hr)
QD2W							
SHEET	25	0.0108	0.150				0.100
SHALLOW	167	0.0113	0.025				0.021

Time of Concentration							.121
							=====

Sub-Area Land Use and Curve Number Details

Sub-Area Identifier	Land Use	Hydrologic Soil Group	Sub-Area Area (ac)	Curve Number
QD2W	Paved parking lots, roofs, driveways	C	.01	98
	Paved; curbs and storm sewers	C	.02	98
	Newly graded area (pervious only)	C	.06	91
Total Area / Weighted Curve Number				
				.09 93
				=== ==

WinTR-55 Current Data Description

--- Identification Data ---

User: CPC Date: 6/21/2024
 Project: LOS PRADOS Units: English
 SubTitle: QD2X PROP OUTFLOW Areal Units: Acres
 State: New Mexico
 County: Sante Fe
 Filename: K:\Homewise Los Prados Plat Submittal\Drainage\TR55\2024-06-17\QD2X PRO.w55

--- Sub-Area Data ---

Name	Description	Reach	Area(ac)	RCN	Tc
QD2X		Outlet	5.05	74	.416

Total area: 5.05 (ac)

--- Storm Data --

Rainfall Depth by Rainfall Return Period

2-Yr (in)	-Yr (in)	-Yr (in)	25-Yr (in)	50-Yr (in)	100-Yr (in)	-Yr (in)
1.52	.0	.0	2.6	2.91	3.23	.0

Storm Data Source: User-provided custom storm data
 Rainfall Distribution Type: Type II
 Dimensionless Unit Hydrograph: <standard>

Watershed Peak Table

Sub-Area or Reach Identifier	25-Yr (cfs)	50-Yr (cfs)	100-Yr (cfs)
SUBAREAS			
QD2X	2.93	3.90	4.97
REACHES			
OUTLET	2.93	3.90	4.97

Hydrograph Peak/Peak Time Table

Sub-Area or Reach Identifier	25-Yr (cfs) (hr)	50-Yr (cfs) (hr)	100-Yr (cfs) (hr)
SUBAREAS			
QD2X	2.93 12.15	3.90 12.16	4.97 12.16
REACHES			
OUTLET	2.93	3.90	4.97

CPC

LOS PRADOS
QD2X PROP OUTFLOW
Sante Fe County, New Mexico

Sub-Area Summary Table

Sub-Area Identifier	Drainage Area (ac)	Time of Concentration (hr)	Curve Number	Receiving Reach	Sub-Area Description
QD2X	5.05	0.416	74	Outlet	

Total Area:	5.05 (ac)				

Sub-Area Time of Concentration Details

Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wetted Perimeter (ft)	Velocity (ft/sec)	Travel Time (hr)
QD2X							
SHEET	100	0.0086	0.150				0.332
SHALLOW	676	0.0193	0.050				0.084

Time of Concentration							.416
							=====

Sub-Area Land Use and Curve Number Details

Sub-Area Identifier	Land Use	Hydrologic Soil Group	Sub-Area Area (ac)	Curve Number
QD2X	Herbaceous	(good) C	5.05	74
Total Area / Weighted Curve Number				
				74
				====
				==

WinTR-55 Current Data Description

--- Identification Data ---

User: CPC Date: 6/21/2024
 Project: LOS PRADOS Units: English
 SubTitle: QD3 PROP OFFSITE FLOW Areal Units: Acres
 State: New Mexico
 County: Sante Fe
 Filename: K:\Homewise Los Prados Plat Submittal\Drainage\TR55\2024-06-17\QD3 PRO.w55

--- Sub-Area Data ---

Name	Description	Reach	Area(ac)	RCN	Tc
QD3		Outlet	0.75	81	.238

Total area: .75 (ac)

--- Storm Data --

Rainfall Depth by Rainfall Return Period

2-Yr (in)	-Yr (in)	-Yr (in)	25-Yr (in)	50-Yr (in)	100-Yr (in)	-Yr (in)
1.52	.0	.0	2.6	2.91	3.23	.0

Storm Data Source: User-provided custom storm data
 Rainfall Distribution Type: Type II
 Dimensionless Unit Hydrograph: <standard>

Watershed Peak Table

Sub-Area or Reach Identifier	25-Yr (cfs)	50-Yr (cfs)	100-Yr (cfs)
SUBAREAS			
QD3	0.95	1.17	1.41
REACHES			
OUTLET	0.95	1.17	1.41

Hydrograph Peak/Peak Time Table

Sub-Area or Reach Identifier	25-Yr (cfs) (hr)	50-Yr (cfs) (hr)	100-Yr (cfs) (hr)
SUBAREAS			
QD3	0.95 12.04	1.17 12.04	1.41 12.04
REACHES			
OUTLET	0.95	1.17	1.41

CPC

LOS PRADOS
QD3 PROP OFFSITE FLOW
Sante Fe County, New Mexico

Sub-Area Summary Table

Sub-Area Identifier	Drainage Area (ac)	Time of Concentration (hr)	Curve Number	Receiving Reach	Sub-Area Description
QD3	.75	0.238	81	Outlet	

Total Area: .75 (ac)

Sub-Area Time of Concentration Details

Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wetted Perimeter (ft)	Velocity (ft/sec)	Travel Time (hr)
QD3							
SHEET	100	0.0337	0.150				0.192
SHALLOW	248	0.0086	0.050				0.046

Time of Concentration .238
=====

Sub-Area Land Use and Curve Number Details

Sub-Area Identifier	Land Use	Hydrologic Soil Group	Sub-Area Area (ac)	Curve Number
QD3	Herbaceous	(fair) C	.75	81
Total Area / Weighted Curve Number			.75 ===	81 ==

APPENDIX D
ENERGY DISSIPATOR CALCULATIONS

Design External Dissipator ▼

Parameter	Value	Units
Select Culvert and Flow		
Crossing	Crossing 1 ▼	
Culvert	Culvert 1 ▼	
Flow	47.56 ▼	cfs
Culvert Data		
Culvert Width (including multiple barrels)	10.4	ft
Culvert Height	2.7	ft
Outlet Depth	0.43	ft
Outlet Velocity	6.07	ft/s
Froude Number	1.61	
Tailwater Depth	0.43	ft
Tailwater Velocity	1.25	ft/s
Tailwater Slope (S0)	0.0111	
External Dissipator Data		
External Dissipator Category	Streambed Level Structures ▼	
External Dissipator Type	Riprap Basin ▼	
Restrictions		
Froude Number	<3	
? Input Data		
Condition to be used to Compute Basin Outlet Velocity	Envelope Curve ▼	
D50 of the Riprap Mixture		
Note: Minimum HS/D50 = 2 is Obtained if D50 = 0.304 ft		
D50 of the Riprap Mixture	0.304	ft
DMax of the Riprap Mixture	2.000	ft
Results		
Brink Depth	0.434	ft
Brink Velocity	6.072	ft/s
Depth (YE)	1.505	ft
Riprap Thickness	3.000	ft
Riprap Foreslope	4.0000	ft
Check HS/D50		
Note: OK if HS/D50 > 2.0		
HS/D50	2.020	
HS/D50 Check	HS/D50 is OK	
Check D50/YE		
Note: OK if $0.1 < D50/YE < 0.7$		
Check D50/YE	0.202	
D50/YE Check	D50/YE is OK	
Basin Length (LB)	12.101	ft
Basin Width	11.077	ft
Apron Length	3.070	ft
Pool Length	9.031	ft
Pool Depth (HS)	0.614	ft
TW/YE	0.284	
Tailwater Depth (TW)	0.428	ft
Average Velocity with TW	5.387	ft/s
Critical Depth (Yc)	0.557	ft
Average Velocity with Yc	4.049	ft/s

APPENDIX E
FLOW ROUTING CALCULATIONS

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Hydrograph Return Period Recap

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

Hyd. No.	Hydrograph type (origin)	Inflow hyd(s)	Peak Outflow (cfs)								Hydrograph Description
			1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	
1	SCS Runoff	-----	-----	1.549	-----	-----	-----	3.332	3.851	4.386	QD2A
2	Reach	1	-----	1.558	-----	-----	-----	3.350	3.869	4.403	FLOW FROM QD2A
3	SCS Runoff	-----	-----	0.668	-----	-----	-----	1.558	1.822	2.096	QD2B
4	Combine	2, 3	-----	2.225	-----	-----	-----	4.908	5.691	6.500	FLOW FROM QD2B
5	Reach	4	-----	2.243	-----	-----	-----	4.916	5.712	6.533	FLOW FROM QD2B
6	SCS Runoff	-----	-----	0.866	-----	-----	-----	1.668	1.896	2.131	QD2C
7	Combine	5, 6	-----	2.745	-----	-----	-----	6.022	6.978	7.965	FLOW FROM QD2C
8	Reach	7	-----	2.770	-----	-----	-----	6.062	7.017	8.002	FLOW FROM QD2C
9	SCS Runoff	-----	-----	1.376	-----	-----	-----	2.975	3.440	3.921	QD2D
10	Combine	8, 9	-----	4.146	-----	-----	-----	9.037	10.46	11.92	FLOW FROM QD2D
11	Reach	10	-----	4.121	-----	-----	-----	9.052	10.47	11.93	FLOW FROM QD2D
12	SCS Runoff	-----	-----	1.055	-----	-----	-----	2.443	2.854	3.280	QD2E
13	Reach	12	-----	1.038	-----	-----	-----	2.457	2.877	3.311	FLOW FROM QD2E
14	SCS Runoff	-----	-----	0.661	-----	-----	-----	1.507	1.757	2.015	QD2F
15	Combine	13, 14	-----	1.434	-----	-----	-----	3.492	4.108	4.748	FLOW FROM QD2F
16	Reach	15	-----	1.436	-----	-----	-----	3.570	4.196	4.844	FLOW FROM QD2F
18	SCS Runoff	-----	-----	0.764	-----	-----	-----	1.768	2.066	2.374	QD2G
19	Combine	11, 16, 18	-----	5.972	-----	-----	-----	13.61	15.83	18.17	FLOW FROM QD2G
20	Reach	19	-----	5.999	-----	-----	-----	13.76	16.03	18.35	FLOW FROM QD2G
21	SCS Runoff	-----	-----	0.603	-----	-----	-----	1.396	1.631	1.875	QD2J
22	Combine	20, 21	-----	6.466	-----	-----	-----	14.99	17.47	20.00	FLOW FROM QD2J
23	Reach	22	-----	6.465	-----	-----	-----	15.03	17.51	20.24	FLOW FROM QD2J
24	SCS Runoff	-----	-----	0.905	-----	-----	-----	2.094	2.446	2.812	QD2M
25	Combine	23, 24	-----	7.070	-----	-----	-----	16.75	19.67	22.72	FLOW FROM QD2M
26	Reach	25	-----	6.971	-----	-----	-----	16.82	19.81	22.95	FLOW FROM QD2M
27	SCS Runoff	-----	-----	0.742	-----	-----	-----	1.706	1.992	2.287	QD2O
28	Combine	26, 27	-----	7.139	-----	-----	-----	17.60	20.72	23.98	FLOW FROM QD2O
29	Reach	28	-----	7.180	-----	-----	-----	17.91	21.36	25.37	FLOW TO POND FROM WEST PIPE
31	SCS Runoff	-----	-----	0.695	-----	-----	-----	1.628	1.906	2.194	QD2P
32	Reach	31	-----	0.694	-----	-----	-----	1.636	1.915	2.204	FLOW FROM QD2P
33	SCS Runoff	-----	-----	0.059	-----	-----	-----	0.105	0.118	0.131	QD2H
34	Combine	32, 33	-----	0.701	-----	-----	-----	1.650	1.930	2.221	FLOW FROM QD2H
35	Reach	34	-----	0.641	-----	-----	-----	1.613	1.903	2.202	FLOW FROM QD2H
Proj. file: 2024-06-21 PROPOSED.gpw									Monday, 06 / 24 / 2024		

Hydrograph Return Period Recap

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

Hyd. No.	Hydrograph type (origin)	Inflow hyd(s)	Peak Outflow (cfs)								Hydrograph Description
			1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	
36	SCS Runoff	-----	-----	0.281	-----	-----	-----	0.651	0.761	0.875	QD2I
37	Combine	35, 36	-----	0.731	-----	-----	-----	1.944	2.306	2.683	FLOW FROM QD2I
38	Reach	37	-----	0.720	-----	-----	-----	1.943	2.306	2.685	FLOW FROM QD2I
40	SCS Runoff	-----	-----	0.909	-----	-----	-----	2.121	2.481	2.854	QD2Q
41	Reach	40	-----	0.910	-----	-----	-----	2.133	2.494	2.868	FLOW FROM QD2Q
42	SCS Runoff	-----	-----	0.864	-----	-----	-----	2.001	2.338	2.687	QD2R
43	Combine	41, 42	-----	1.656	-----	-----	-----	3.921	4.593	5.291	FLOW FROM QD2R
44	Reach	43	-----	1.671	-----	-----	-----	3.950	4.621	5.317	FLOW FROM QD2R
46	SCS Runoff	-----	-----	0.149	-----	-----	-----	0.346	0.405	0.465	QD2K
47	Combine	38, 44, 46	-----	2.412	-----	-----	-----	6.088	7.186	8.324	FLOW FROM QD2K
48	Reach	47	-----	2.405	-----	-----	-----	6.122	7.219	8.352	FLOW FROM QD2K
49	SCS Runoff	-----	-----	0.263	-----	-----	-----	0.611	0.714	0.821	QD2L
50	Combine	48, 49	-----	2.616	-----	-----	-----	6.666	7.853	9.079	FLOW FROM QD2L
51	Reach	50	-----	2.506	-----	-----	-----	6.652	7.852	9.088	FLOW FROM QD2L
53	SCS Runoff	-----	-----	0.969	-----	-----	-----	2.269	2.656	3.058	QD2S
54	Reach	53	-----	0.974	-----	-----	-----	2.282	2.670	3.072	FLOW FROM QD2S
55	SCS Runoff	-----	-----	2.292	-----	-----	-----	5.169	6.017	6.894	QD2T
56	Combine	54, 55	-----	3.266	-----	-----	-----	7.452	8.687	9.967	FLOW FROM QD2T
57	Reach	56	-----	3.289	-----	-----	-----	7.472	8.703	9.977	FLOW FROM QD2T
58	Combine	51, 57	-----	5.775	-----	-----	-----	14.07	16.52	19.03	FLOW AT MANHOLE
59	Reach	58	-----	5.773	-----	-----	-----	14.15	16.57	19.05	FLOW FROM MANHOLE
60	SCS Runoff	-----	-----	0.162	-----	-----	-----	0.300	0.340	0.380	QD2N
61	Combine	59, 60	-----	5.792	-----	-----	-----	14.19	16.61	19.10	FLOW FROM QD2N
62	Reach	61	-----	5.836	-----	-----	-----	14.20	16.69	19.23	FLOW TO POND FROM EAST PIPE
64	SCS Runoff	-----	-----	0.375	-----	-----	-----	2.889	3.847	4.900	QD2X FLOW FROM PARK AREA
65	SCS Runoff	-----	-----	0.267	-----	-----	-----	0.941	1.166	1.407	QD3
67	Combine	29, 62, 64, 65,	-----	13.39	-----	-----	-----	35.13	42.27	50.18	TOTAL FLOW TO POND
69	Reservoir(i)	67	-----	12.23	-----	-----	-----	24.51	26.68	27.60	Stage 3
Proj. file: 2024-06-21 PROPOSED.gpw									Monday, 06 / 24 / 2024		

Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	3.332	2	722	9,481	-----	-----	-----	QD2A
2	Reach	3.350	2	724	9,480	1	-----	-----	FLOW FROM QD2A
3	SCS Runoff	1.558	2	724	4,882	-----	-----	-----	QD2B
4	Combine	4.908	2	724	14,363	2, 3	-----	-----	FLOW FROM QD2B
5	Reach	4.916	2	724	14,363	4	-----	-----	FLOW FROM QD2B
6	SCS Runoff	1.668	2	718	4,038	-----	-----	-----	QD2C
7	Combine	6.022	2	722	18,401	5, 6	-----	-----	FLOW FROM QD2C
8	Reach	6.062	2	724	18,405	7	-----	-----	FLOW FROM QD2C
9	SCS Runoff	2.975	2	724	9,400	-----	-----	-----	QD2D
10	Combine	9.037	2	724	27,806	8, 9	-----	-----	FLOW FROM QD2D
11	Reach	9.052	2	726	27,807	10	-----	-----	FLOW FROM QD2D
12	SCS Runoff	2.443	2	720	6,370	-----	-----	-----	QD2E
13	Reach	2.457	2	722	6,368	12	-----	-----	FLOW FROM QD2E
14	SCS Runoff	1.507	2	716	3,088	-----	-----	-----	QD2F
15	Combine	3.492	2	718	9,456	13, 14	-----	-----	FLOW FROM QD2F
16	Reach	3.570	2	720	9,456	15	-----	-----	FLOW FROM QD2F
18	SCS Runoff	1.768	2	720	4,610	-----	-----	-----	QD2G
19	Combine	13.61	2	724	41,873	11, 16, 18	-----	-----	FLOW FROM QD2G
20	Reach	13.76	2	724	41,879	19	-----	-----	FLOW FROM QD2G
21	SCS Runoff	1.396	2	720	3,640	-----	-----	-----	QD2J
22	Combine	14.99	2	724	45,519	20, 21	-----	-----	FLOW FROM QD2J
23	Reach	15.03	2	726	45,526	22	-----	-----	FLOW FROM QD2J
24	SCS Runoff	2.094	2	720	5,460	-----	-----	-----	QD2M
25	Combine	16.75	2	724	50,986	23, 24	-----	-----	FLOW FROM QD2M
26	Reach	16.82	2	726	50,989	25	-----	-----	FLOW FROM QD2M
27	SCS Runoff	1.706	2	718	3,941	-----	-----	-----	QD2O
28	Combine	17.60	2	726	54,930	26, 27	-----	-----	FLOW FROM QD2O
29	Reach	17.91	2	728	54,968	28	-----	-----	FLOW TO POND FROM WEST PIPE
31	SCS Runoff	1.628	2	726	5,628	-----	-----	-----	QD2P
32	Reach	1.636	2	728	5,626	31	-----	-----	FLOW FROM QD2P
33	SCS Runoff	0.105	2	716	242	-----	-----	-----	QD2H
34	Combine	1.650	2	728	5,868	32, 33	-----	-----	FLOW FROM QD2H
35	Reach	1.613	2	732	5,865	34	-----	-----	FLOW FROM QD2H
2024-06-21 PROPOSED.gpw					Return Period: 25 Year			Monday, 06 / 24 / 2024	

Hydrograph Summary Report

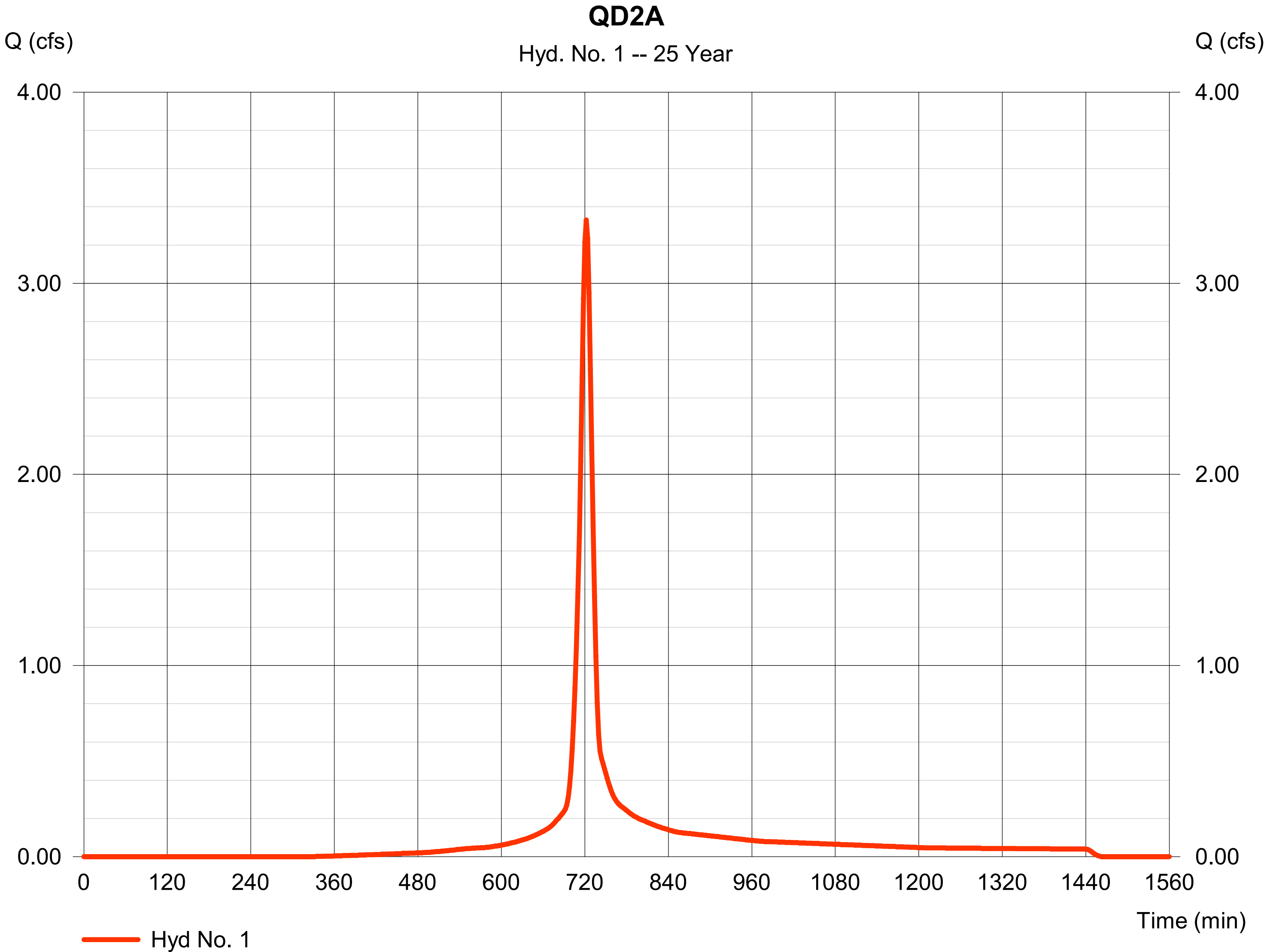
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
36	SCS Runoff	0.651	2	720	1,699	-----	-----	-----	QD2I
37	Combine	1.944	2	728	7,564	35, 36	-----	-----	FLOW FROM QD2I
38	Reach	1.943	2	730	7,564	37	-----	-----	FLOW FROM QD2I
40	SCS Runoff	2.121	2	724	6,647	-----	-----	-----	QD2Q
41	Reach	2.133	2	726	6,645	40	-----	-----	FLOW FROM QD2Q
42	SCS Runoff	2.001	2	720	5,217	-----	-----	-----	QD2R
43	Combine	3.921	2	722	11,862	41, 42	-----	-----	FLOW FROM QD2R
44	Reach	3.950	2	724	11,864	43	-----	-----	FLOW FROM QD2R
46	SCS Runoff	0.346	2	722	975	-----	-----	-----	QD2K
47	Combine	6.088	2	724	20,402	38, 44, 46	-----	-----	FLOW FROM QD2K
48	Reach	6.122	2	726	20,403	47	-----	-----	FLOW FROM QD2K
49	SCS Runoff	0.611	2	722	1,721	-----	-----	-----	QD2L
50	Combine	6.666	2	726	22,124	48, 49	-----	-----	FLOW FROM QD2L
51	Reach	6.652	2	728	22,124	50	-----	-----	FLOW FROM QD2L
53	SCS Runoff	2.269	2	726	7,844	-----	-----	-----	QD2S
54	Reach	2.282	2	728	7,841	53	-----	-----	FLOW FROM QD2S
55	SCS Runoff	5.169	2	728	19,032	-----	-----	-----	QD2T
56	Combine	7.452	2	728	26,873	54, 55	-----	-----	FLOW FROM QD2T
57	Reach	7.472	2	730	26,871	56	-----	-----	FLOW FROM QD2T
58	Combine	14.07	2	728	48,995	51, 57	-----	-----	FLOW AT MANHOLE
59	Reach	14.15	2	730	48,995	58	-----	-----	FLOW FROM MANHOLE
60	SCS Runoff	0.300	2	716	661	-----	-----	-----	QD2N
61	Combine	14.19	2	730	49,657	59, 60	-----	-----	FLOW FROM QD2N
62	Reach	14.20	2	730	49,663	61	-----	-----	FLOW TO POND FROM EAST PIPE
64	SCS Runoff	2.889	2	730	12,005	-----	-----	-----	QD2X FLOW FROM PARK AREA
65	SCS Runoff	0.941	2	722	2,692	-----	-----	-----	QD3
67	Combine	35.13	2	728	119,328	29, 62, 64, 65,	-----	-----	TOTAL FLOW TO POND
69	Reservoir(i)	24.51	2	736	84,038	67	6476.21	17,104	Stage 3
2024-06-21 PROPOSED.gpw					Return Period: 25 Year			Monday, 06 / 24 / 2024	

Hyd. No. 1

QD2A

Hydrograph type	= SCS Runoff	Peak discharge	= 3.332 cfs
Storm frequency	= 25 yrs	Time to peak	= 722 min
Time interval	= 2 min	Hyd. volume	= 9,481 cuft
Drainage area	= 1.500 ac	Curve number	= 92
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 14.00 min
Total precip.	= 2.60 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

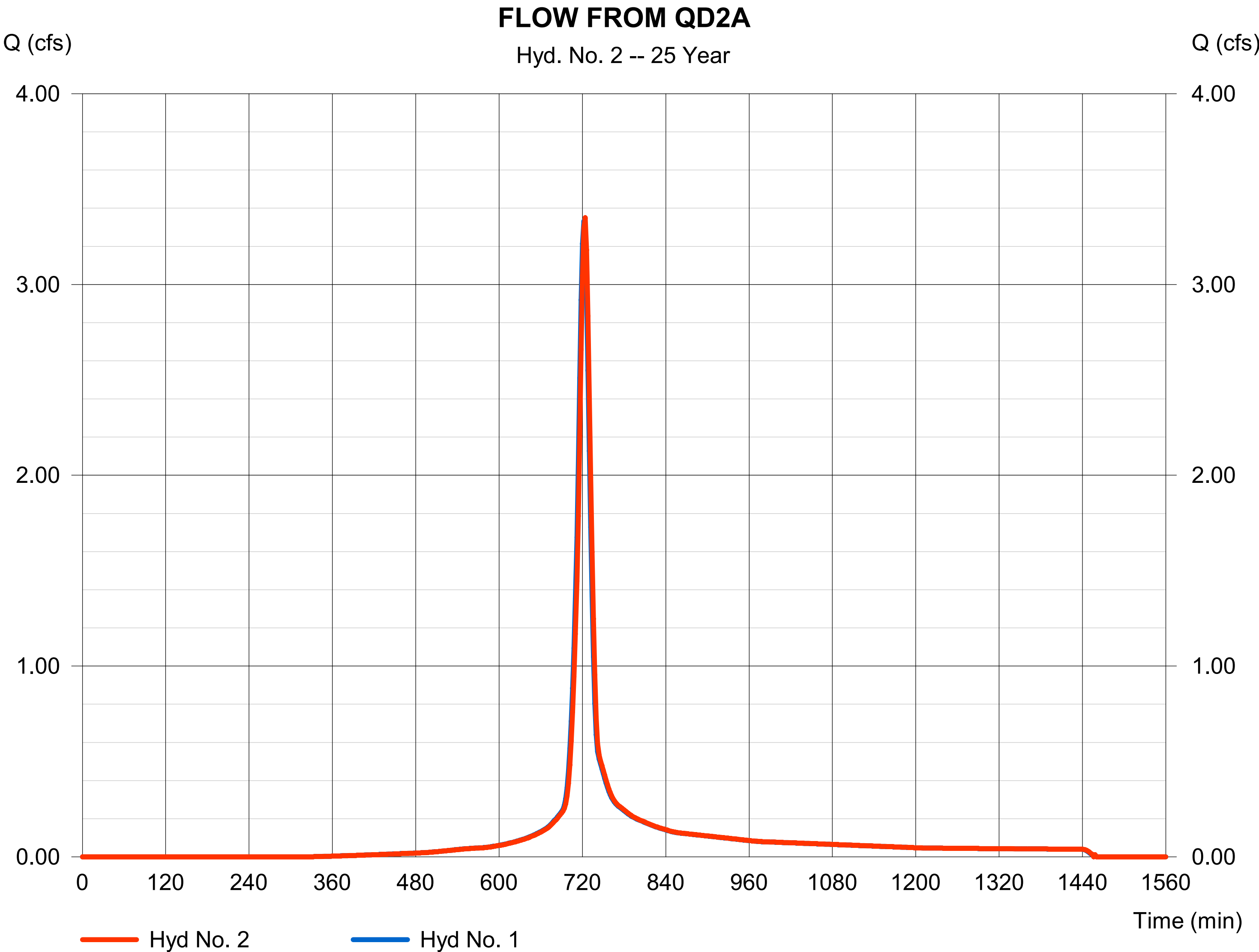
Monday, 06 / 24 / 2024

Hyd. No. 2

FLOW FROM QD2A

Hydrograph type	= Reach	Peak discharge	= 3.350 cfs
Storm frequency	= 25 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 9,480 cuft
Inflow hyd. No.	= 1 - QD2A	Section type	= Circular
Reach length	= 47.7 ft	Channel slope	= 0.9 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 3.438	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.4952

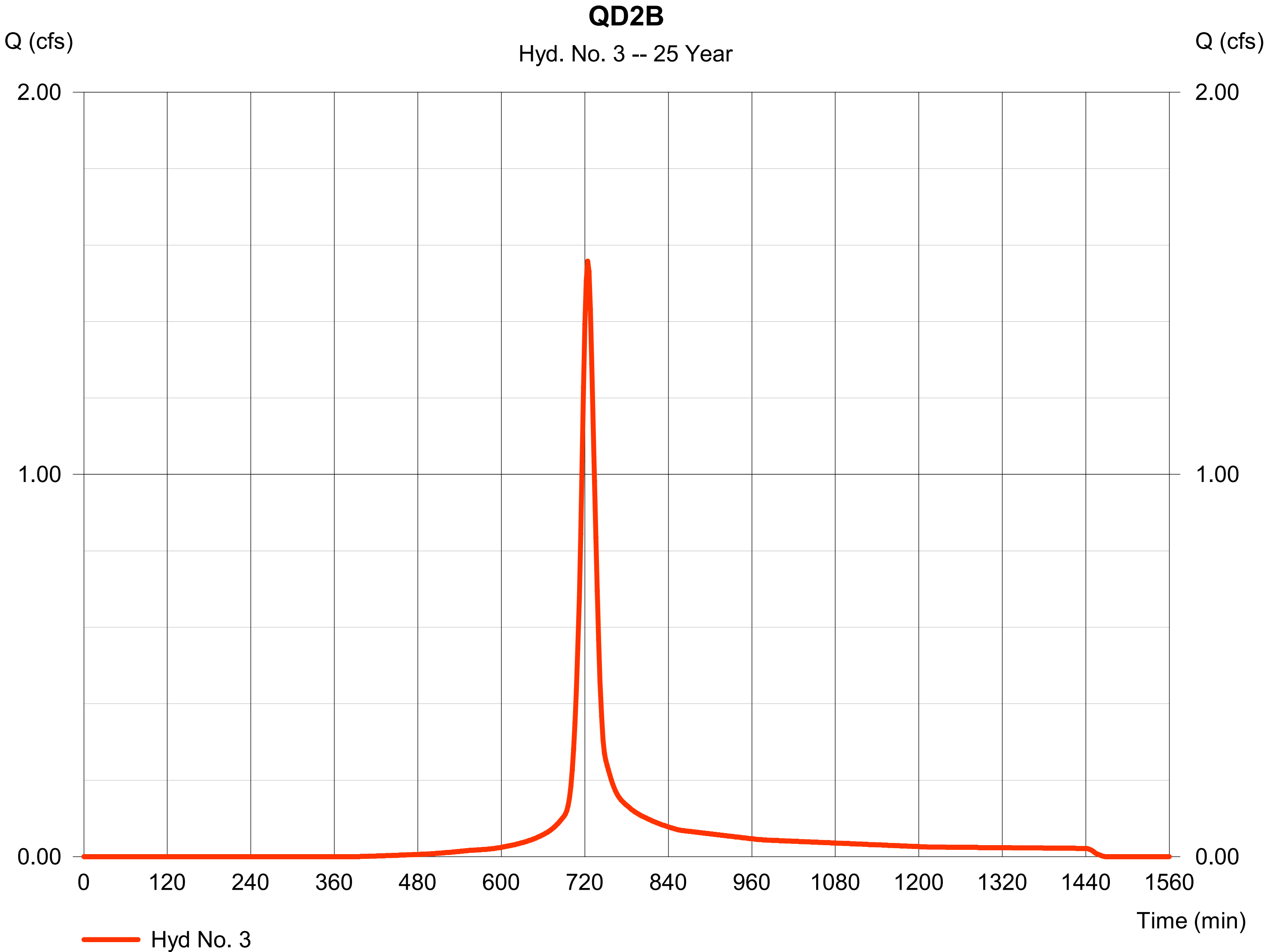
Modified Att-Kin routing method used.



Hyd. No. 3

QD2B

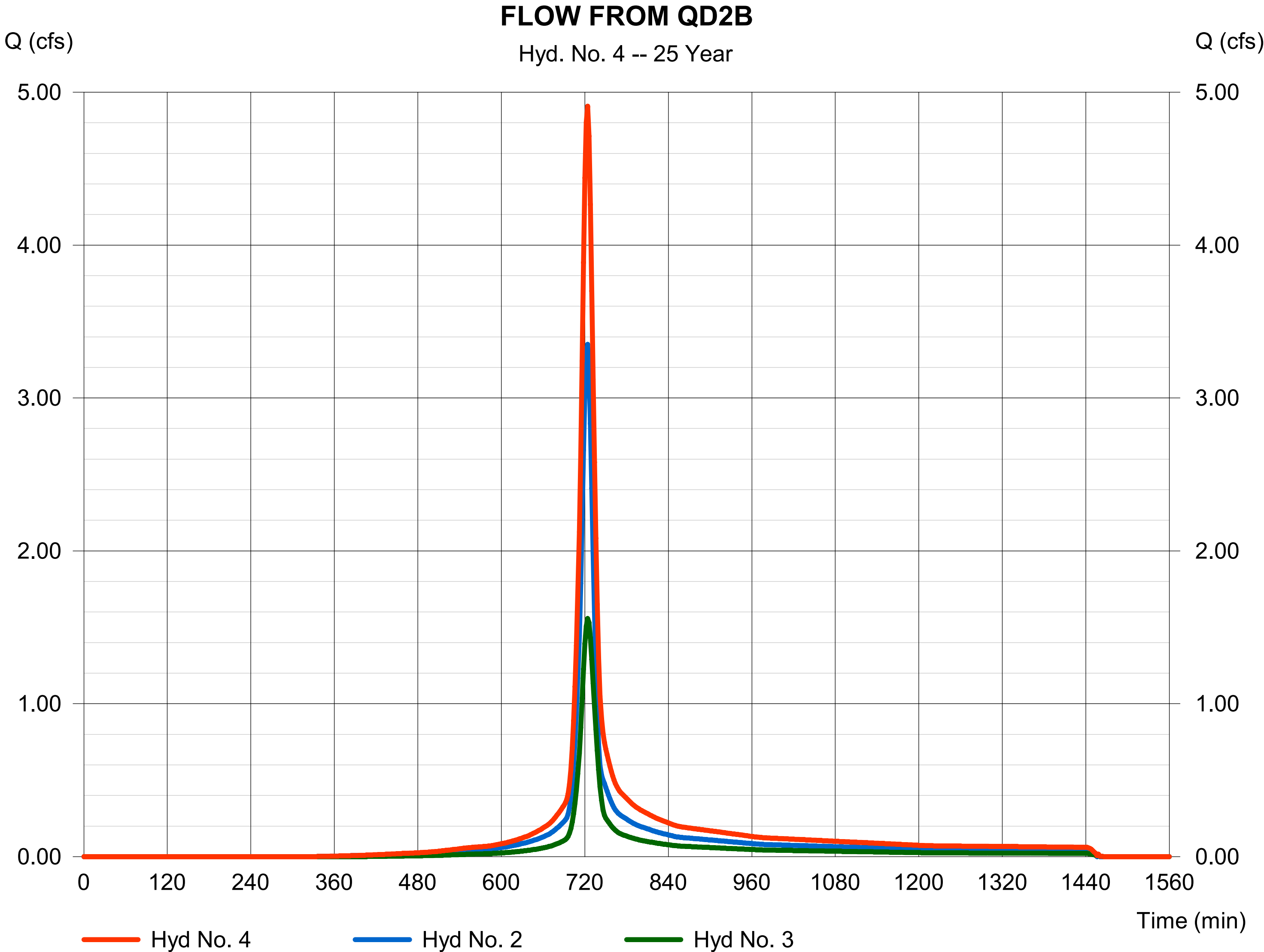
Hydrograph type	= SCS Runoff	Peak discharge	= 1.558 cfs
Storm frequency	= 25 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 4,882 cuft
Drainage area	= 0.830 ac	Curve number	= 90
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 16.90 min
Total precip.	= 2.60 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hyd. No. 4

FLOW FROM QD2B

Hydrograph type	= Combine	Peak discharge	= 4.908 cfs
Storm frequency	= 25 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 14,363 cuft
Inflow hyds.	= 2, 3	Contrib. drain. area	= 0.830 ac

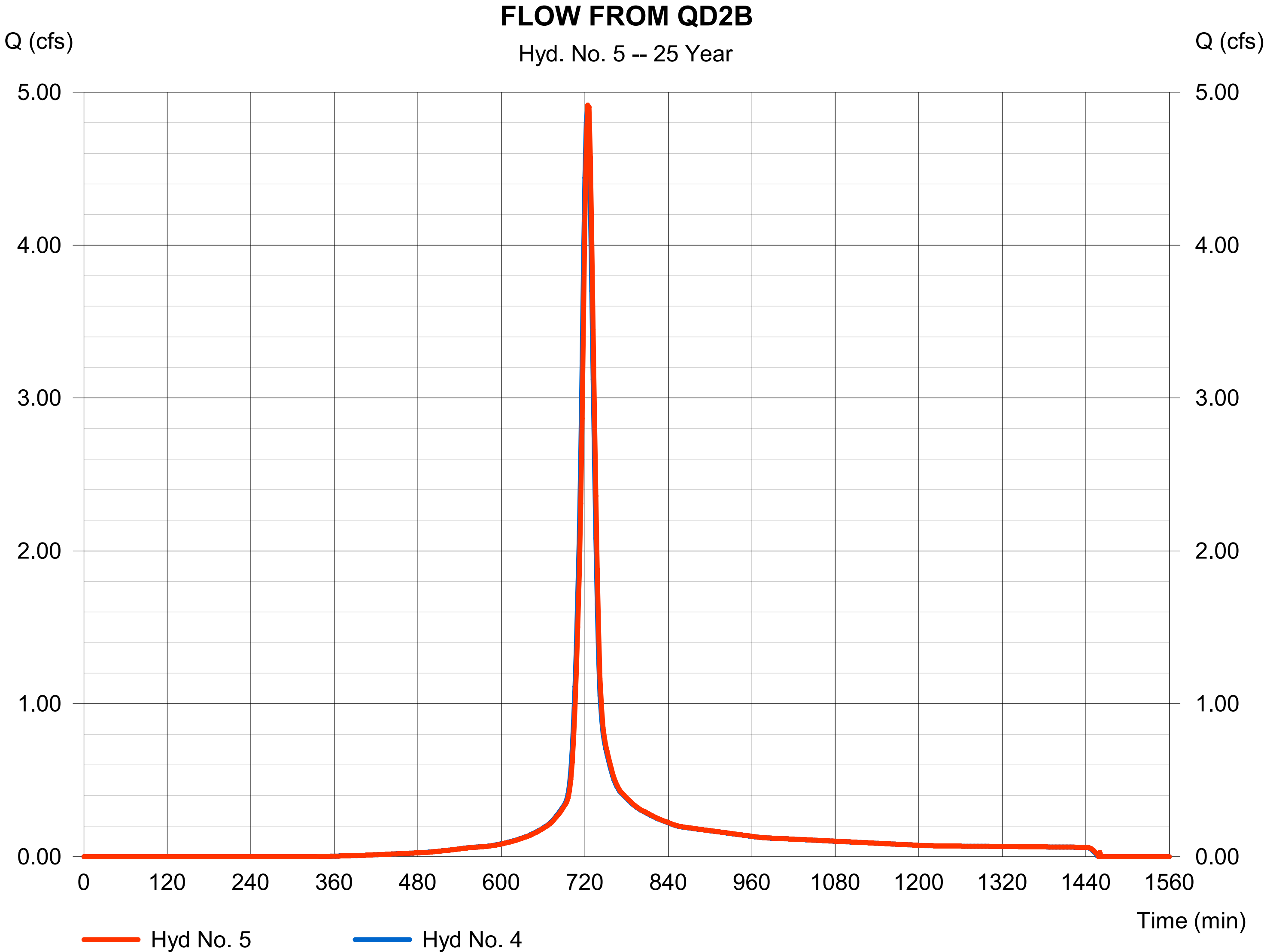


Hyd. No. 5

FLOW FROM QD2B

Hydrograph type	= Reach	Peak discharge	= 4.916 cfs
Storm frequency	= 25 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 14,363 cuft
Inflow hyd. No.	= 4 - FLOW FROM QD2B	Section type	= Circular
Reach length	= 32.2 ft	Channel slope	= 0.9 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 3.438	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.7324

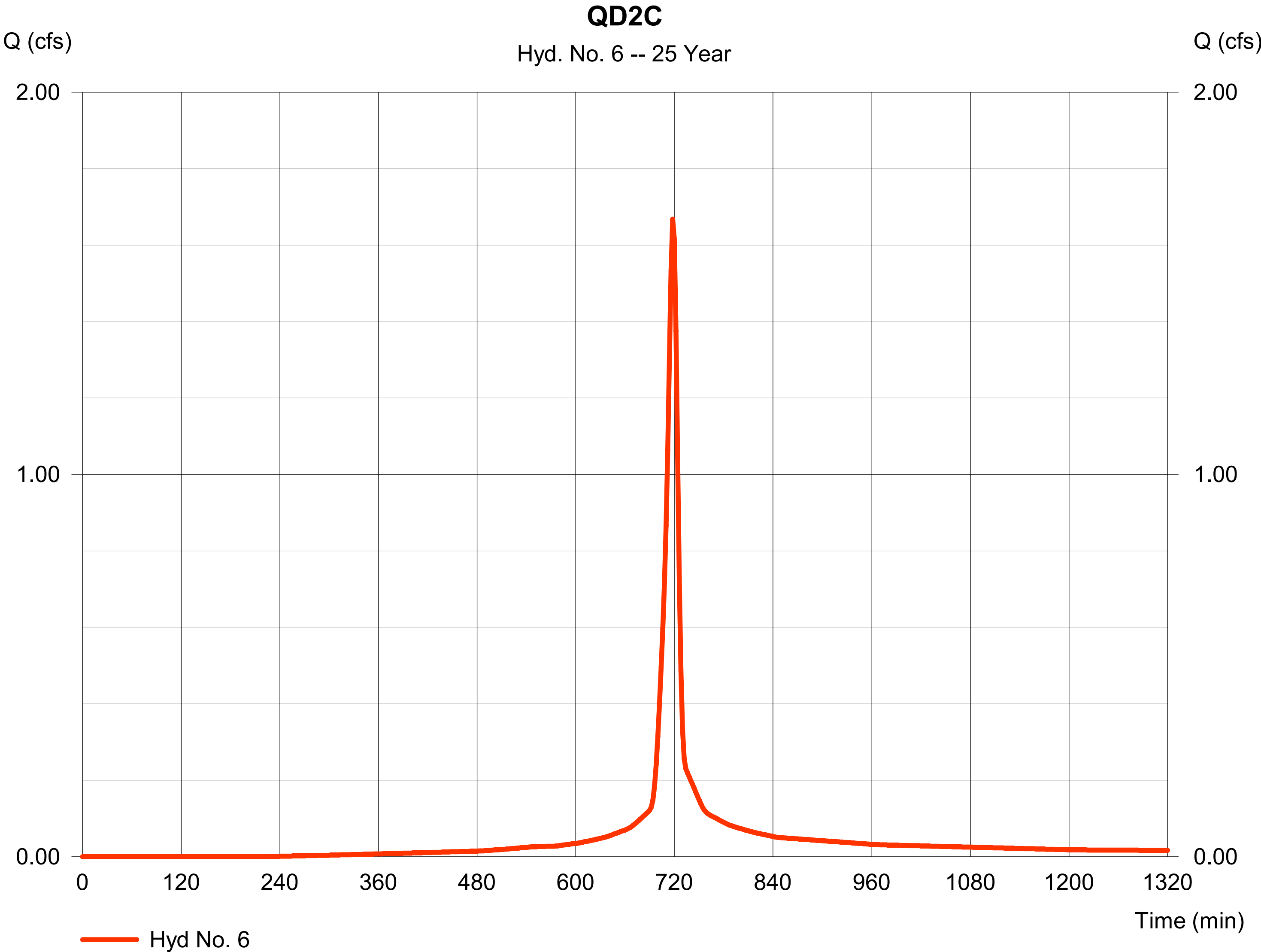
Modified Att-Kin routing method used.



Hyd. No. 6

QD2C

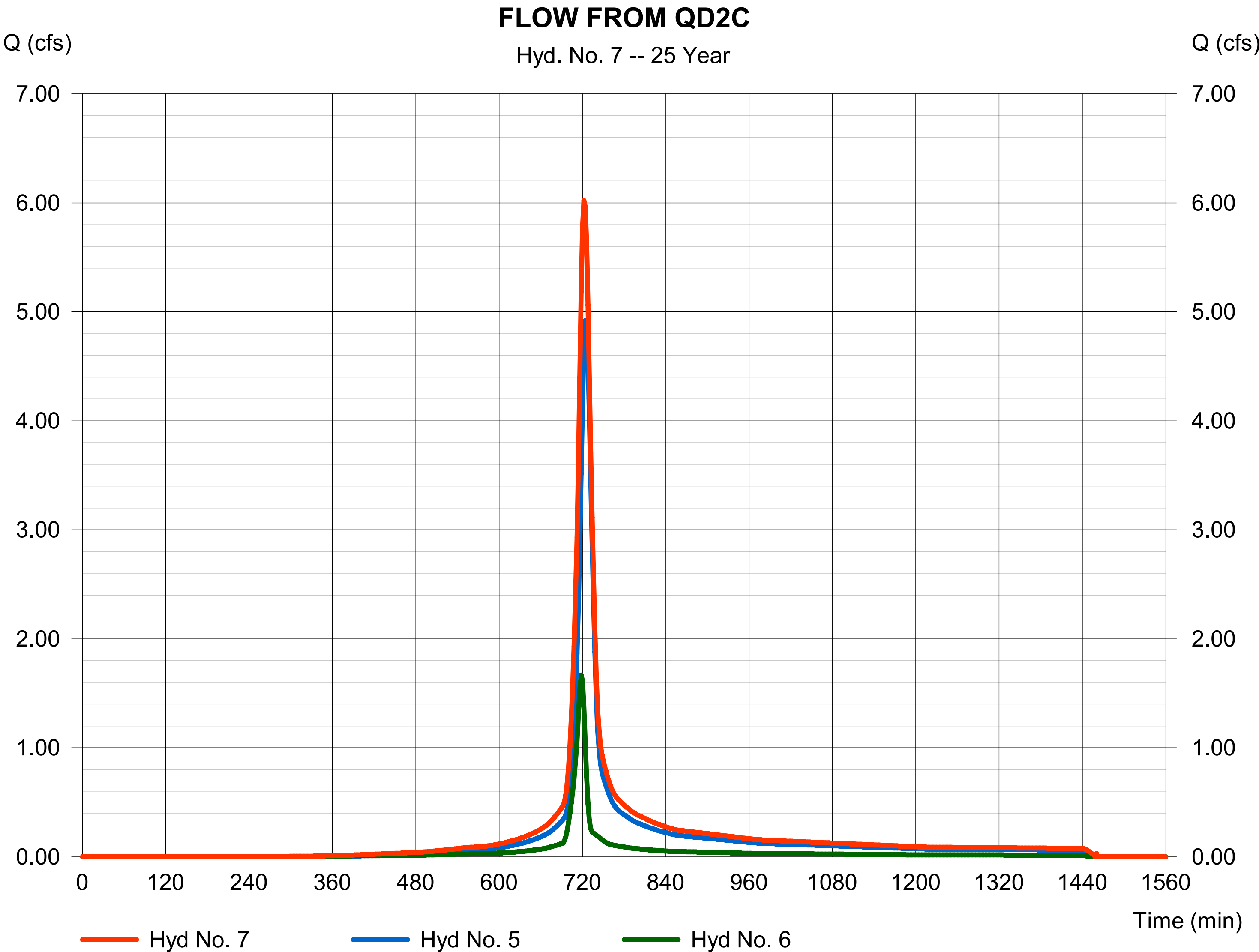
Hydrograph type	= SCS Runoff	Peak discharge	= 1.668 cfs
Storm frequency	= 25 yrs	Time to peak	= 718 min
Time interval	= 2 min	Hyd. volume	= 4,038 cuft
Drainage area	= 0.540 ac	Curve number	= 95
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 9.50 min
Total precip.	= 2.60 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hyd. No. 7

FLOW FROM QD2C

Hydrograph type	= Combine	Peak discharge	= 6.022 cfs
Storm frequency	= 25 yrs	Time to peak	= 722 min
Time interval	= 2 min	Hyd. volume	= 18,401 cuft
Inflow hyds.	= 5, 6	Contrib. drain. area	= 0.540 ac

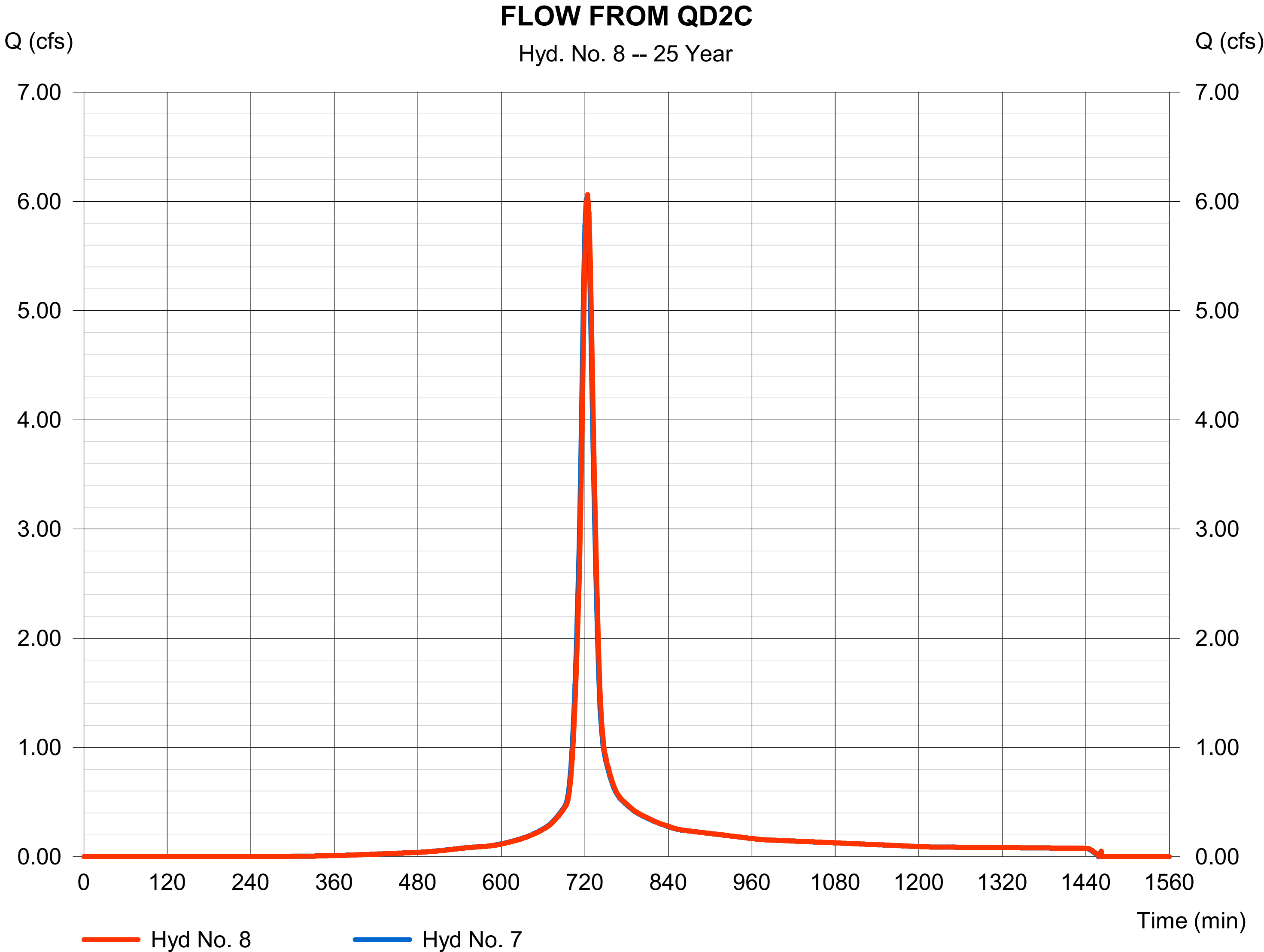


Hyd. No. 8

FLOW FROM QD2C

Hydrograph type	= Reach	Peak discharge	= 6.062 cfs
Storm frequency	= 25 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 18,405 cuft
Inflow hyd. No.	= 7 - FLOW FROM QD2C	Section type	= Circular
Reach length	= 41.0 ft	Channel slope	= 0.9 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 3.438	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.7238

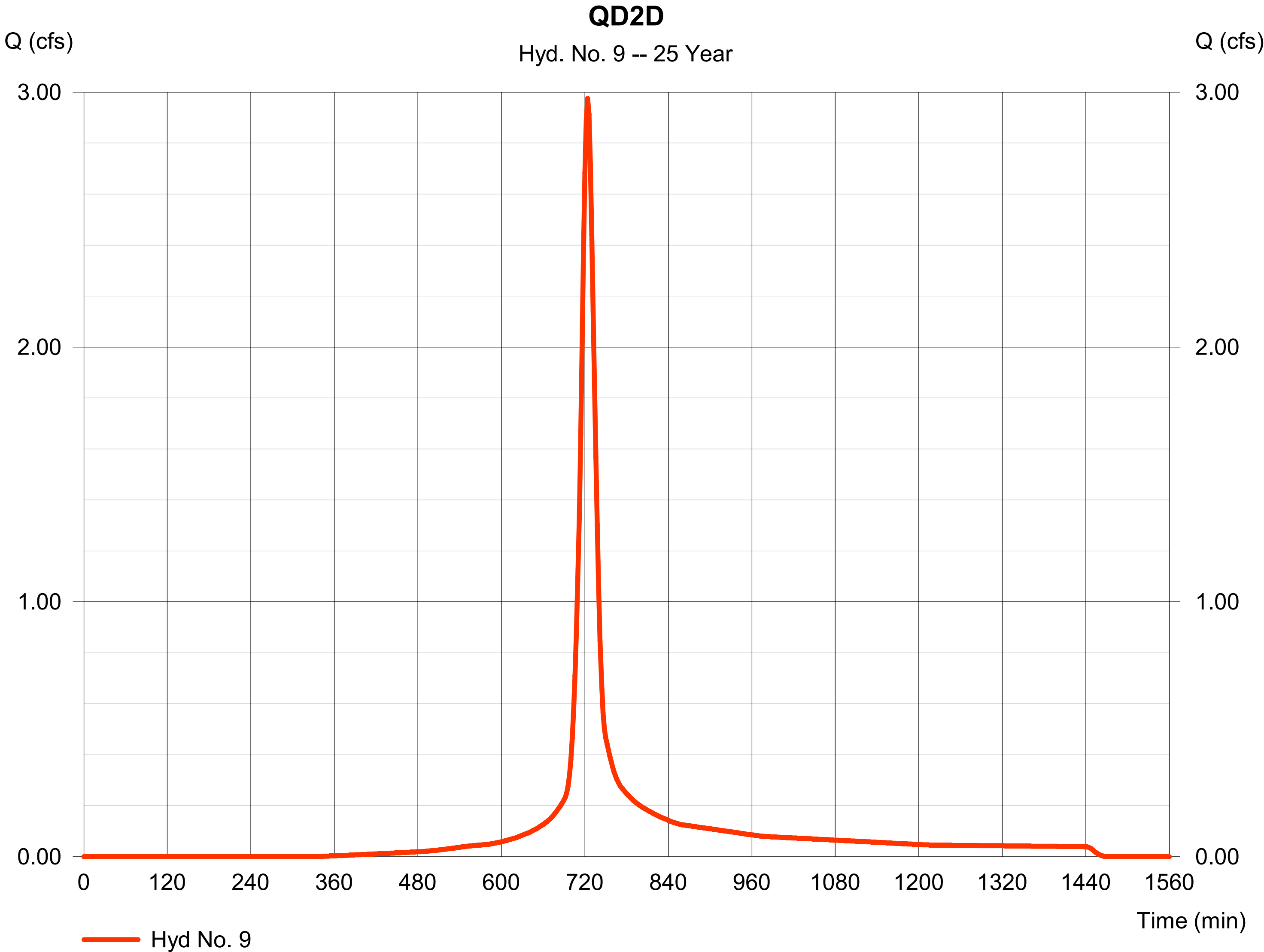
Modified Att-Kin routing method used.



Hyd. No. 9

QD2D

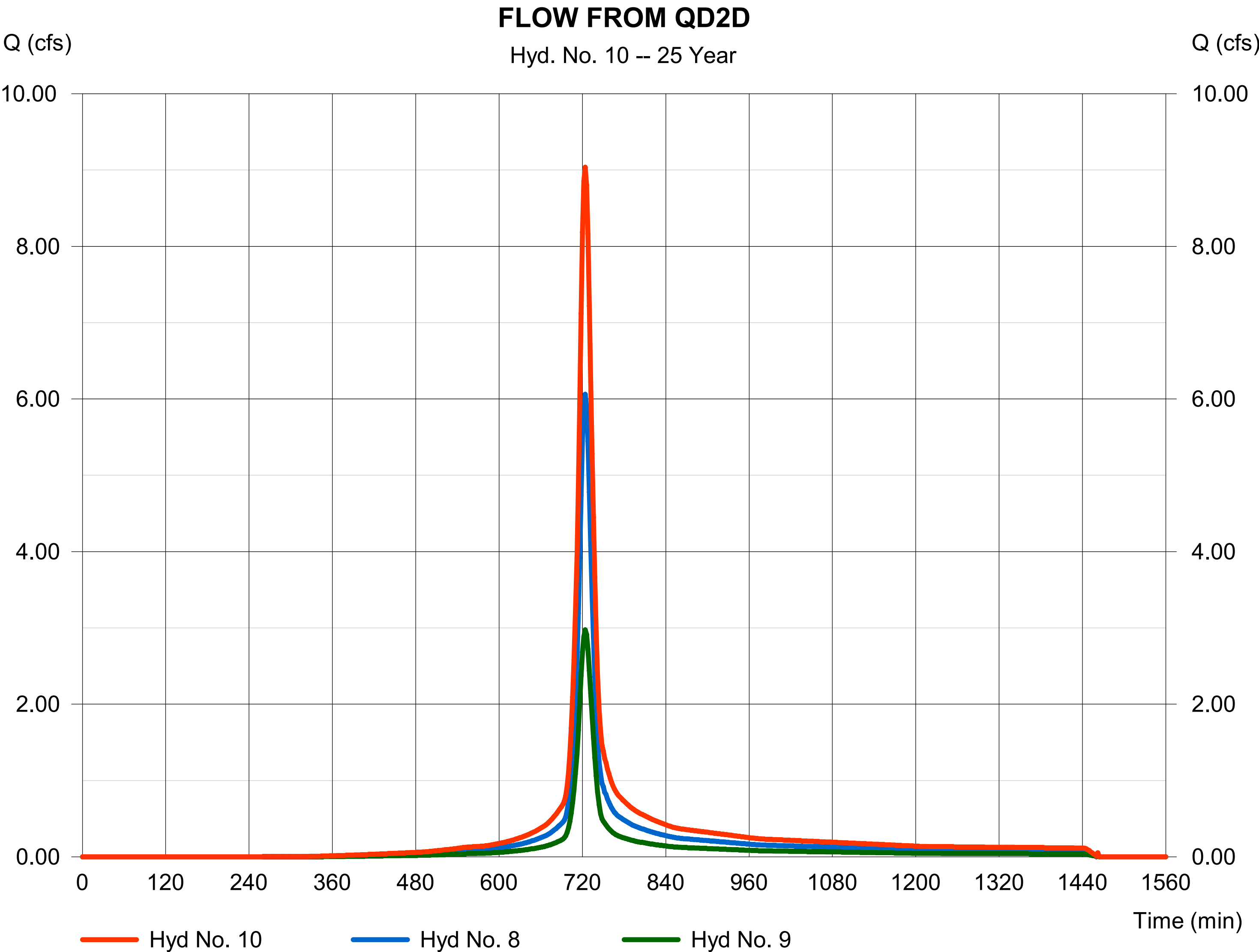
Hydrograph type	= SCS Runoff	Peak discharge	= 2.975 cfs
Storm frequency	= 25 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 9,400 cuft
Drainage area	= 1.450 ac	Curve number	= 92
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 18.60 min
Total precip.	= 2.60 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hyd. No. 10

FLOW FROM QD2D

Hydrograph type	= Combine	Peak discharge	= 9.037 cfs
Storm frequency	= 25 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 27,806 cuft
Inflow hyds.	= 8, 9	Contrib. drain. area	= 1.450 ac

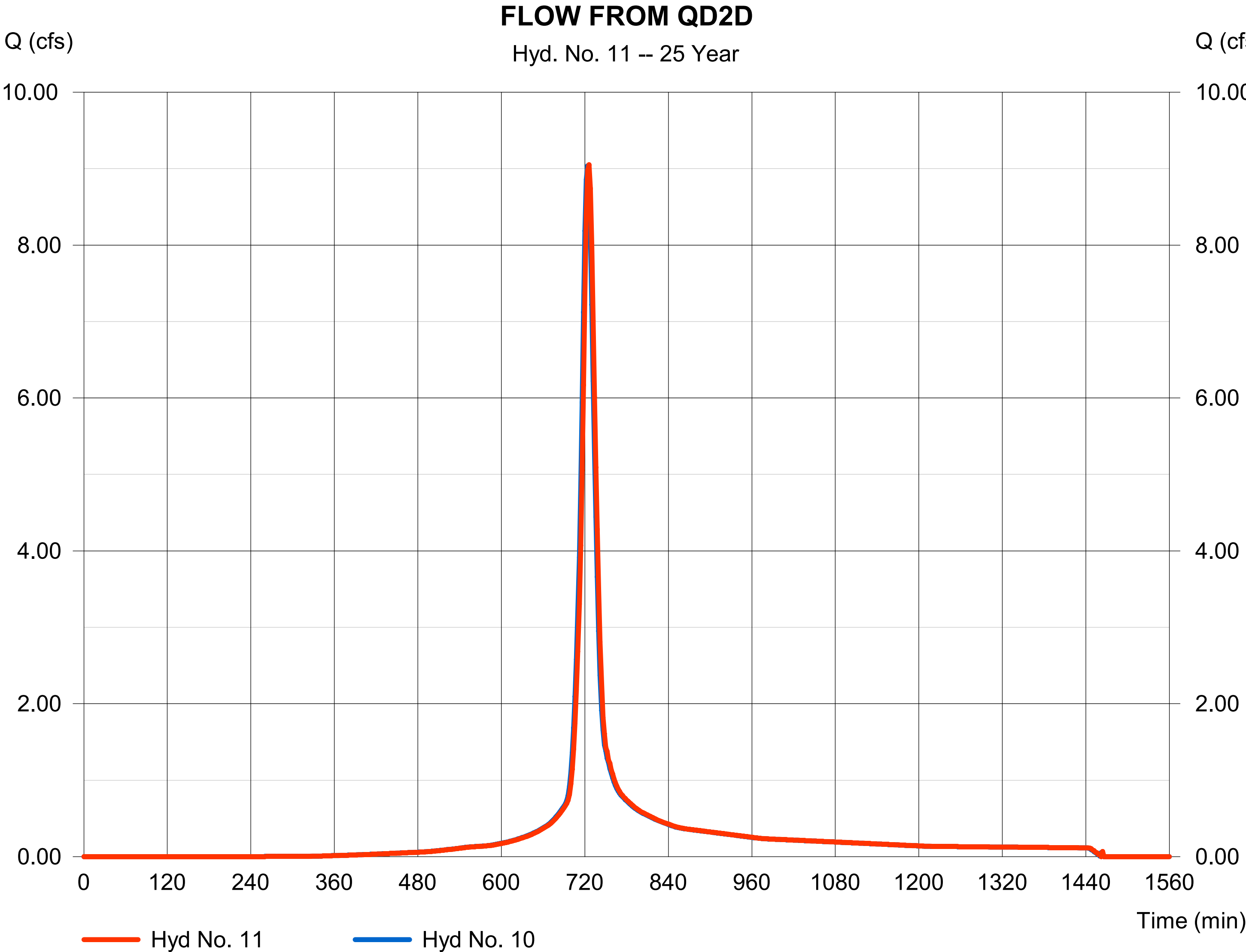


Hyd. No. 11

FLOW FROM QD2D

Hydrograph type	= Reach	Peak discharge	= 9.052 cfs
Storm frequency	= 25 yrs	Time to peak	= 726 min
Time interval	= 2 min	Hyd. volume	= 27,807 cuft
Inflow hyd. No.	= 10 - FLOW FROM QD2D	Section type	= Circular
Reach length	= 225.0 ft	Channel slope	= 0.9 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 3.438	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.2605

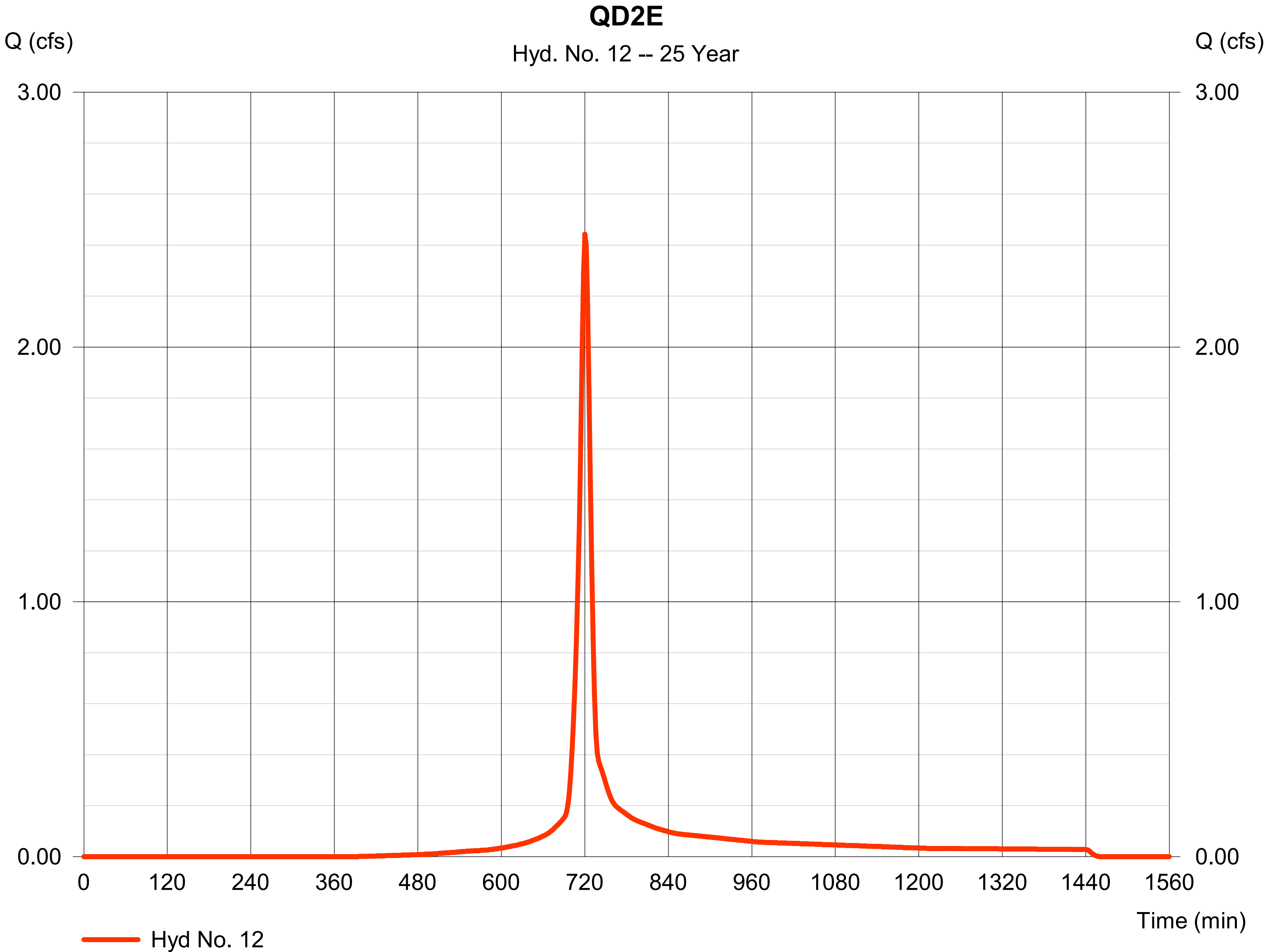
Modified Att-Kin routing method used.



Hyd. No. 12

QD2E

Hydrograph type	= SCS Runoff	Peak discharge	= 2.443 cfs
Storm frequency	= 25 yrs	Time to peak	= 720 min
Time interval	= 2 min	Hyd. volume	= 6,370 cuft
Drainage area	= 1.050 ac	Curve number	= 90
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 11.90 min
Total precip.	= 2.60 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

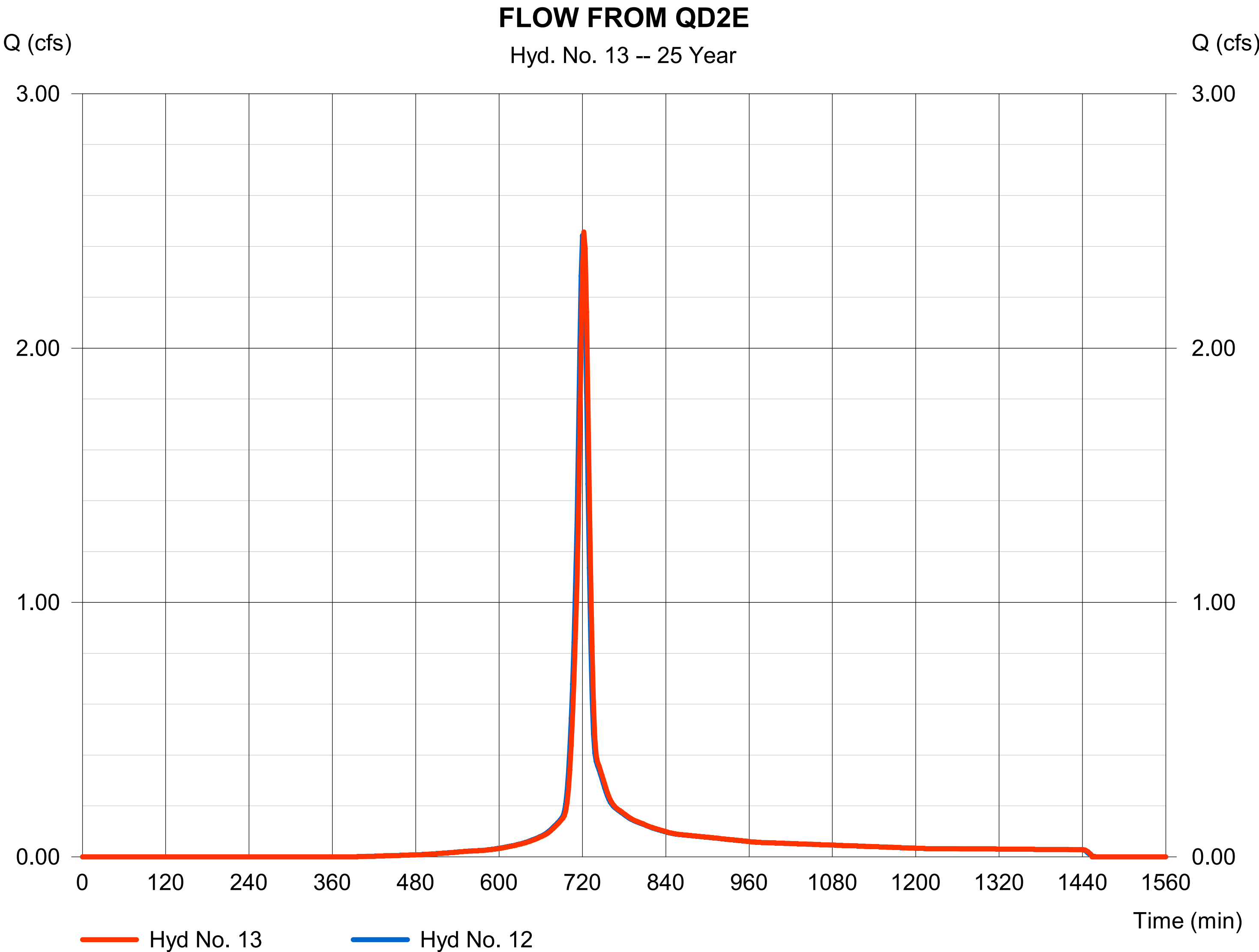


Hyd. No. 13

FLOW FROM QD2E

Hydrograph type	= Reach	Peak discharge	= 2.457 cfs
Storm frequency	= 25 yrs	Time to peak	= 722 min
Time interval	= 2 min	Hyd. volume	= 6,368 cuft
Inflow hyd. No.	= 12 - QD2E	Section type	= Circular
Reach length	= 84.5 ft	Channel slope	= 0.5 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.534	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.1020

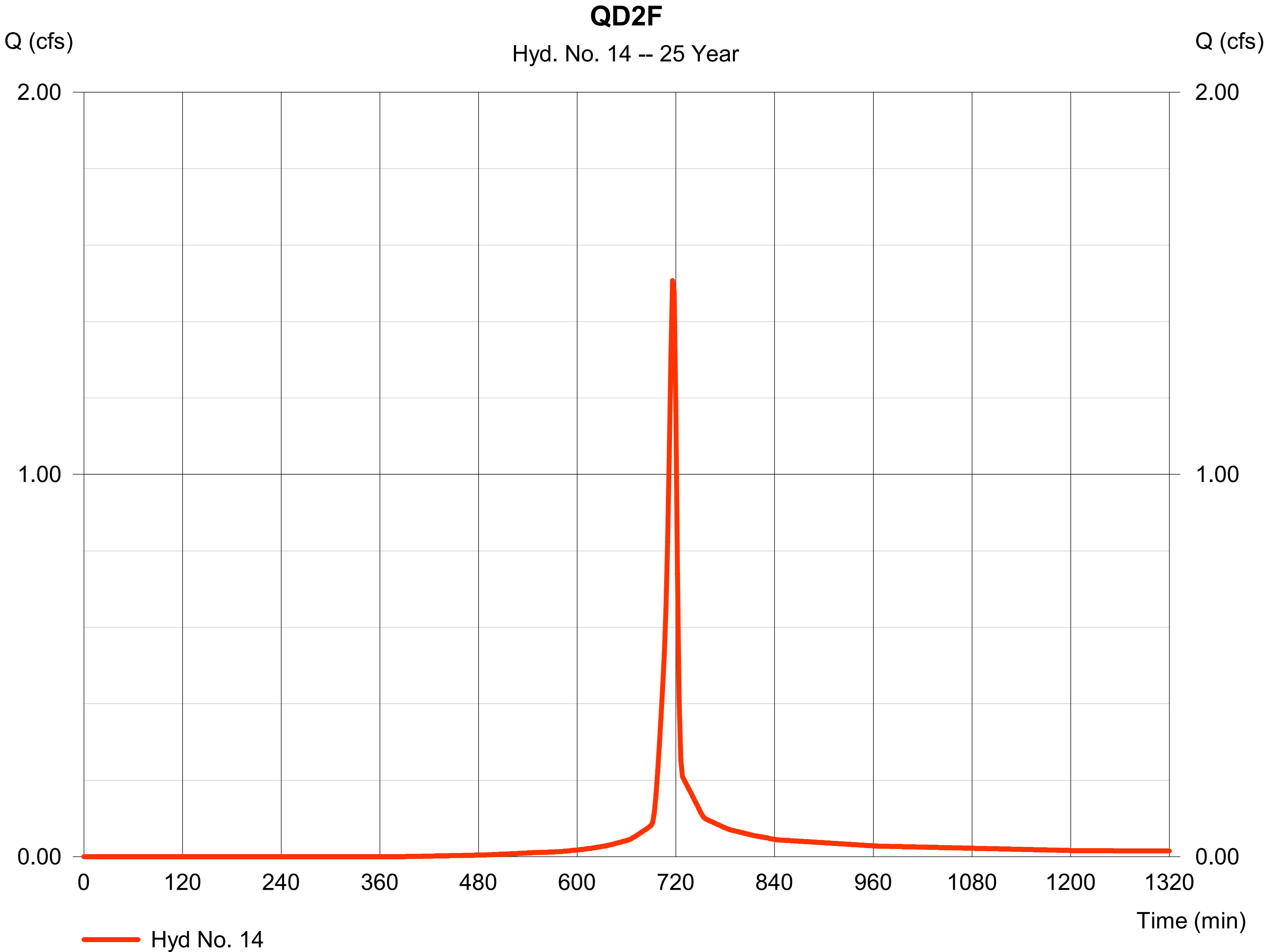
Modified Att-Kin routing method used.



Hyd. No. 14

QD2F

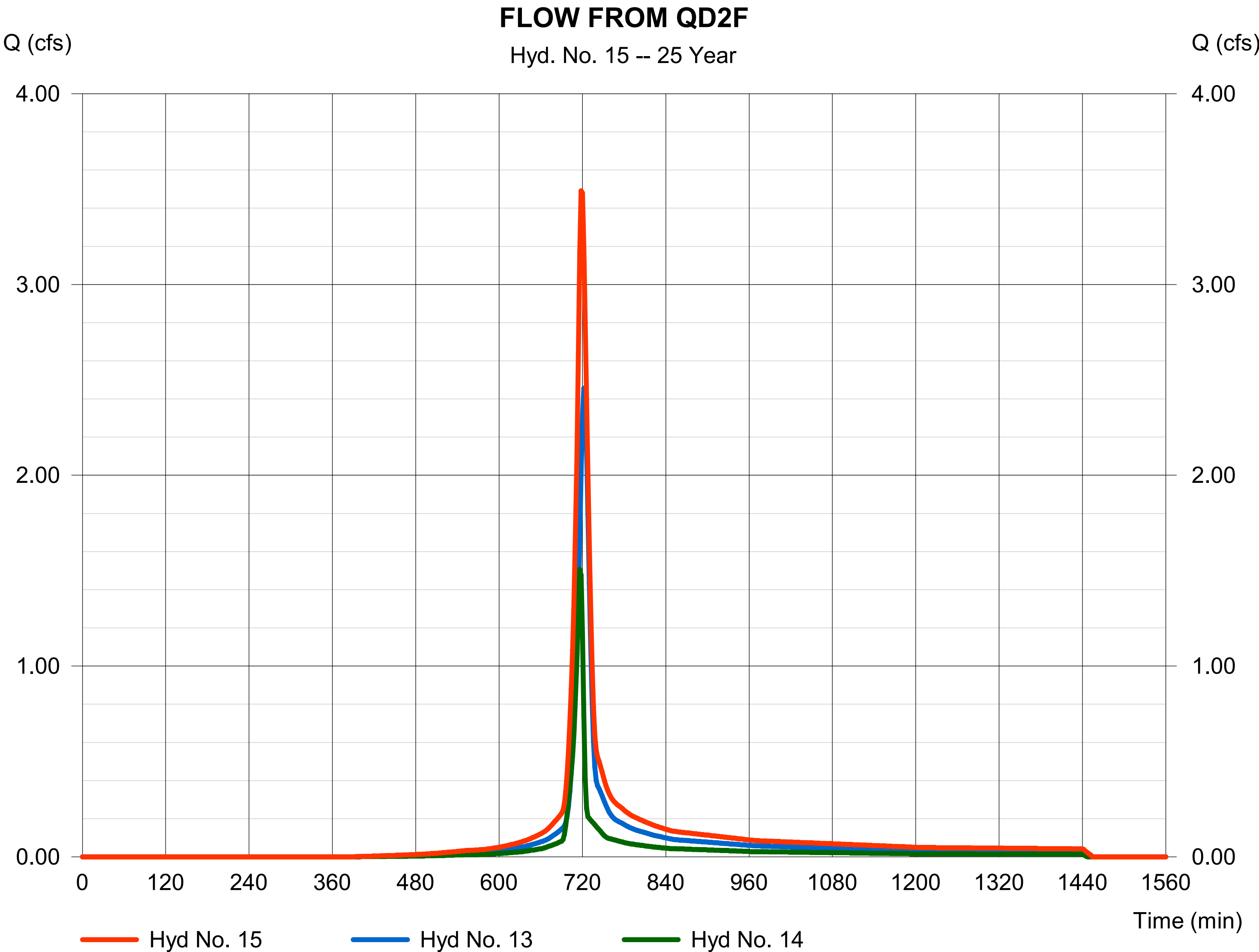
Hydrograph type	= SCS Runoff	Peak discharge	= 1.507 cfs
Storm frequency	= 25 yrs	Time to peak	= 716 min
Time interval	= 2 min	Hyd. volume	= 3,088 cuft
Drainage area	= 0.560 ac	Curve number	= 90
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 6.00 min
Total precip.	= 2.60 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hyd. No. 15

FLOW FROM QD2F

Hydrograph type	= Combine	Peak discharge	= 3.492 cfs
Storm frequency	= 25 yrs	Time to peak	= 718 min
Time interval	= 2 min	Hyd. volume	= 9,456 cuft
Inflow hyds.	= 13, 14	Contrib. drain. area	= 0.560 ac

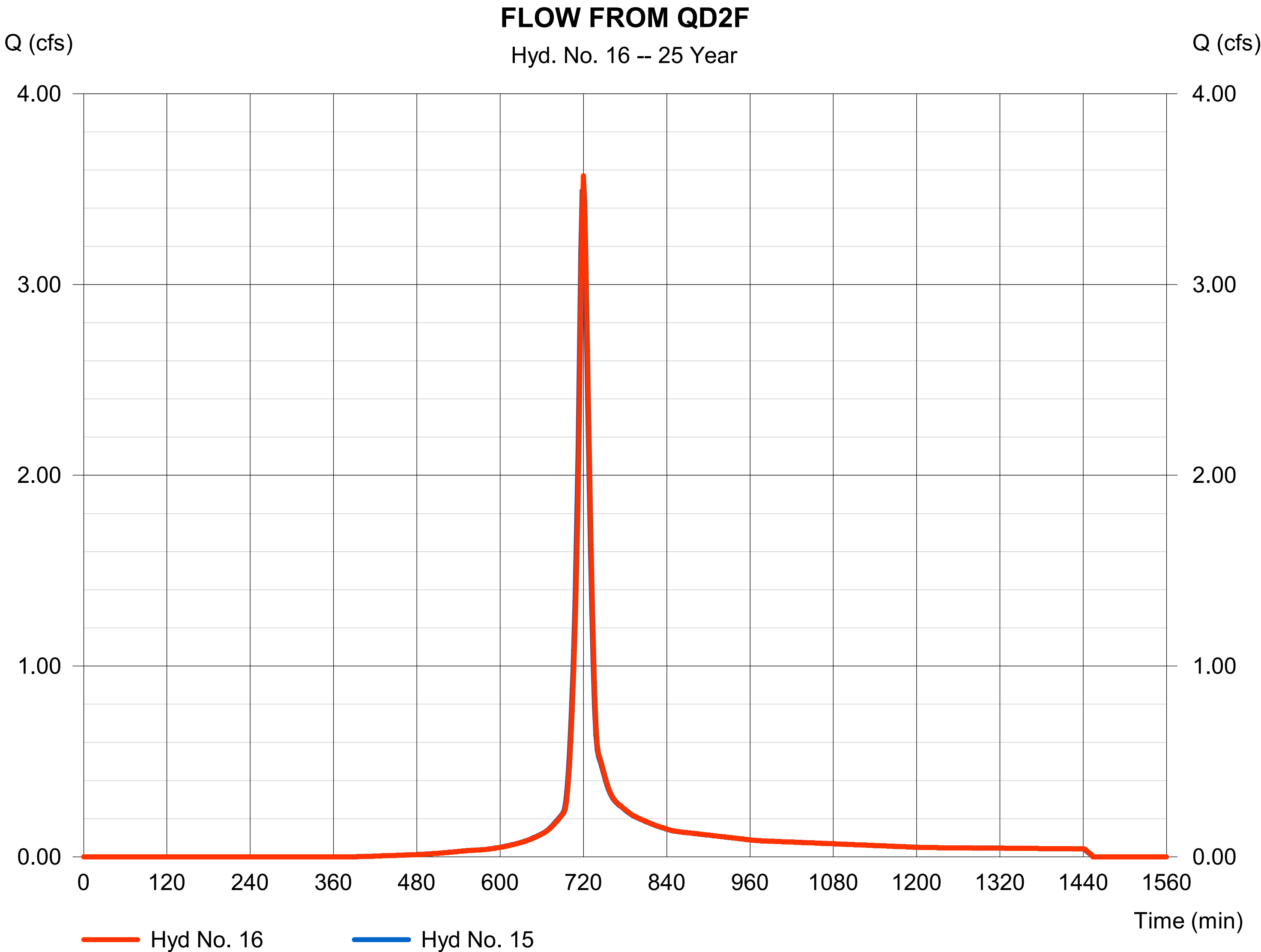


Hyd. No. 16

FLOW FROM QD2F

Hydrograph type	= Reach	Peak discharge	= 3.570 cfs
Storm frequency	= 25 yrs	Time to peak	= 720 min
Time interval	= 2 min	Hyd. volume	= 9,456 cuft
Inflow hyd. No.	= 15 - FLOW FROM QD2F	Section type	= Circular
Reach length	= 40.2 ft	Channel slope	= 0.5 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.534	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.5735

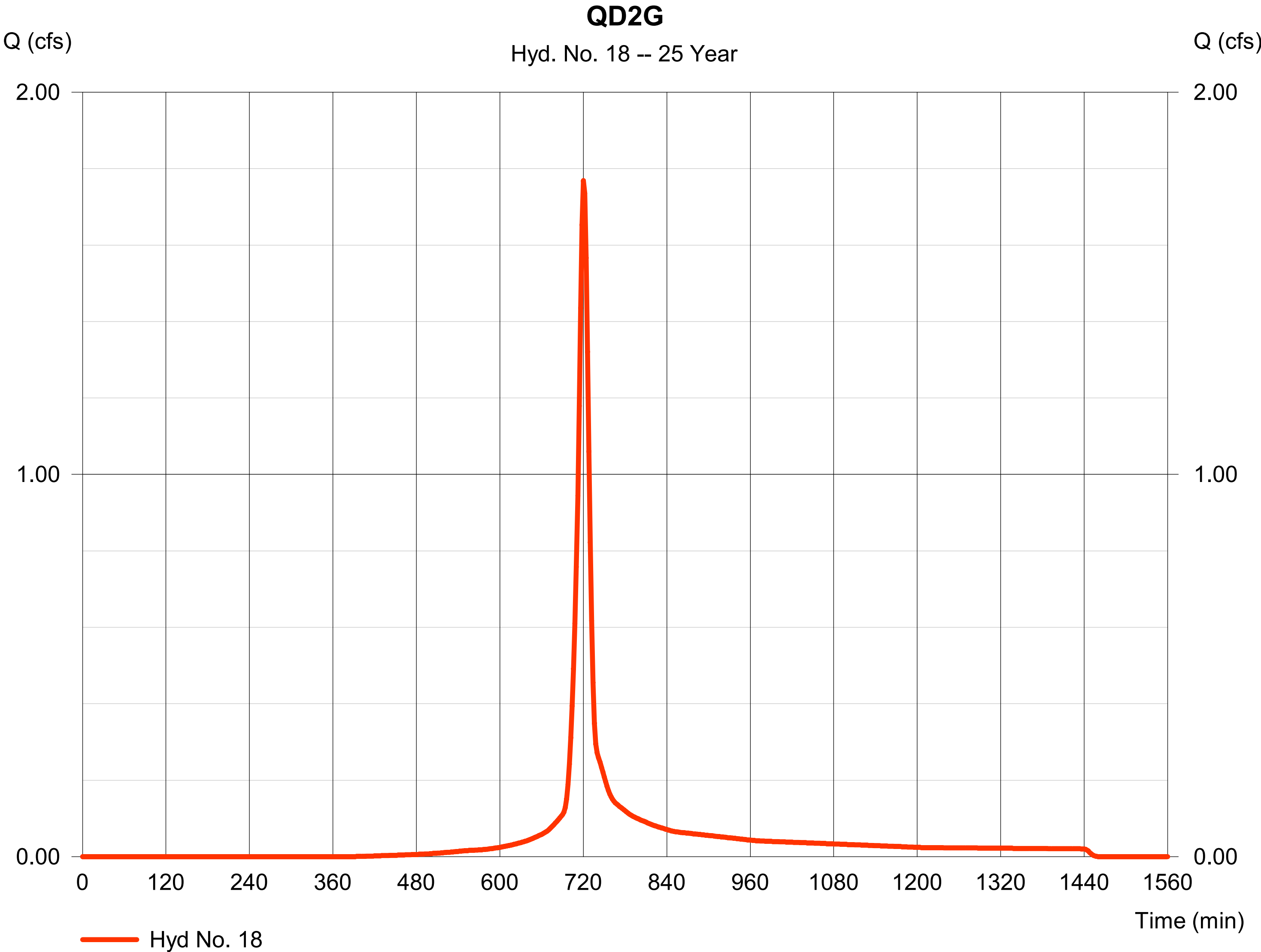
Modified Att-Kin routing method used.



Hyd. No. 18

QD2G

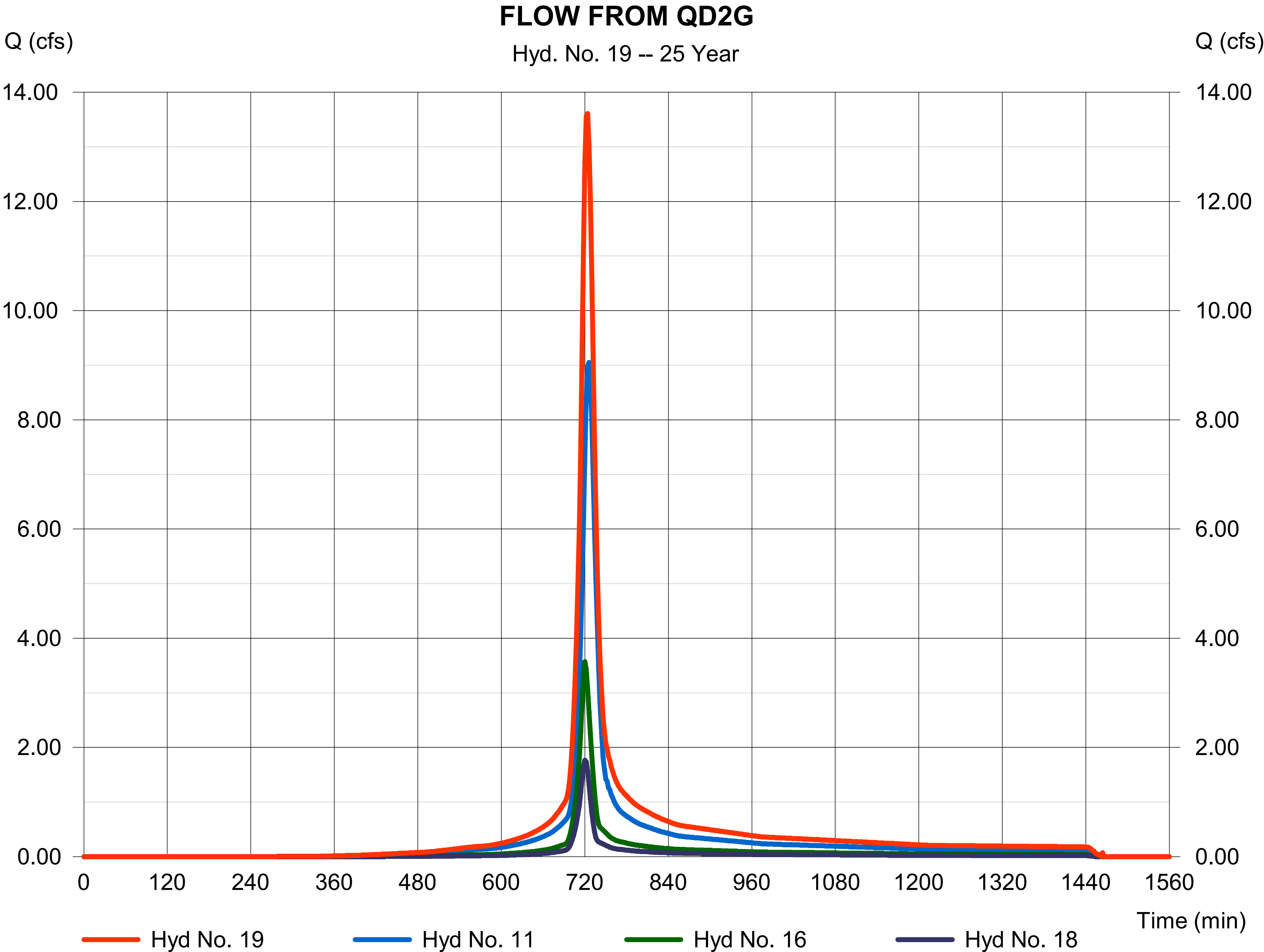
Hydrograph type	= SCS Runoff	Peak discharge	= 1.768 cfs
Storm frequency	= 25 yrs	Time to peak	= 720 min
Time interval	= 2 min	Hyd. volume	= 4,610 cuft
Drainage area	= 0.760 ac	Curve number	= 90
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 12.80 min
Total precip.	= 2.60 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hyd. No. 19

FLOW FROM QD2G

Hydrograph type	= Combine	Peak discharge	= 13.61 cfs
Storm frequency	= 25 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 41,873 cuft
Inflow hyds.	= 11, 16, 18	Contrib. drain. area	= 0.760 ac



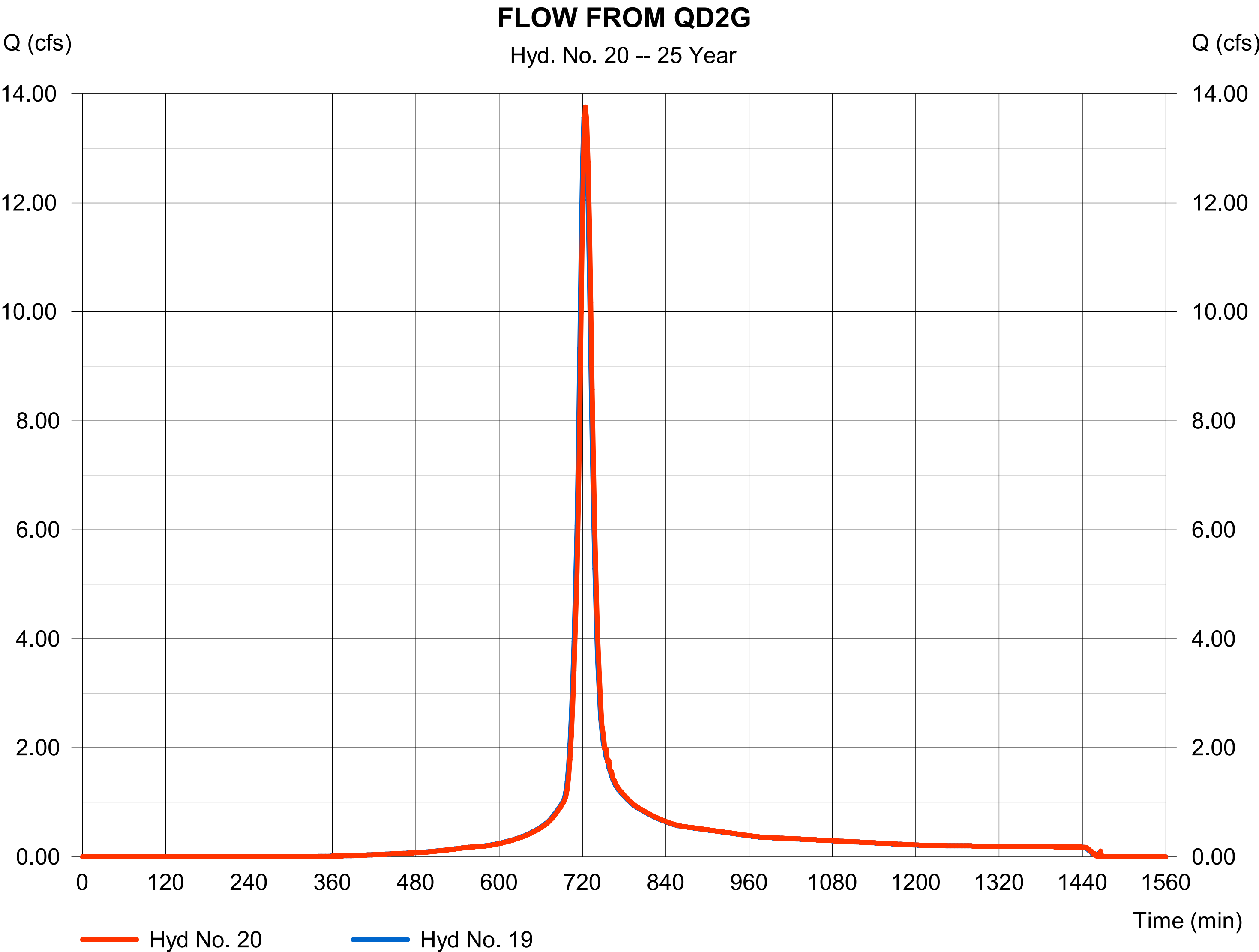
Hydrograph Report

Hyd. No. 20

FLOW FROM QD2G

Hydrograph type	= Reach	Peak discharge	= 13.76 cfs
Storm frequency	= 25 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 41,879 cuft
Inflow hyd. No.	= 19 - FLOW FROM QD2G	Section type	= Circular
Reach length	= 176.6 ft	Channel slope	= 1.0 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 3.584	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.5317

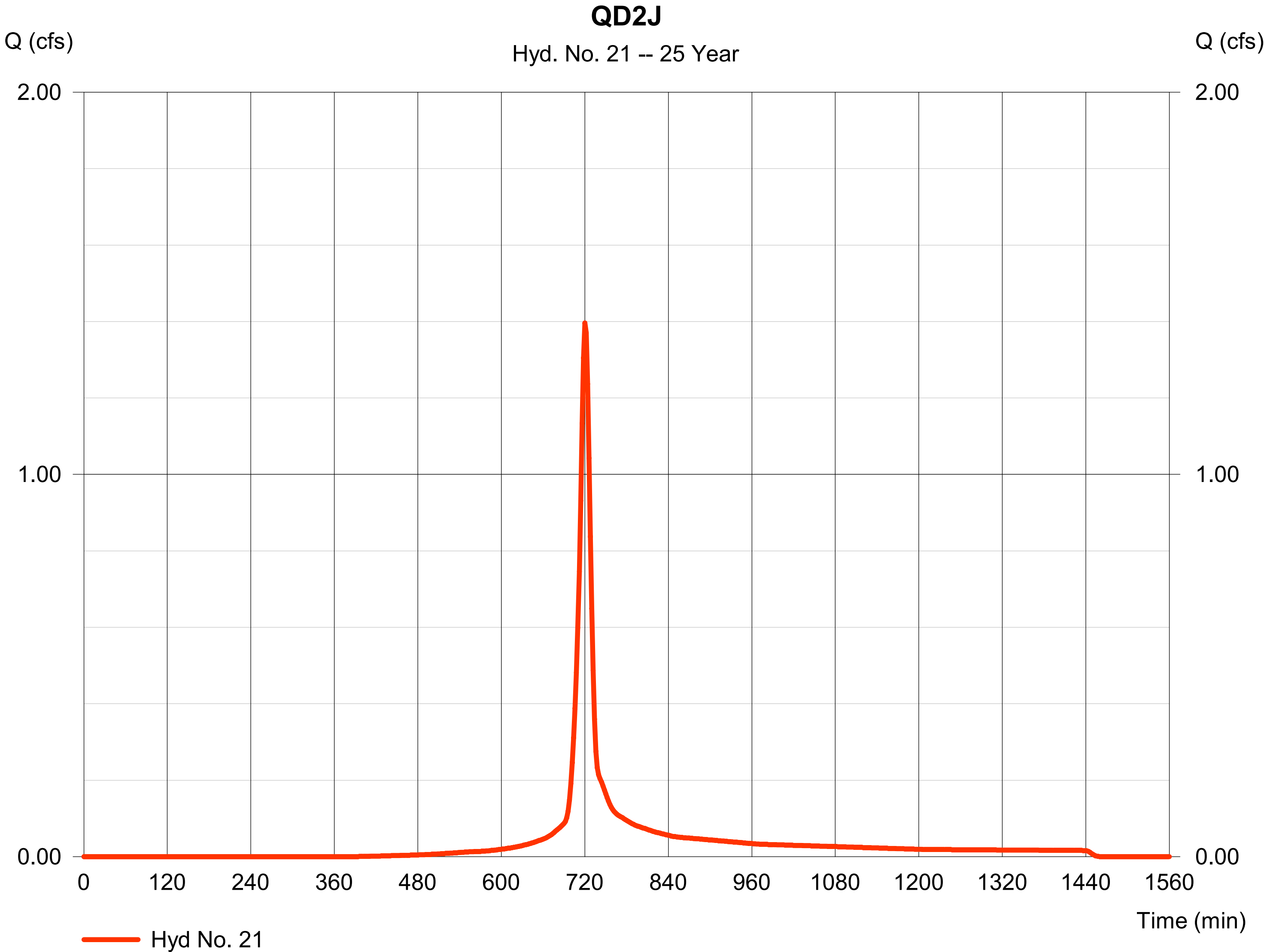
Modified Att-Kin routing method used.



Hyd. No. 21

QD2J

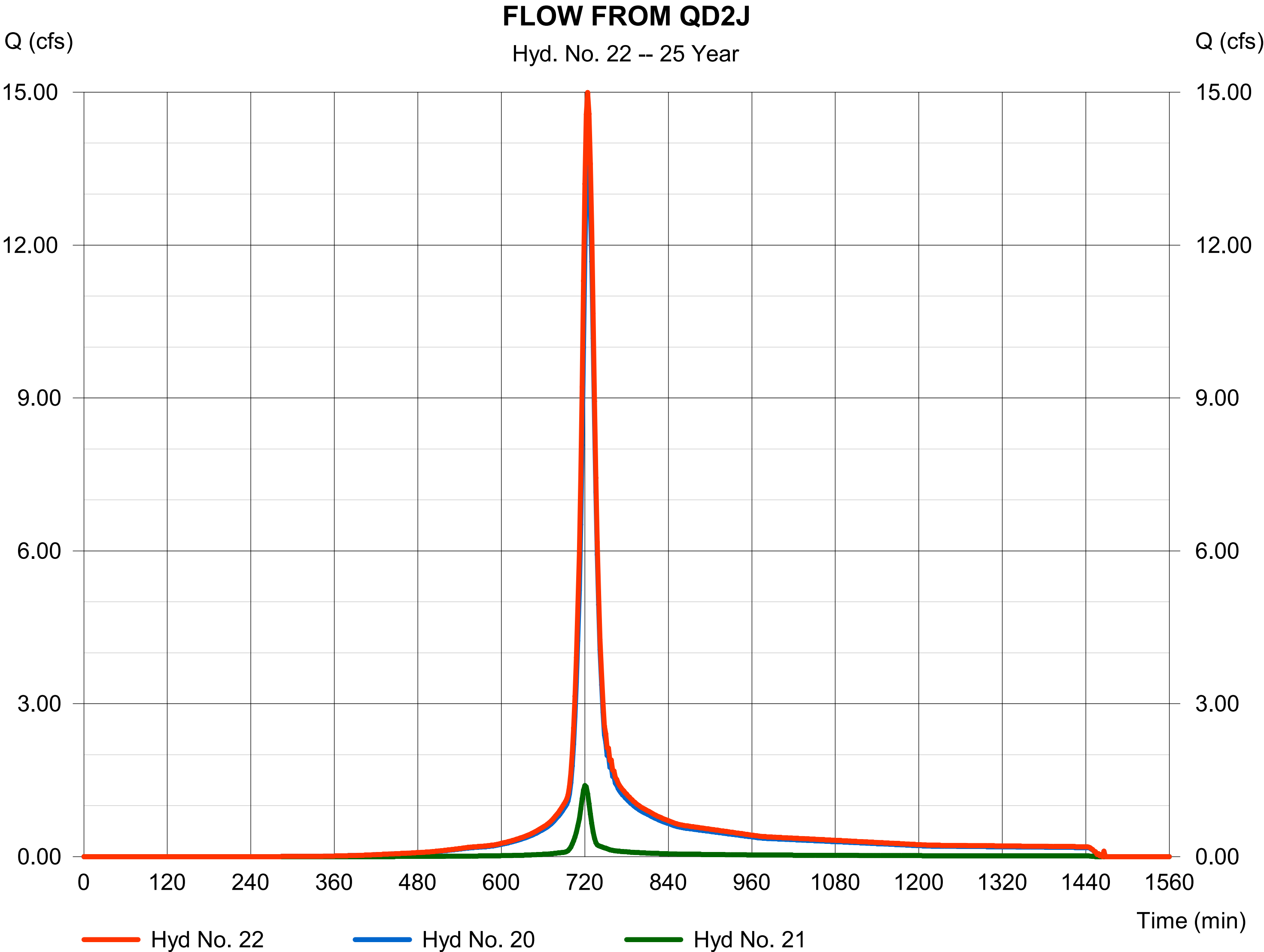
Hydrograph type	= SCS Runoff	Peak discharge	= 1.396 cfs
Storm frequency	= 25 yrs	Time to peak	= 720 min
Time interval	= 2 min	Hyd. volume	= 3,640 cuft
Drainage area	= 0.600 ac	Curve number	= 90
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 12.20 min
Total precip.	= 2.60 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hyd. No. 22

FLOW FROM QD2J

Hydrograph type	= Combine	Peak discharge	= 14.99 cfs
Storm frequency	= 25 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 45,519 cuft
Inflow hyds.	= 20, 21	Contrib. drain. area	= 0.600 ac

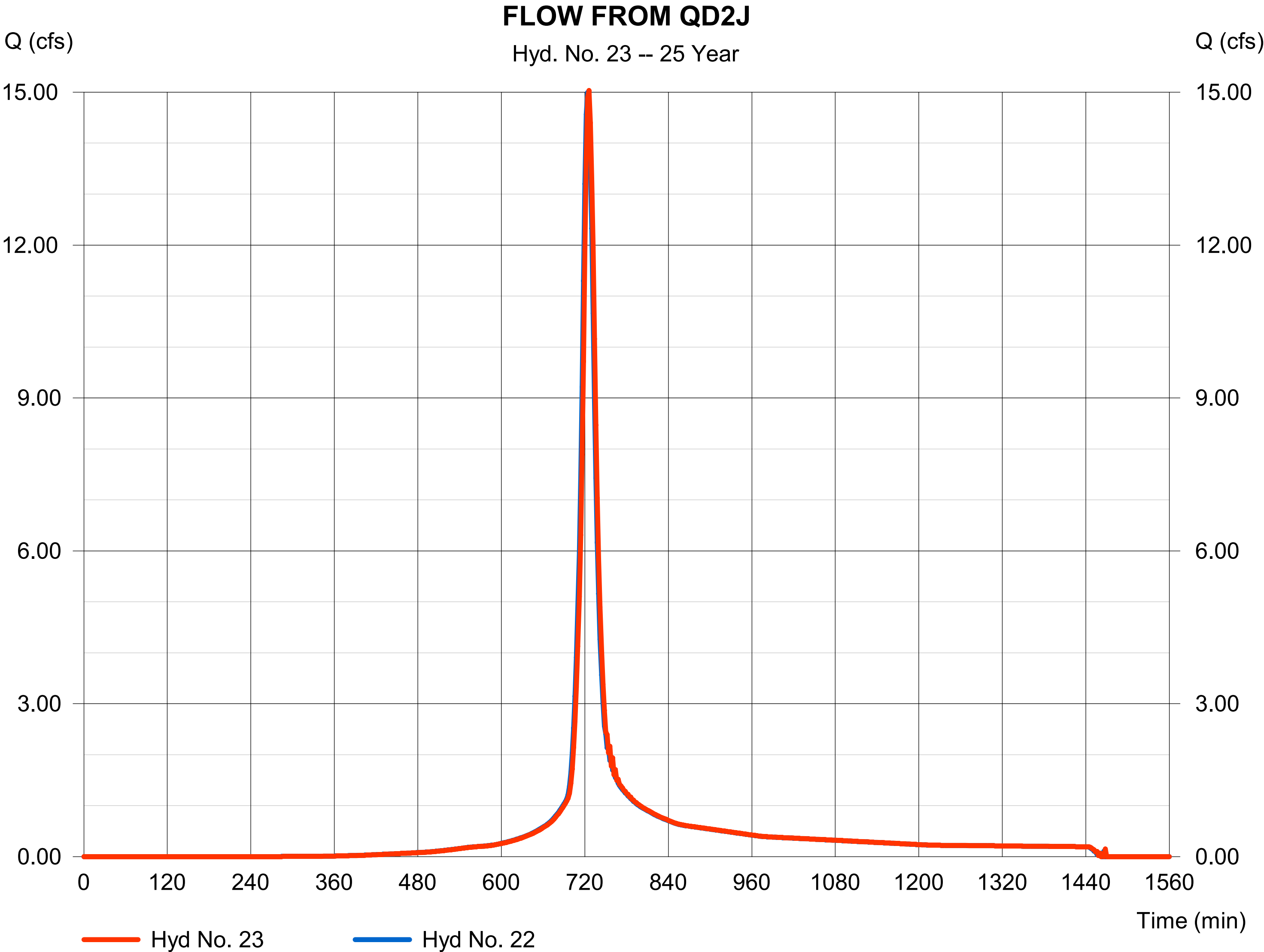


Hyd. No. 23

FLOW FROM QD2J

Hydrograph type	= Reach	Peak discharge	= 15.03 cfs
Storm frequency	= 25 yrs	Time to peak	= 726 min
Time interval	= 2 min	Hyd. volume	= 45,526 cuft
Inflow hyd. No.	= 22 - FLOW FROM QD2J	Section type	= Circular
Reach length	= 158.4 ft	Channel slope	= 0.5 %
Manning's n	= 0.024	Bottom width	= 2.0 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.659	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.3864

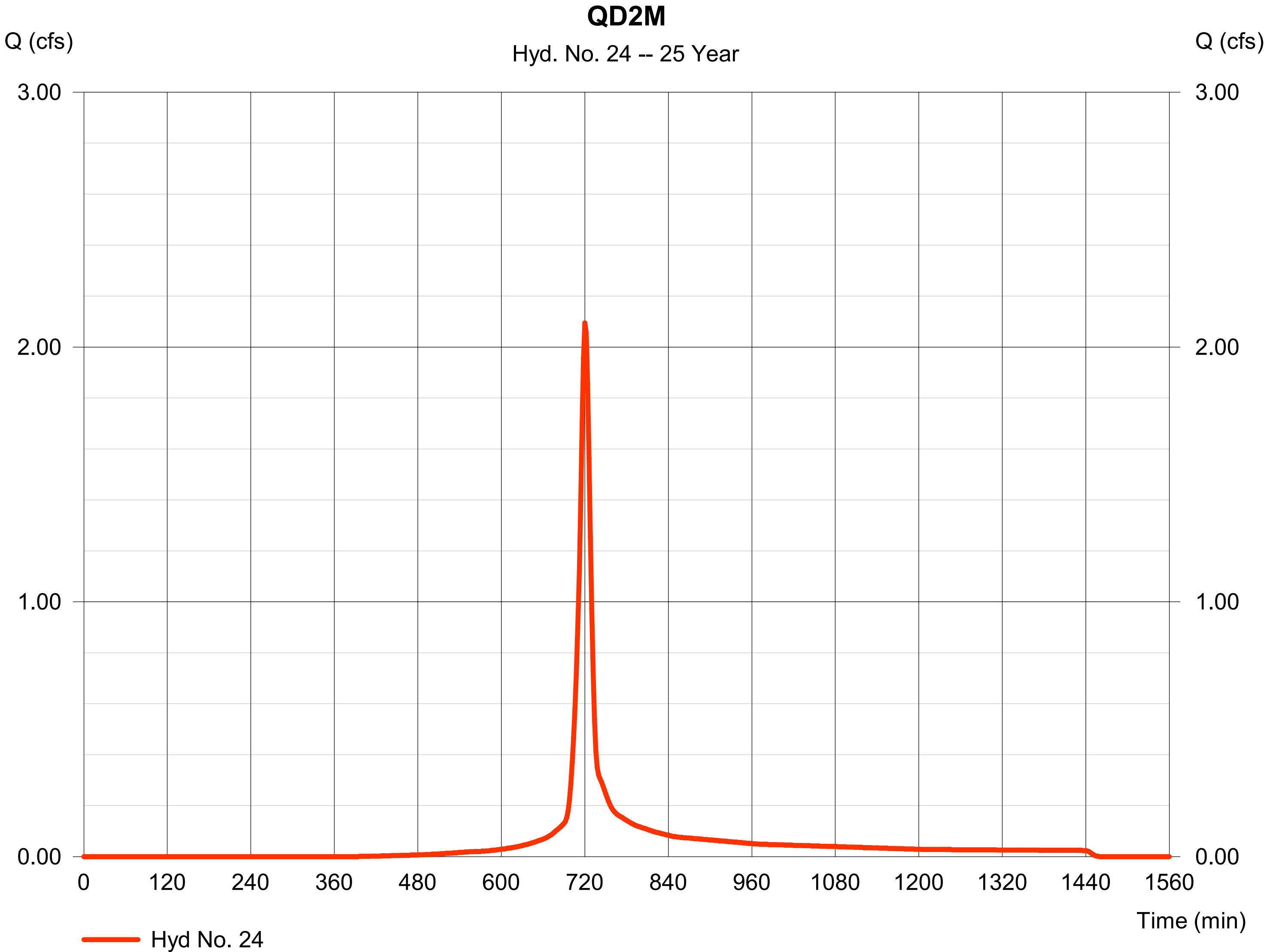
Modified Att-Kin routing method used.



Hyd. No. 24

QD2M

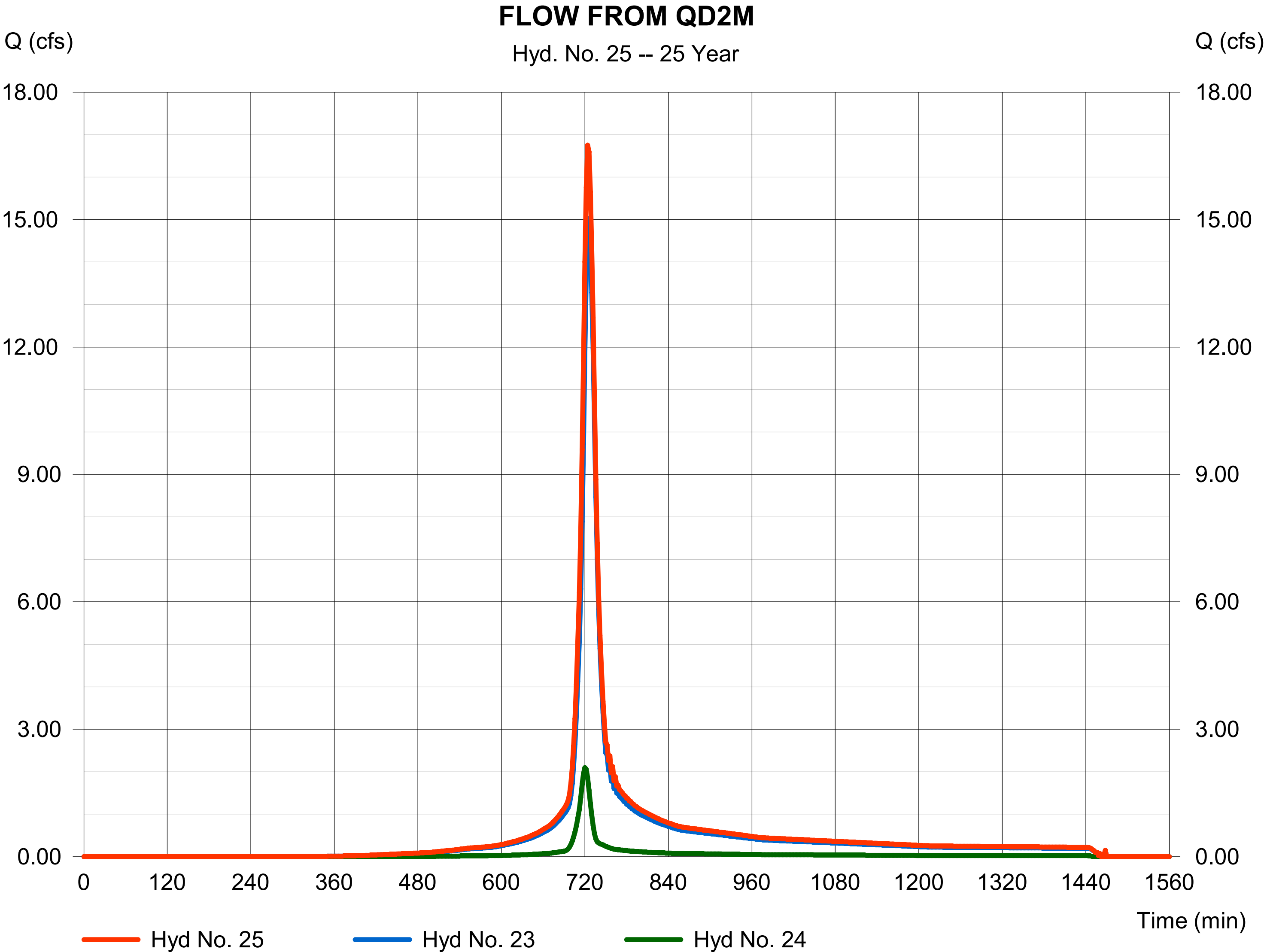
Hydrograph type	=	SCS Runoff	Peak discharge	=	2.094 cfs
Storm frequency	=	25 yrs	Time to peak	=	720 min
Time interval	=	2 min	Hyd. volume	=	5,460 cuft
Drainage area	=	0.900 ac	Curve number	=	90
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	11.90 min
Total precip.	=	2.60 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hyd. No. 25

FLOW FROM QD2M

Hydrograph type	= Combine	Peak discharge	= 16.75 cfs
Storm frequency	= 25 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 50,986 cuft
Inflow hyds.	= 23, 24	Contrib. drain. area	= 0.900 ac

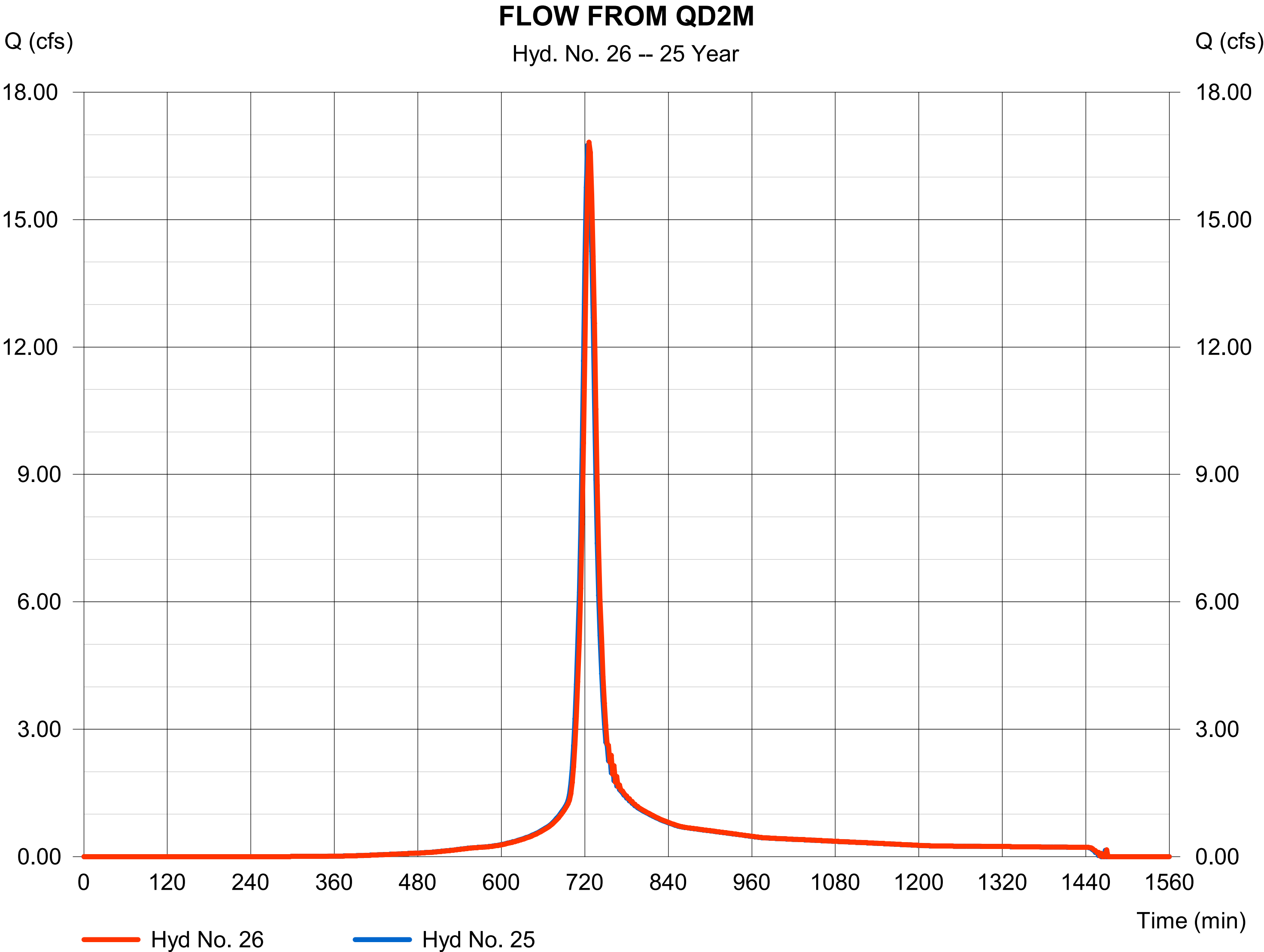


Hyd. No. 26

FLOW FROM QD2M

Hydrograph type	= Reach	Peak discharge	= 16.82 cfs
Storm frequency	= 25 yrs	Time to peak	= 726 min
Time interval	= 2 min	Hyd. volume	= 50,989 cuft
Inflow hyd. No.	= 25 - FLOW FROM QD2M	Section type	= Circular
Reach length	= 336.3 ft	Channel slope	= 0.3 %
Manning's n	= 0.024	Bottom width	= 2.0 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.060	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.0863

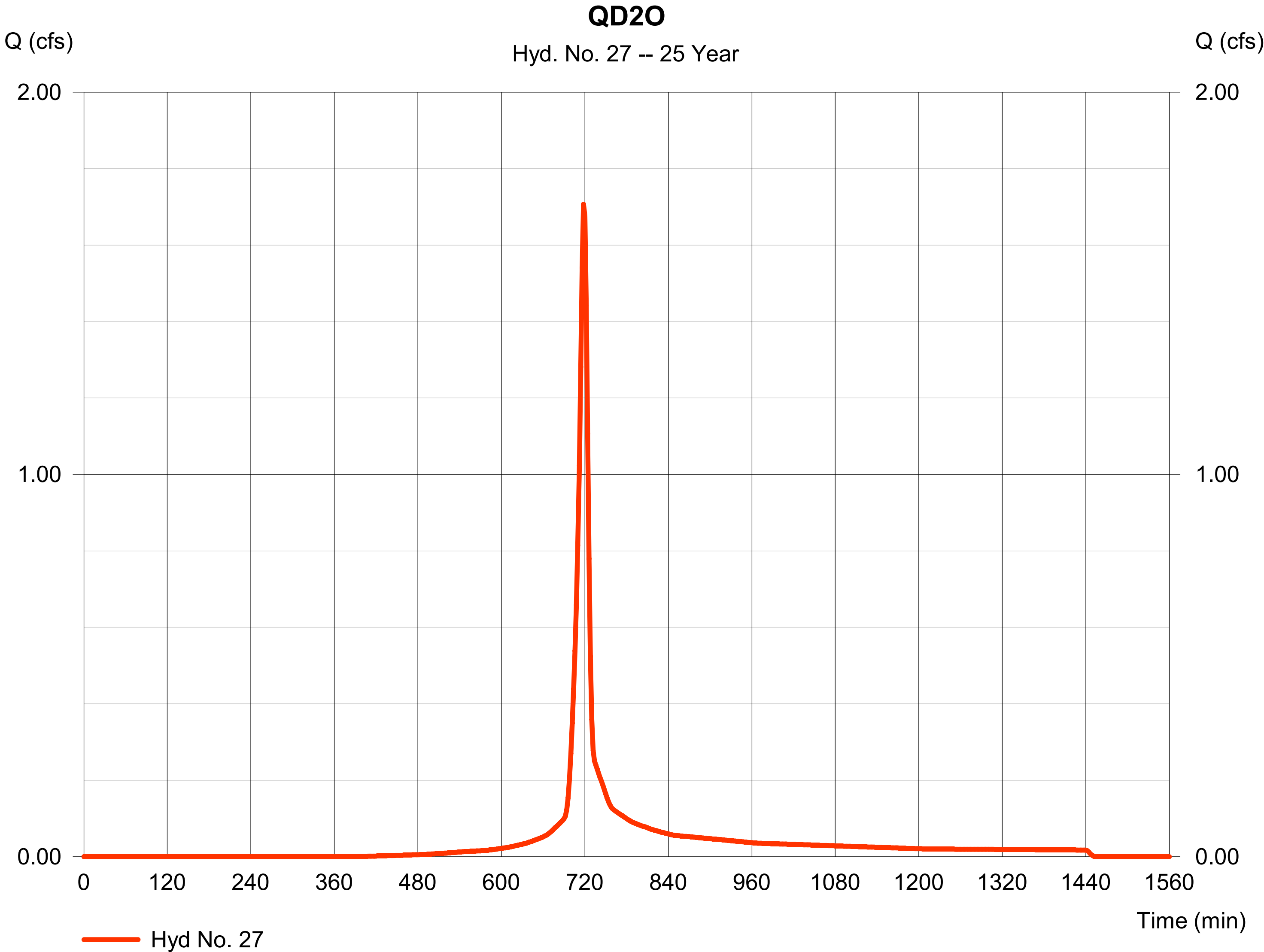
Modified Att-Kin routing method used.



Hyd. No. 27

QD2O

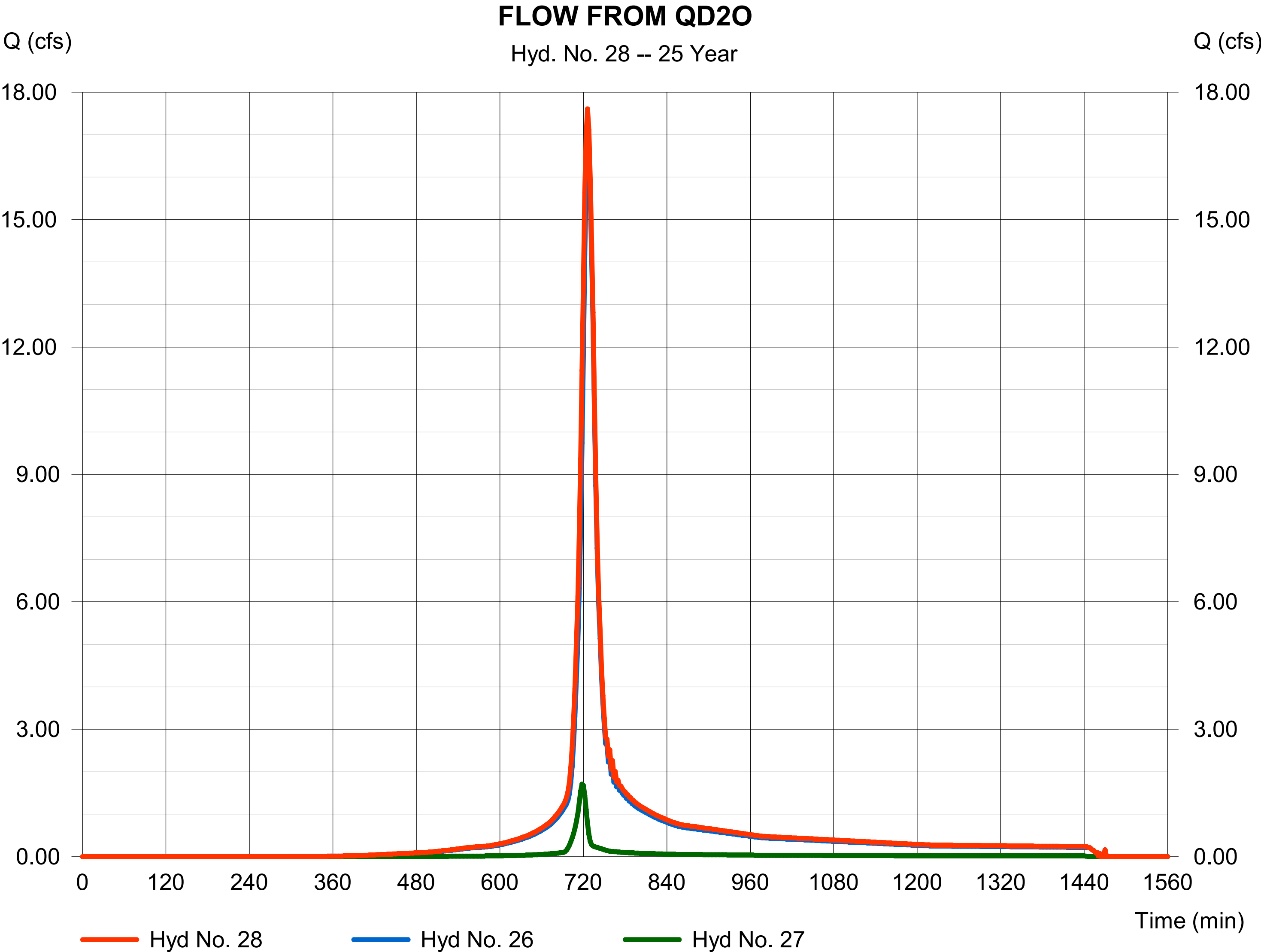
Hydrograph type	= SCS Runoff	Peak discharge	= 1.706 cfs
Storm frequency	= 25 yrs	Time to peak	= 718 min
Time interval	= 2 min	Hyd. volume	= 3,941 cuft
Drainage area	= 0.670 ac	Curve number	= 90
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 7.40 min
Total precip.	= 2.60 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hyd. No. 28

FLOW FROM QD20

Hydrograph type	= Combine	Peak discharge	= 17.60 cfs
Storm frequency	= 25 yrs	Time to peak	= 726 min
Time interval	= 2 min	Hyd. volume	= 54,930 cuft
Inflow hyds.	= 26, 27	Contrib. drain. area	= 0.670 ac

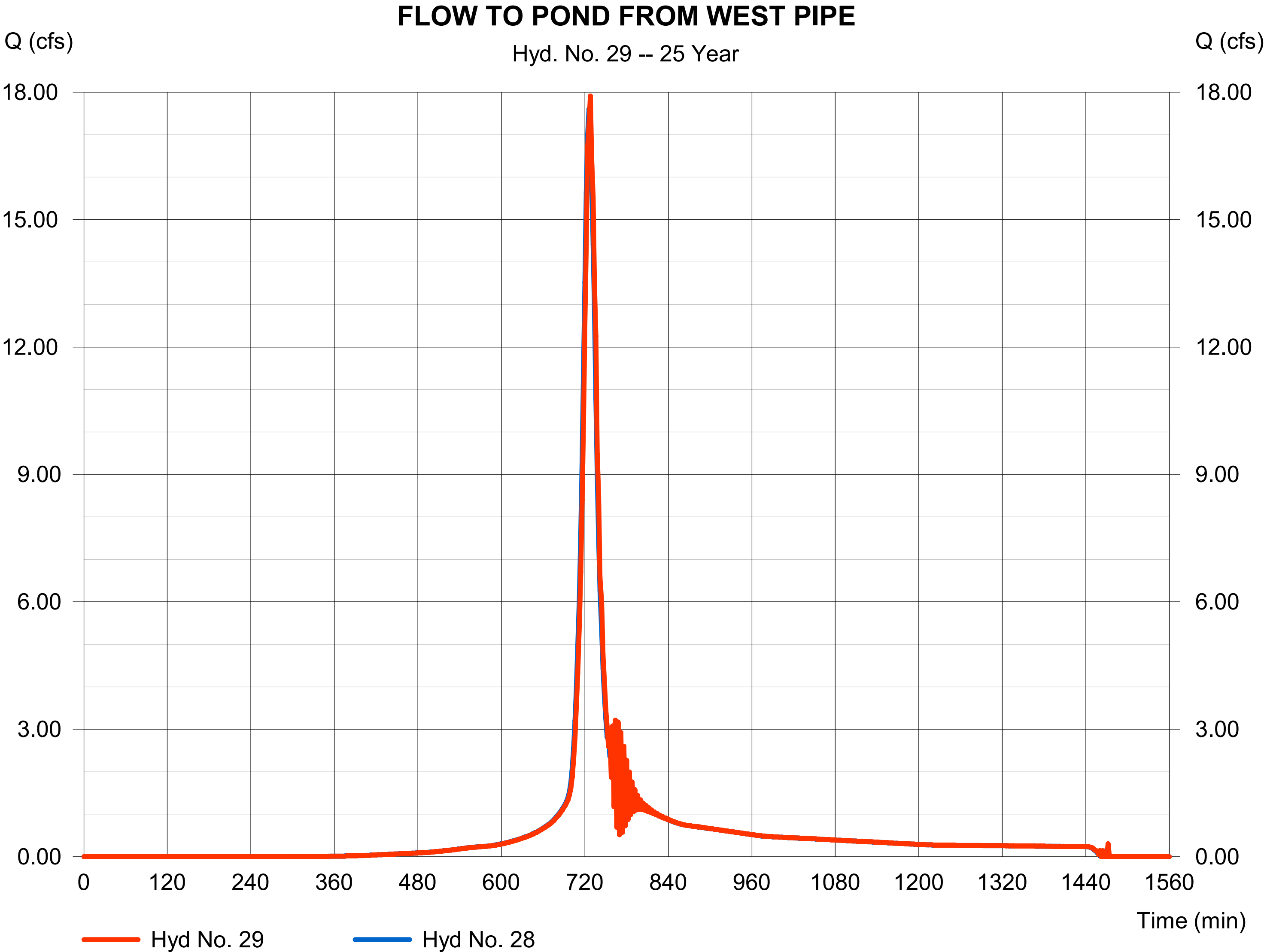


Hyd. No. 29

FLOW TO POND FROM WEST PIPE

Hydrograph type	= Reach	Peak discharge	= 17.91 cfs
Storm frequency	= 25 yrs	Time to peak	= 728 min
Time interval	= 2 min	Hyd. volume	= 54,968 cuft
Inflow hyd. No.	= 28 - FLOW FROM QD20	Section type	= Circular
Reach length	= 27.1 ft	Channel slope	= 0.3 %
Manning's n	= 0.024	Bottom width	= 2.0 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.060	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.8786

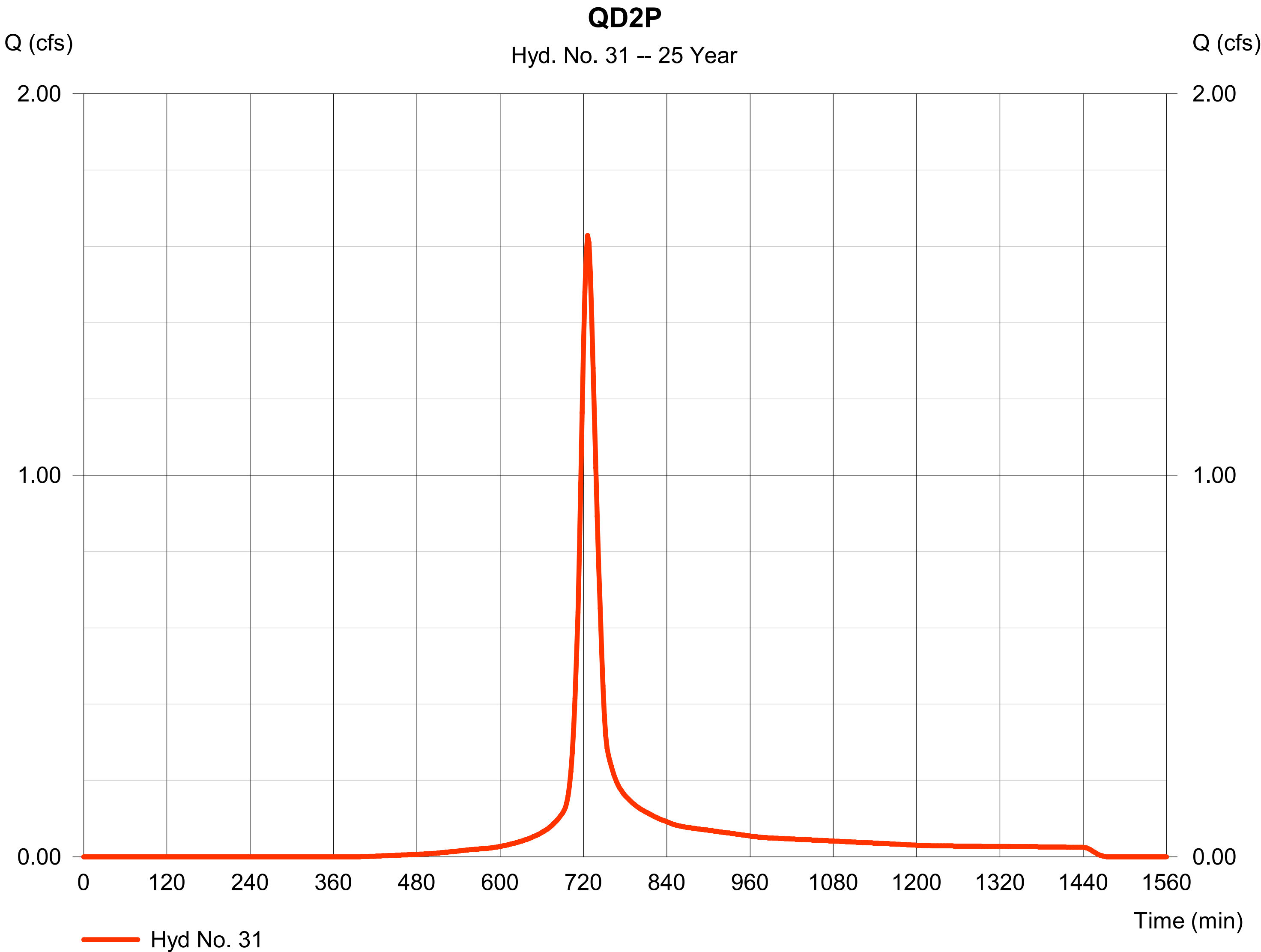
Modified Att-Kin routing method used.



Hyd. No. 31

QD2P

Hydrograph type	=	SCS Runoff	Peak discharge	=	1.628 cfs
Storm frequency	=	25 yrs	Time to peak	=	726 min
Time interval	=	2 min	Hyd. volume	=	5,628 cuft
Drainage area	=	0.940 ac	Curve number	=	90
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	22.40 min
Total precip.	=	2.60 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484

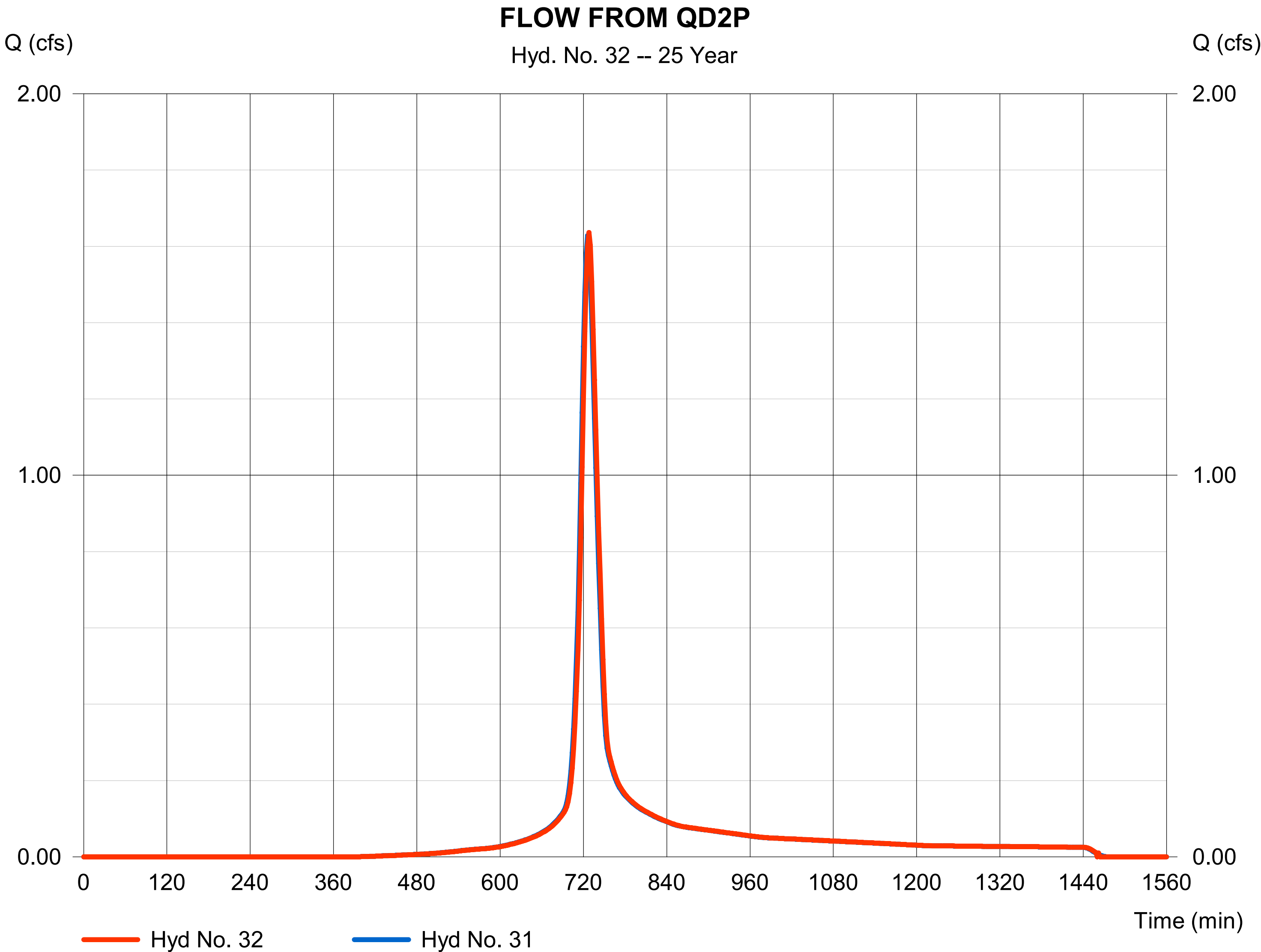


Hyd. No. 32

FLOW FROM QD2P

Hydrograph type	= Reach	Peak discharge	= 1.636 cfs
Storm frequency	= 25 yrs	Time to peak	= 728 min
Time interval	= 2 min	Hyd. volume	= 5,626 cuft
Inflow hyd. No.	= 31 - QD2P	Section type	= Circular
Reach length	= 36.3 ft	Channel slope	= 1.0 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 3.584	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.3118

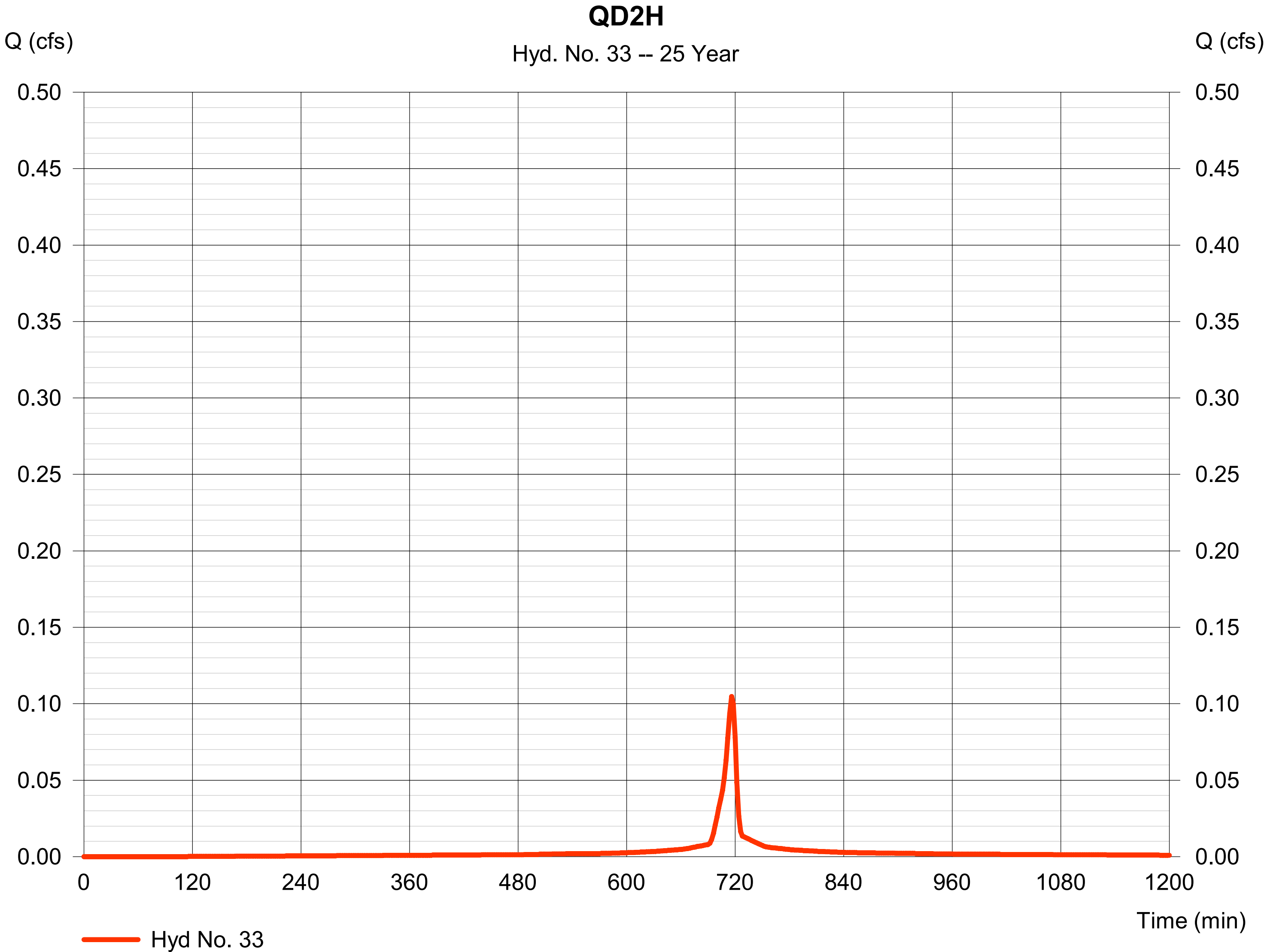
Modified Att-Kin routing method used.



Hyd. No. 33

QD2H

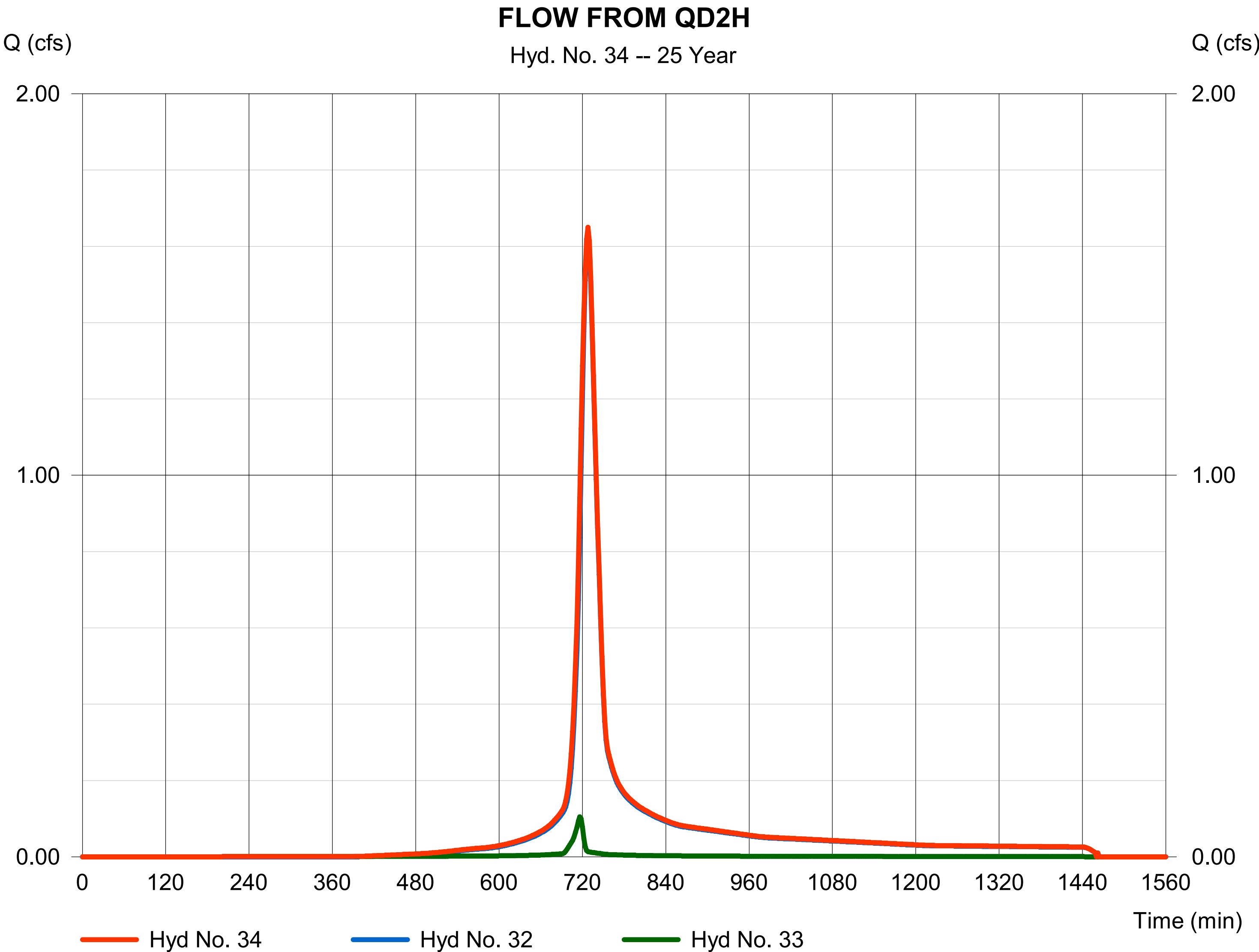
Hydrograph type	= SCS Runoff	Peak discharge	= 0.105 cfs
Storm frequency	= 25 yrs	Time to peak	= 716 min
Time interval	= 2 min	Hyd. volume	= 242 cuft
Drainage area	= 0.030 ac	Curve number	= 98
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 6.00 min
Total precip.	= 2.60 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hyd. No. 34

FLOW FROM QD2H

Hydrograph type	= Combine	Peak discharge	= 1.650 cfs
Storm frequency	= 25 yrs	Time to peak	= 728 min
Time interval	= 2 min	Hyd. volume	= 5,868 cuft
Inflow hyds.	= 32, 33	Contrib. drain. area	= 0.030 ac

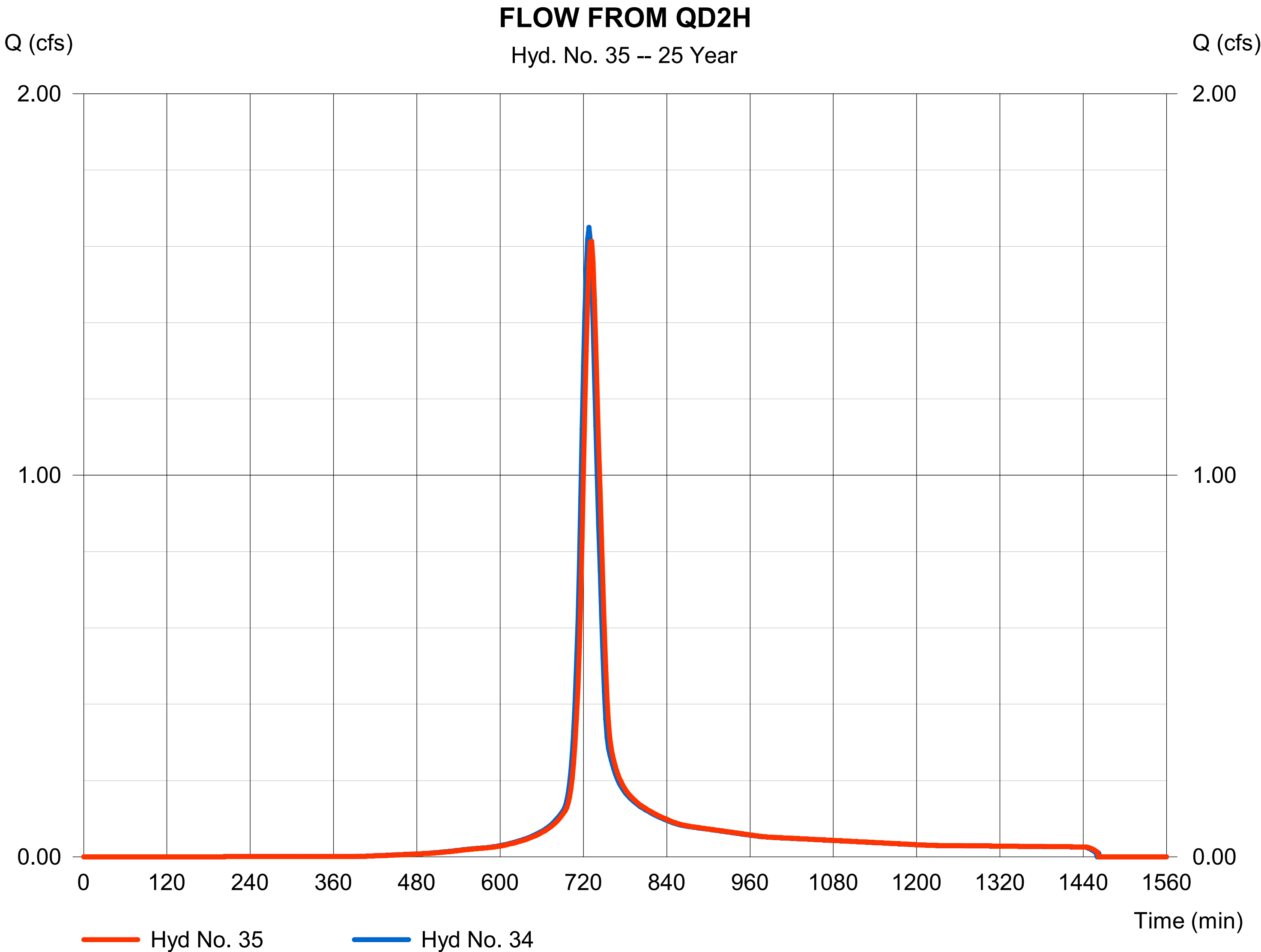


Hyd. No. 35

FLOW FROM QD2H

Hydrograph type	= Reach	Peak discharge	= 1.613 cfs
Storm frequency	= 25 yrs	Time to peak	= 732 min
Time interval	= 2 min	Hyd. volume	= 5,865 cuft
Inflow hyd. No.	= 34 - FLOW FROM QD2H	Section type	= Circular
Reach length	= 162.5 ft	Channel slope	= 1.0 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 3.584	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 0.6022

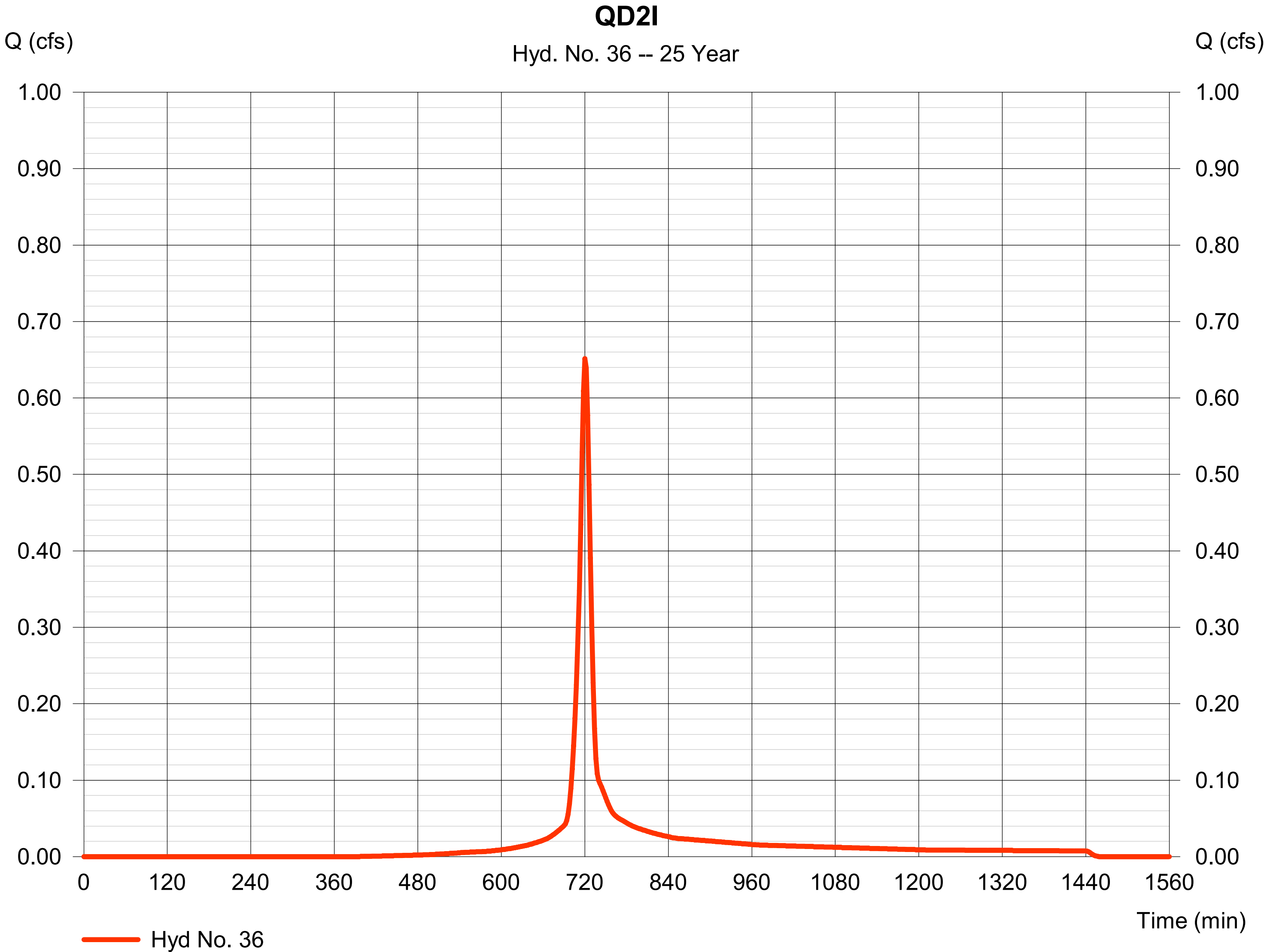
Modified Att-Kin routing method used.



Hyd. No. 36

QD2I

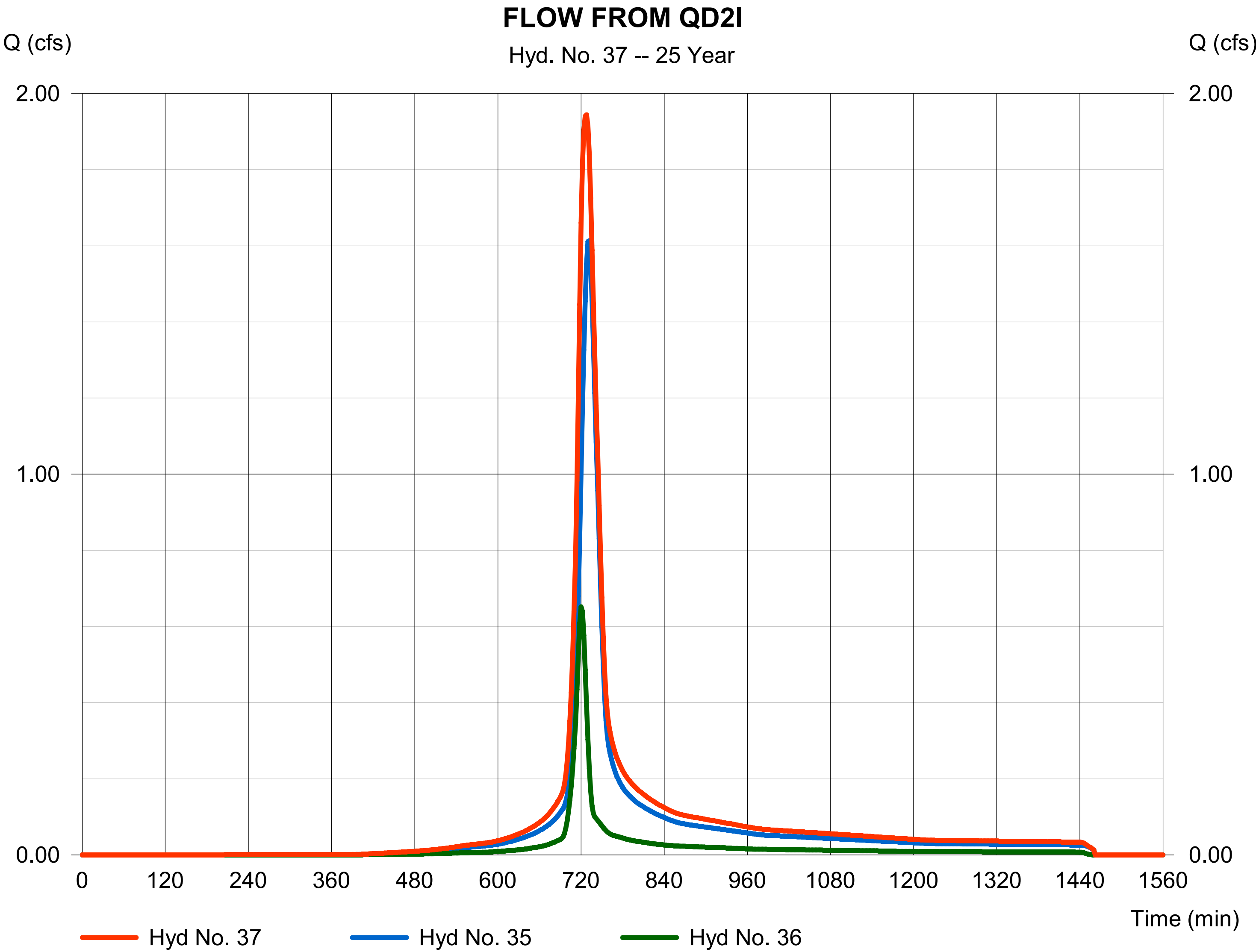
Hydrograph type	= SCS Runoff	Peak discharge	= 0.651 cfs
Storm frequency	= 25 yrs	Time to peak	= 720 min
Time interval	= 2 min	Hyd. volume	= 1,699 cuft
Drainage area	= 0.280 ac	Curve number	= 90
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 12.30 min
Total precip.	= 2.60 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hyd. No. 37

FLOW FROM QD2I

Hydrograph type	= Combine	Peak discharge	= 1.944 cfs
Storm frequency	= 25 yrs	Time to peak	= 728 min
Time interval	= 2 min	Hyd. volume	= 7,564 cuft
Inflow hyds.	= 35, 36	Contrib. drain. area	= 0.280 ac

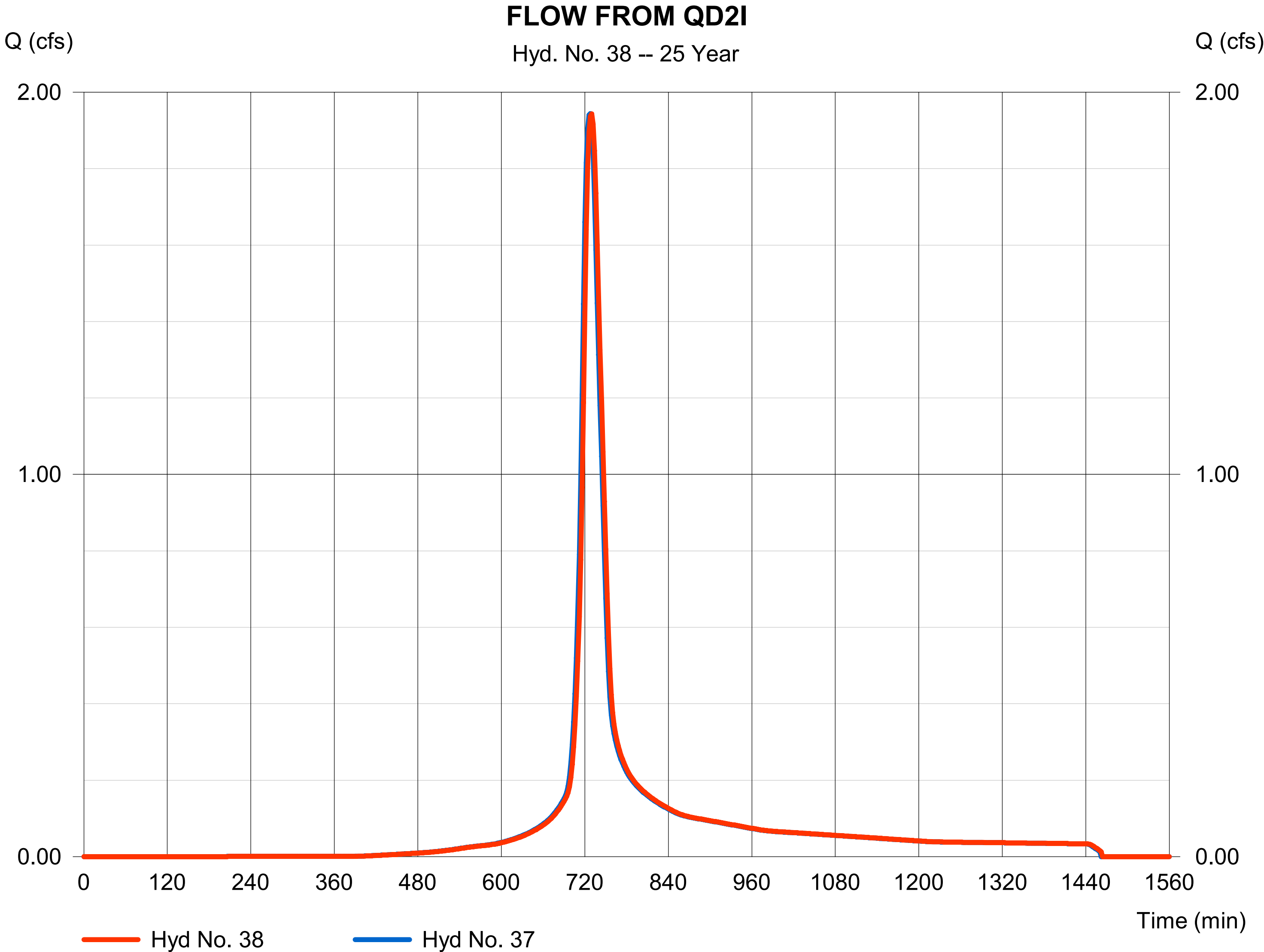


Hyd. No. 38

FLOW FROM QD2I

Hydrograph type	= Reach	Peak discharge	= 1.943 cfs
Storm frequency	= 25 yrs	Time to peak	= 730 min
Time interval	= 2 min	Hyd. volume	= 7,564 cuft
Inflow hyd. No.	= 37 - FLOW FROM QD2I	Section type	= Circular
Reach length	= 97.5 ft	Channel slope	= 0.7 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.934	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 0.9168

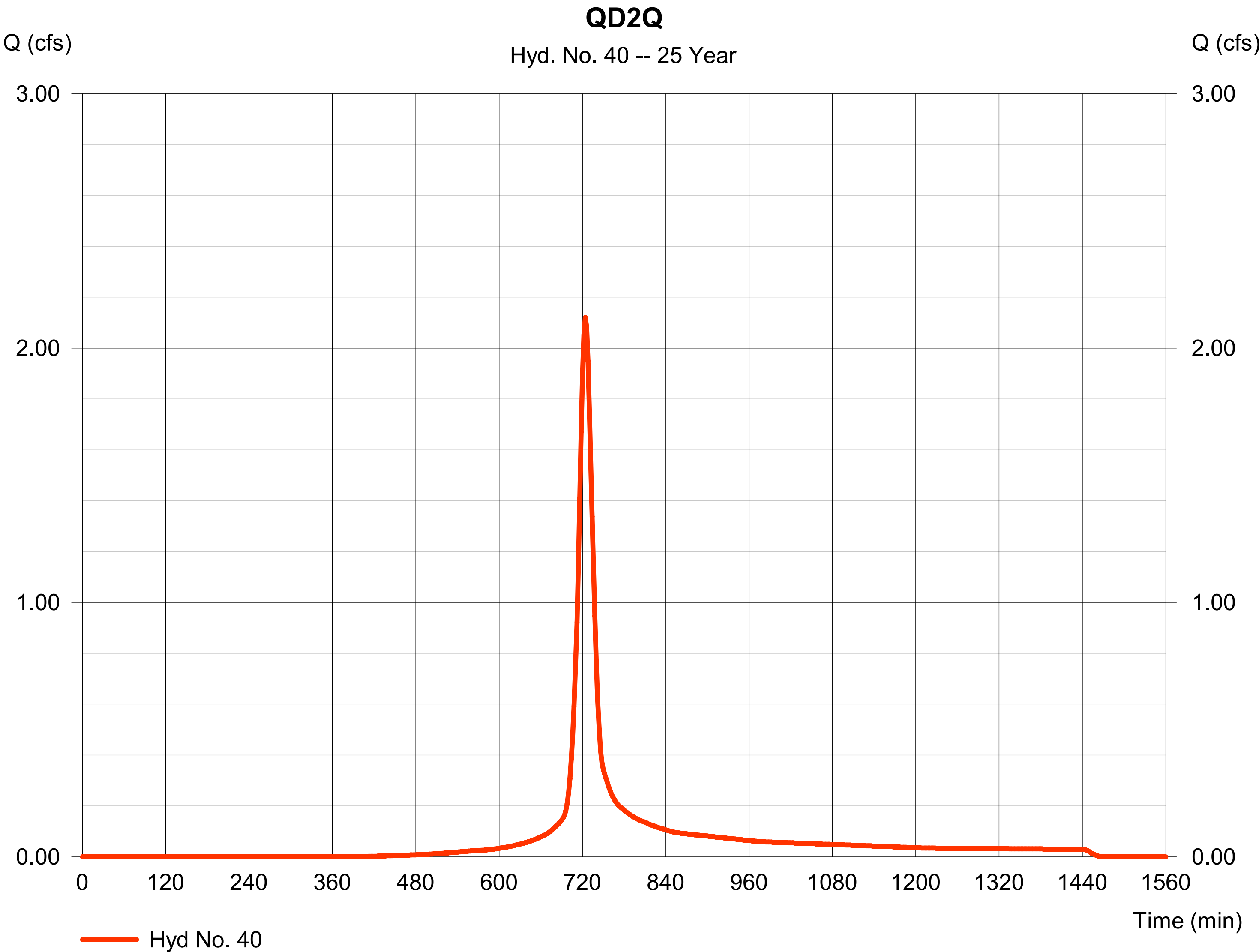
Modified Att-Kin routing method used.



Hyd. No. 40

QD2Q

Hydrograph type	= SCS Runoff	Peak discharge	= 2.121 cfs
Storm frequency	= 25 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 6,647 cuft
Drainage area	= 1.130 ac	Curve number	= 90
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 16.90 min
Total precip.	= 2.60 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

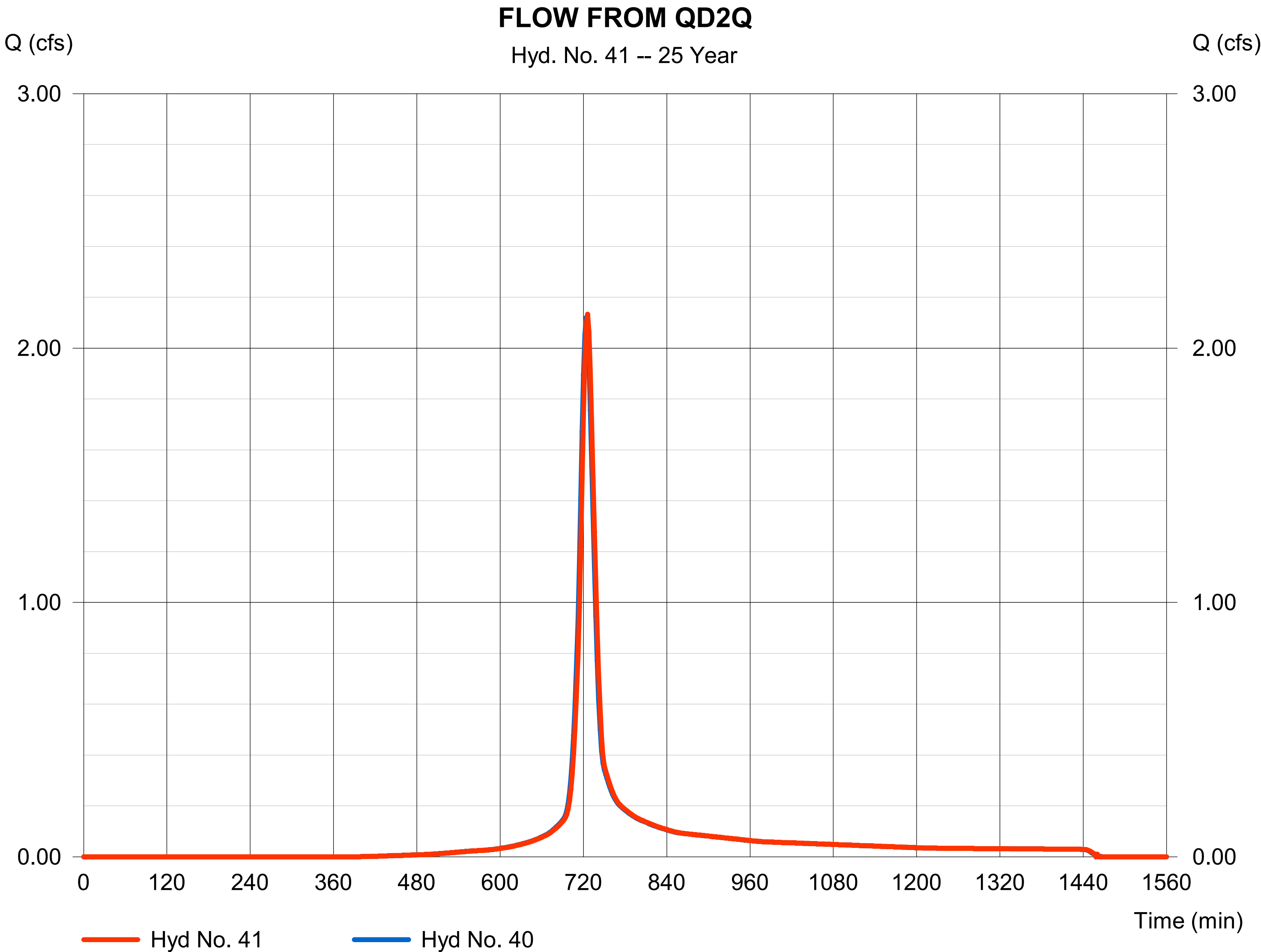


Hyd. No. 41

FLOW FROM QD2Q

Hydrograph type	= Reach	Peak discharge	= 2.133 cfs
Storm frequency	= 25 yrs	Time to peak	= 726 min
Time interval	= 2 min	Hyd. volume	= 6,645 cuft
Inflow hyd. No.	= 40 - QD2Q	Section type	= Circular
Reach length	= 36.9 ft	Channel slope	= 0.7 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.999	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.4180

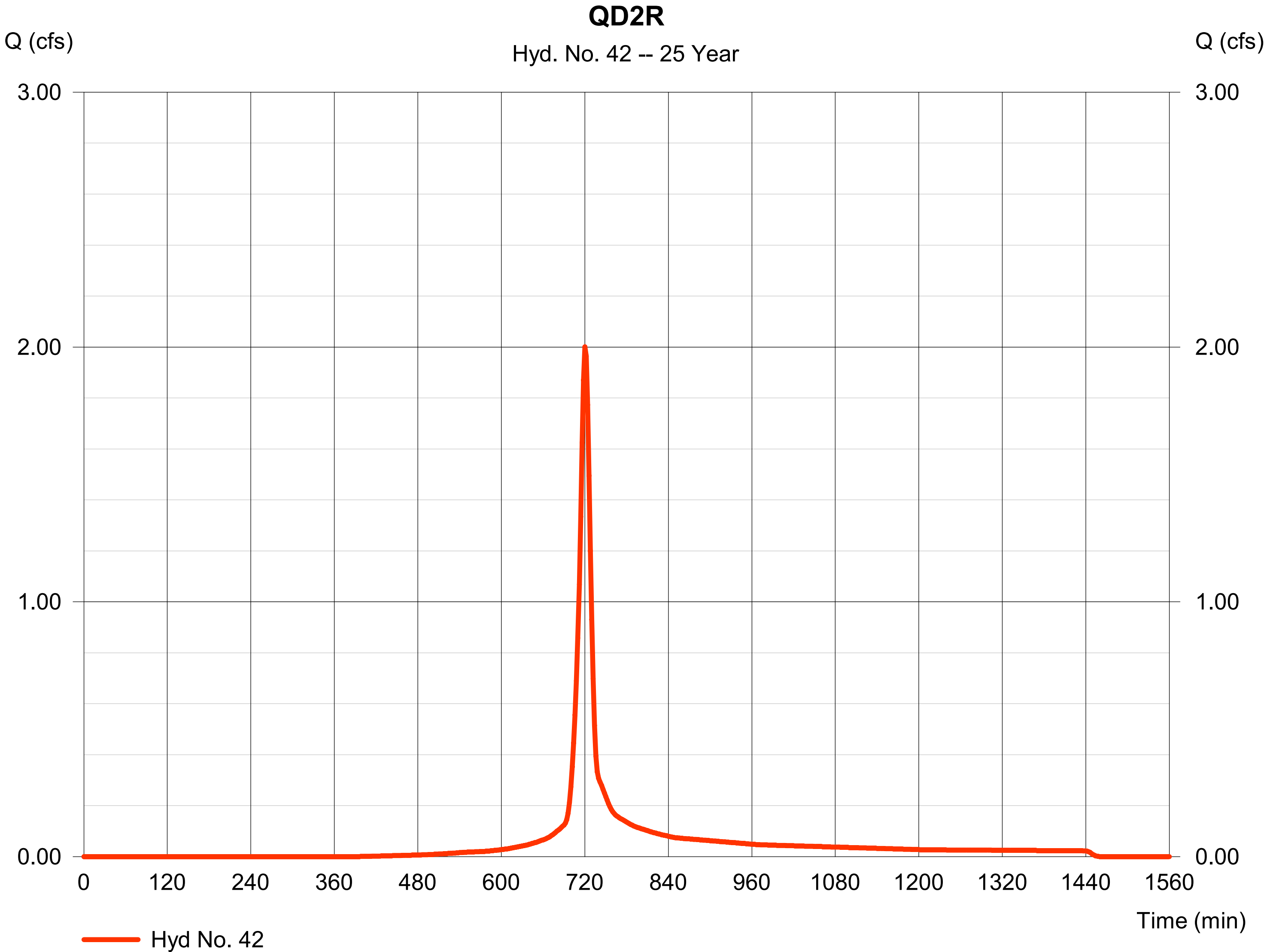
Modified Att-Kin routing method used.



Hyd. No. 42

QD2R

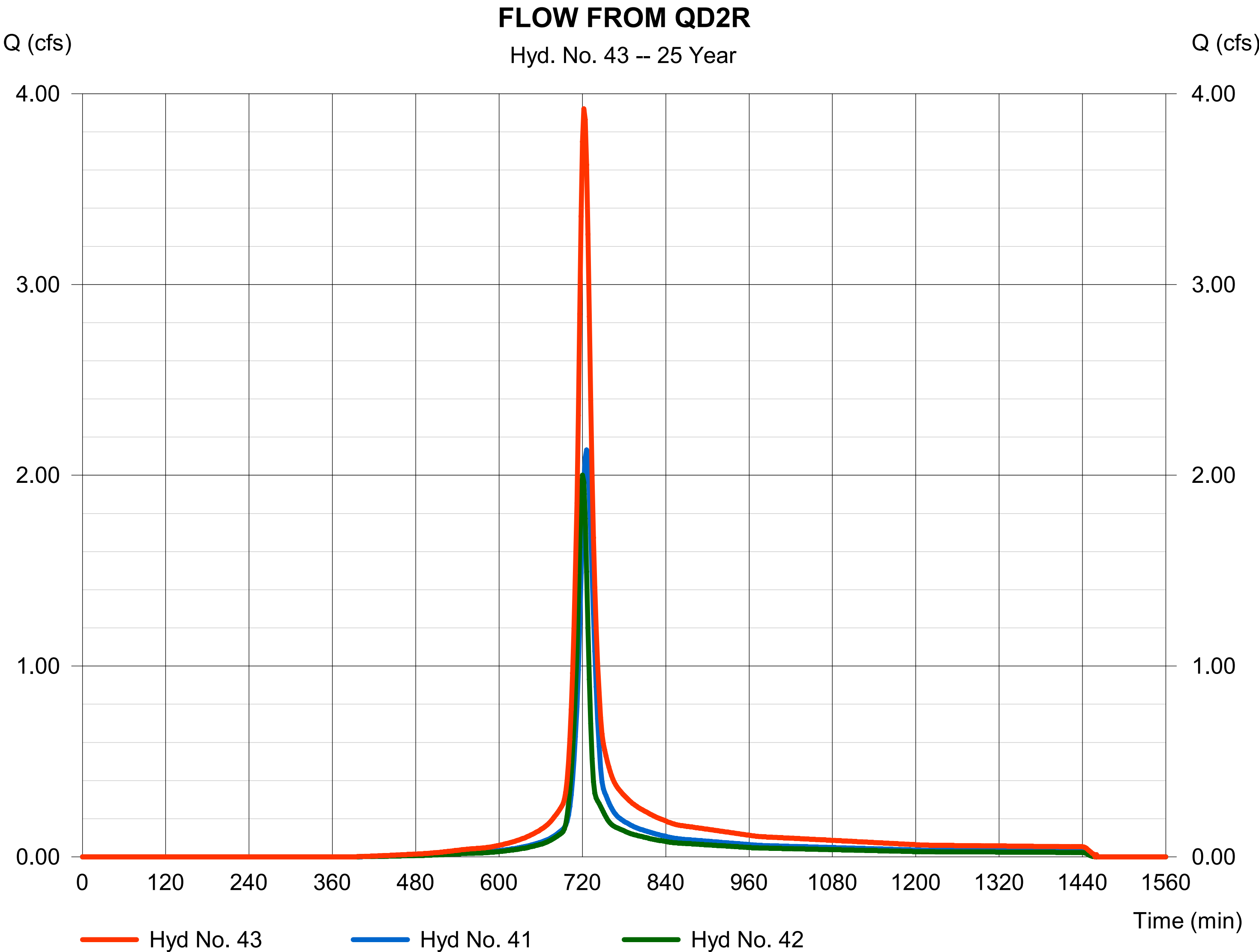
Hydrograph type	= SCS Runoff	Peak discharge	= 2.001 cfs
Storm frequency	= 25 yrs	Time to peak	= 720 min
Time interval	= 2 min	Hyd. volume	= 5,217 cuft
Drainage area	= 0.860 ac	Curve number	= 90
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 13.10 min
Total precip.	= 2.60 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hyd. No. 43

FLOW FROM QD2R

Hydrograph type	= Combine	Peak discharge	= 3.921 cfs
Storm frequency	= 25 yrs	Time to peak	= 722 min
Time interval	= 2 min	Hyd. volume	= 11,862 cuft
Inflow hyds.	= 41, 42	Contrib. drain. area	= 0.860 ac

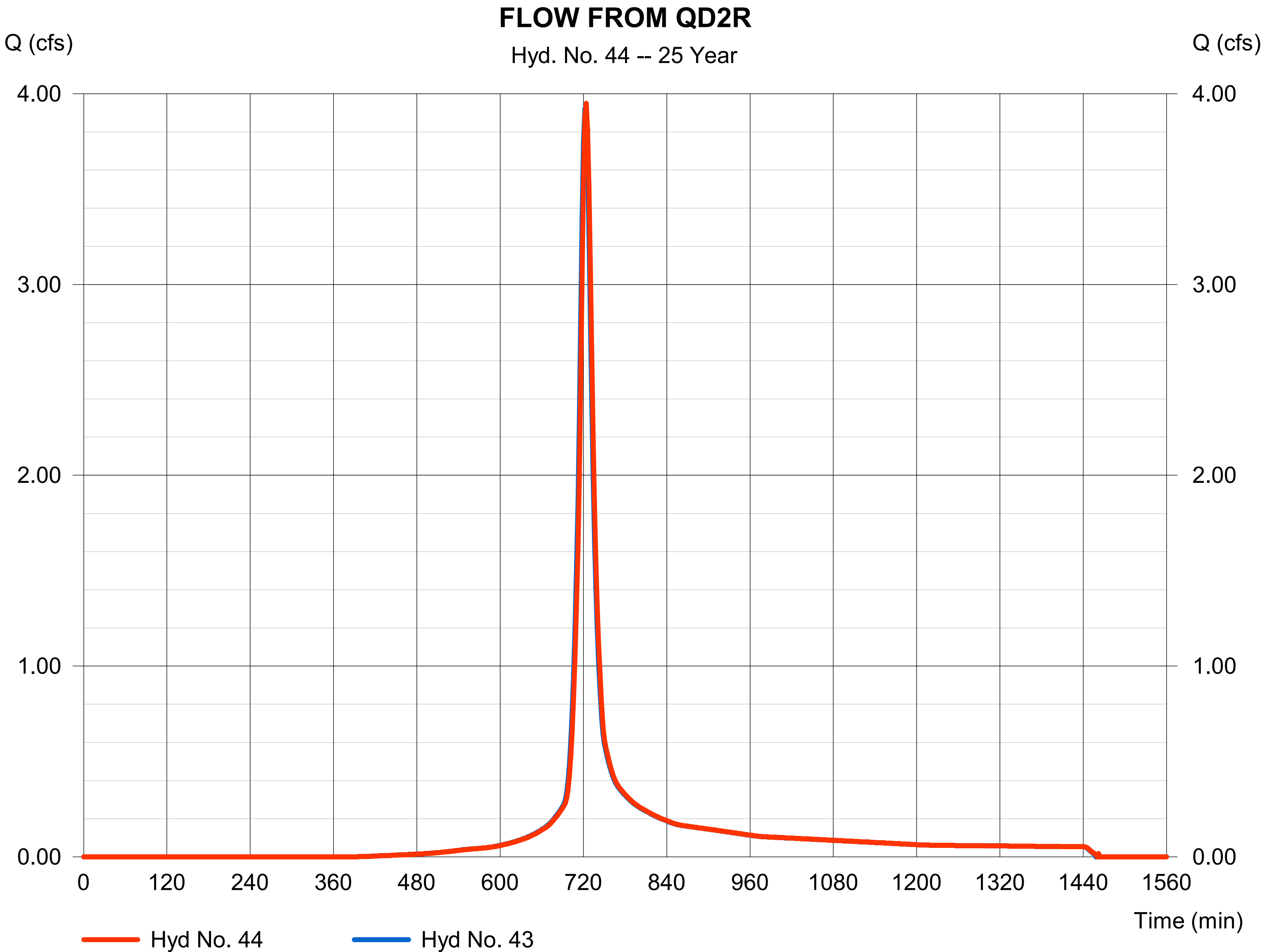


Hyd. No. 44

FLOW FROM QD2R

Hydrograph type	= Reach	Peak discharge	= 3.950 cfs
Storm frequency	= 25 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 11,864 cuft
Inflow hyd. No.	= 43 - FLOW FROM QD2R	Section type	= Circular
Reach length	= 36.3 ft	Channel slope	= 1.3 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 4.102	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.6422

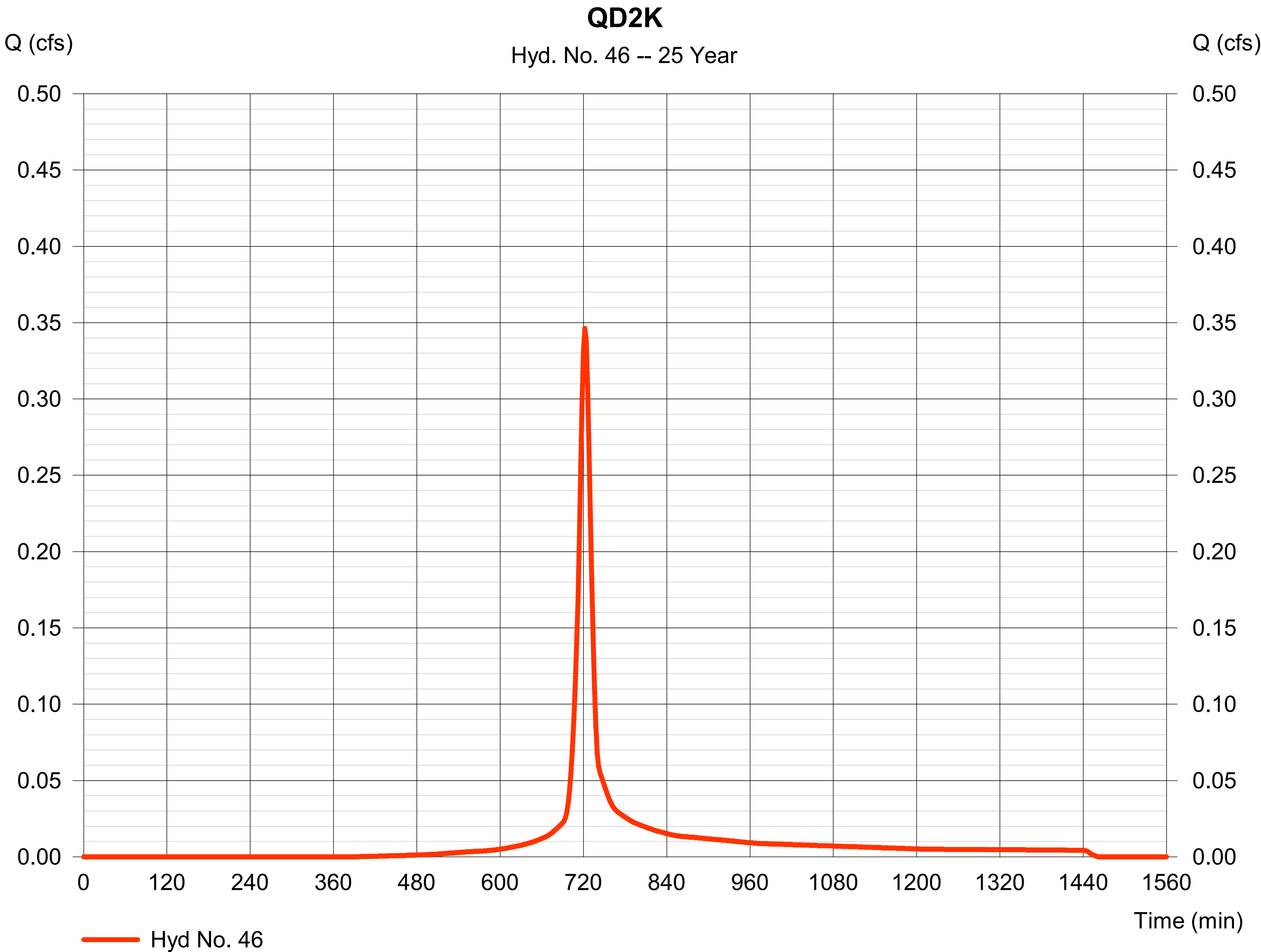
Modified Att-Kin routing method used.



Hyd. No. 46

QD2K

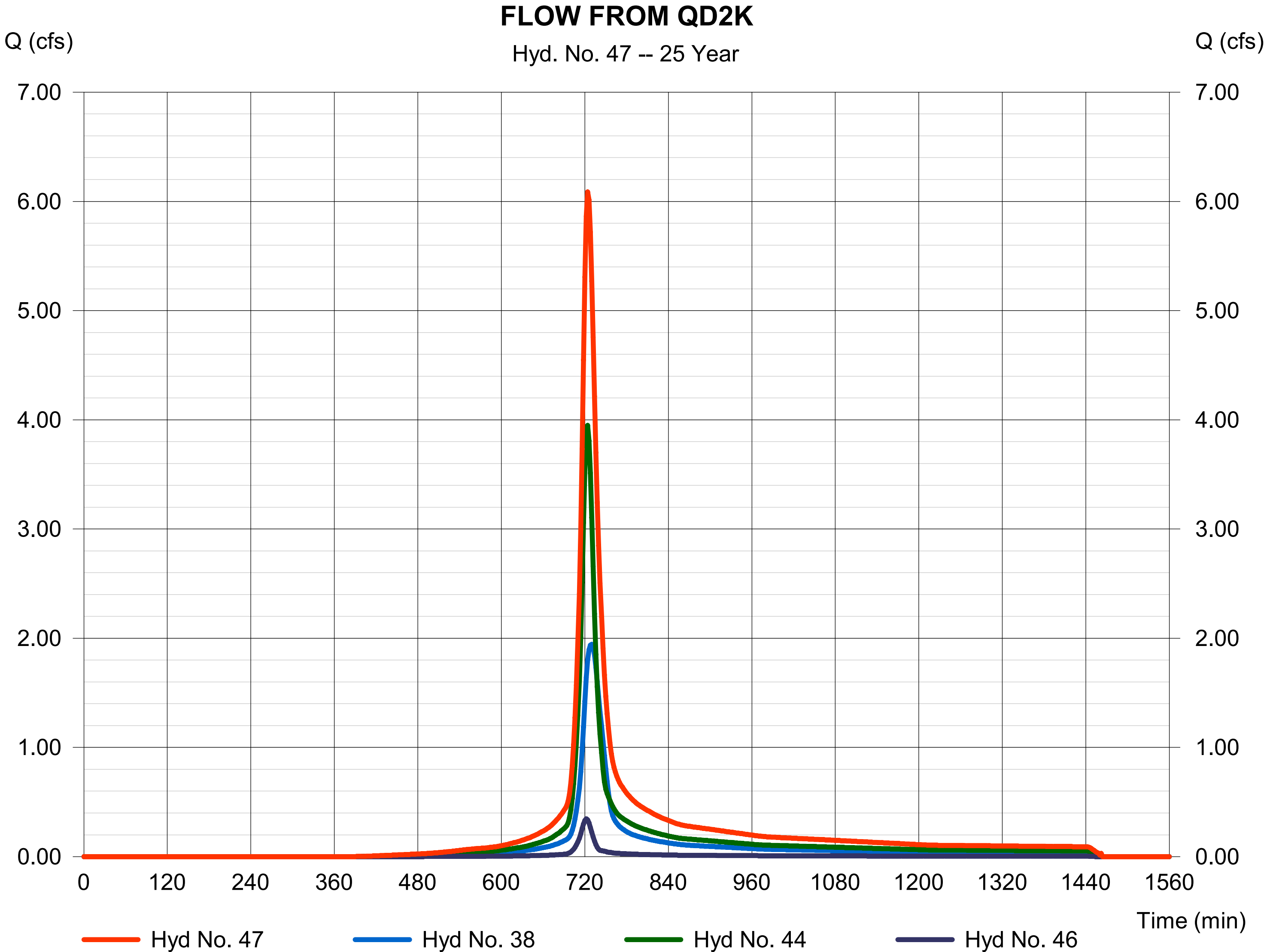
Hydrograph type	= SCS Runoff	Peak discharge	= 0.346 cfs
Storm frequency	= 25 yrs	Time to peak	= 722 min
Time interval	= 2 min	Hyd. volume	= 975 cuft
Drainage area	= 0.170 ac	Curve number	= 90
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 13.80 min
Total precip.	= 2.60 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hyd. No. 47

FLOW FROM QD2K

Hydrograph type	= Combine	Peak discharge	= 6.088 cfs
Storm frequency	= 25 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 20,402 cuft
Inflow hyds.	= 38, 44, 46	Contrib. drain. area	= 0.170 ac

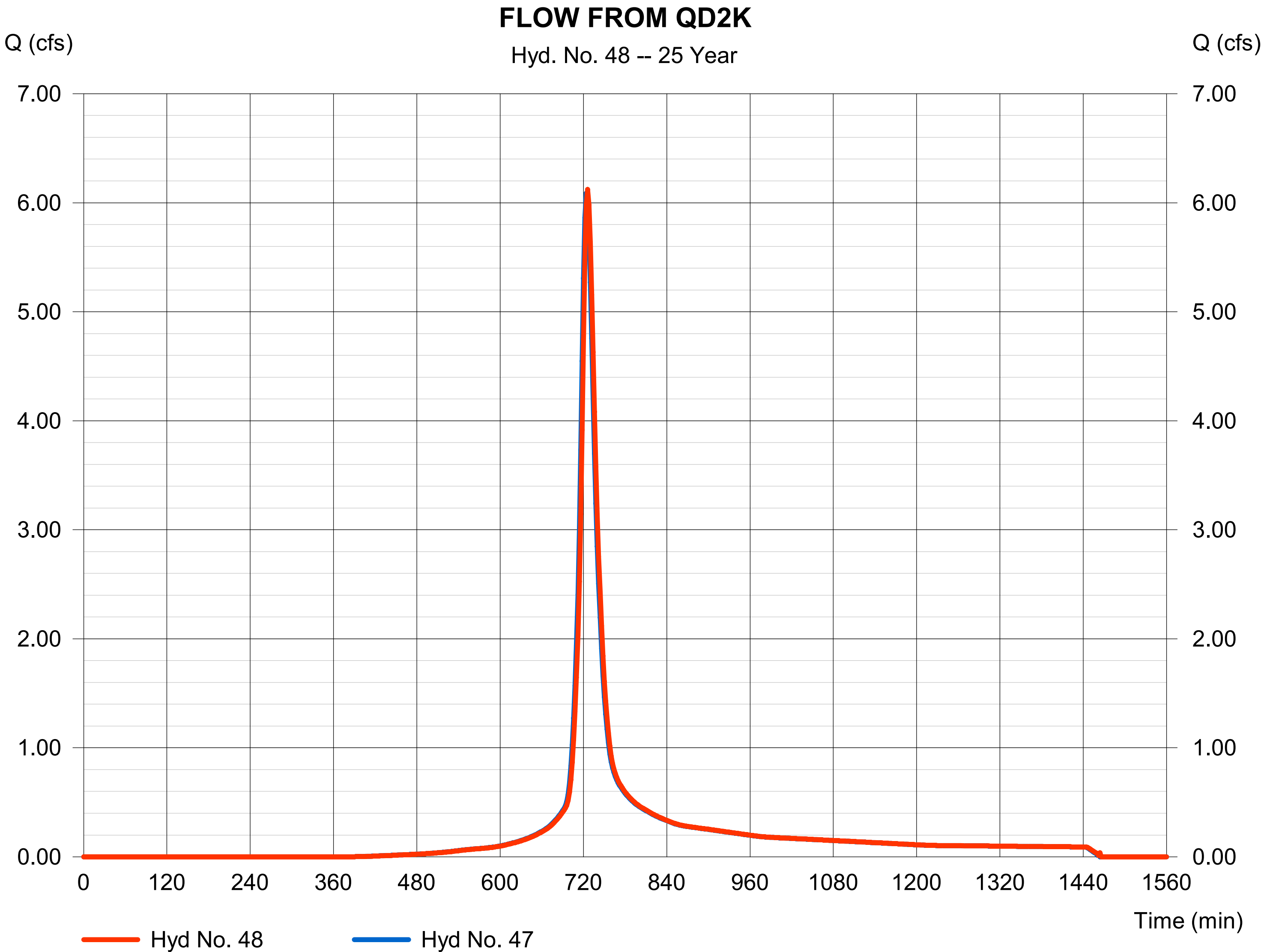


Hyd. No. 48

FLOW FROM QD2K

Hydrograph type	= Reach	Peak discharge	= 6.122 cfs
Storm frequency	= 25 yrs	Time to peak	= 726 min
Time interval	= 2 min	Hyd. volume	= 20,403 cuft
Inflow hyd. No.	= 47 - FLOW FROM QD2K	Section type	= Circular
Reach length	= 75.6 ft	Channel slope	= 0.3 %
Manning's n	= 0.024	Bottom width	= 2.0 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.060	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.3160

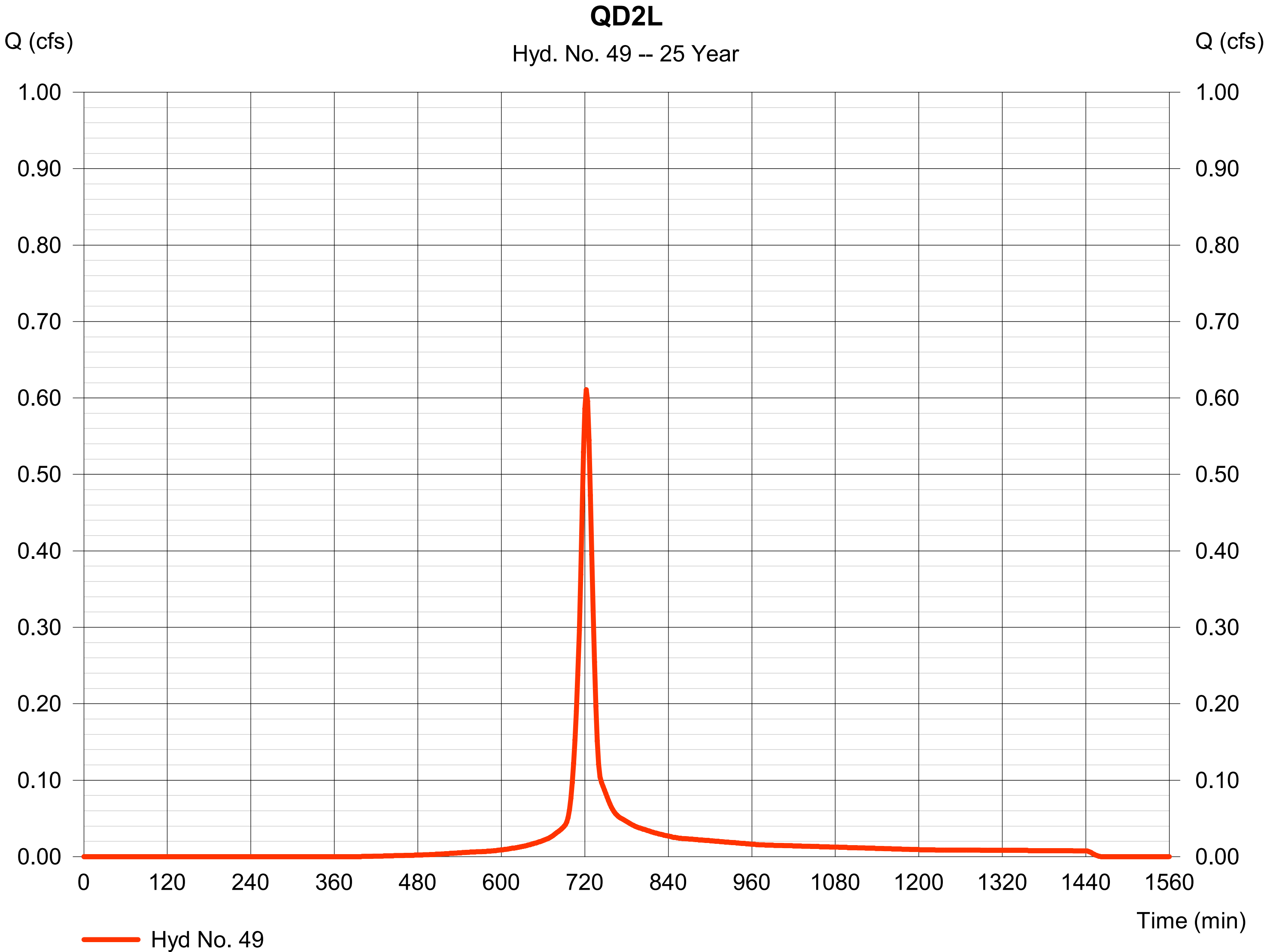
Modified Att-Kin routing method used.



Hyd. No. 49

QD2L

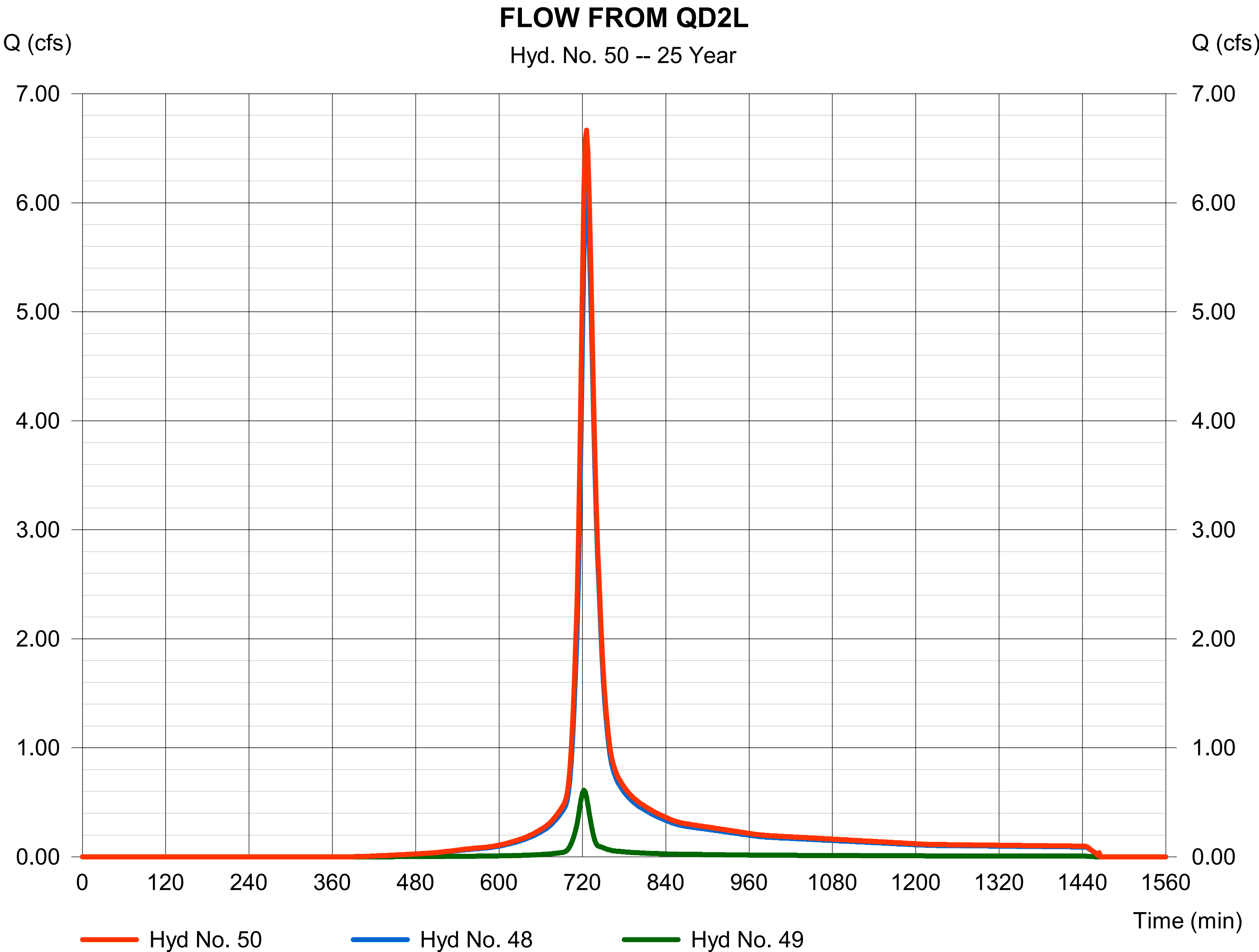
Hydrograph type	= SCS Runoff	Peak discharge	= 0.611 cfs
Storm frequency	= 25 yrs	Time to peak	= 722 min
Time interval	= 2 min	Hyd. volume	= 1,721 cuft
Drainage area	= 0.300 ac	Curve number	= 90
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 14.30 min
Total precip.	= 2.60 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hyd. No. 50

FLOW FROM QD2L

Hydrograph type	= Combine	Peak discharge	= 6.666 cfs
Storm frequency	= 25 yrs	Time to peak	= 726 min
Time interval	= 2 min	Hyd. volume	= 22,124 cuft
Inflow hyds.	= 48, 49	Contrib. drain. area	= 0.300 ac

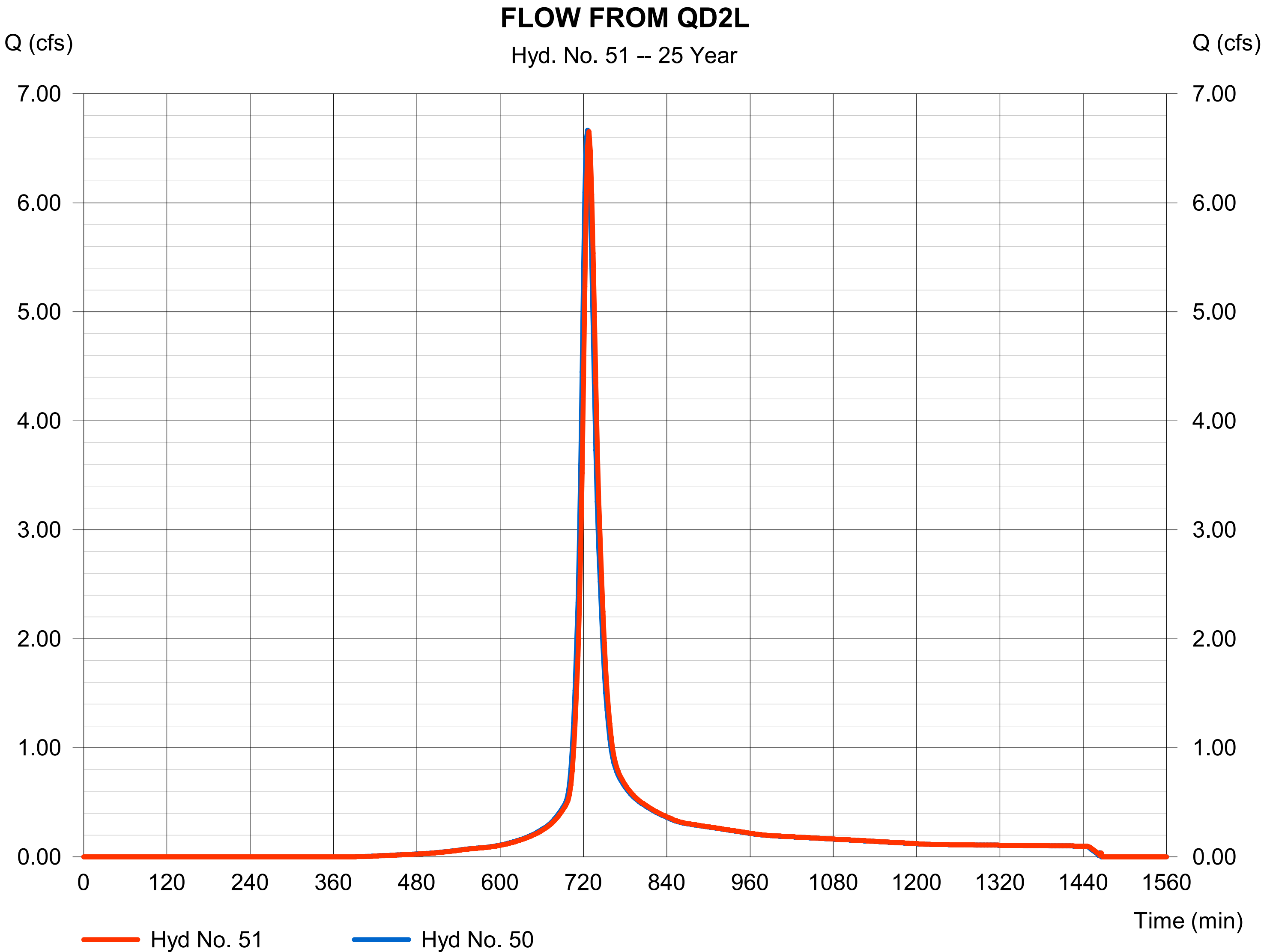


Hyd. No. 51

FLOW FROM QD2L

Hydrograph type	= Reach	Peak discharge	= 6.652 cfs
Storm frequency	= 25 yrs	Time to peak	= 728 min
Time interval	= 2 min	Hyd. volume	= 22,124 cuft
Inflow hyd. No.	= 50 - FLOW FROM QD2L	Section type	= Circular
Reach length	= 194.8 ft	Channel slope	= 0.3 %
Manning's n	= 0.024	Bottom width	= 2.0 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.060	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 0.8992

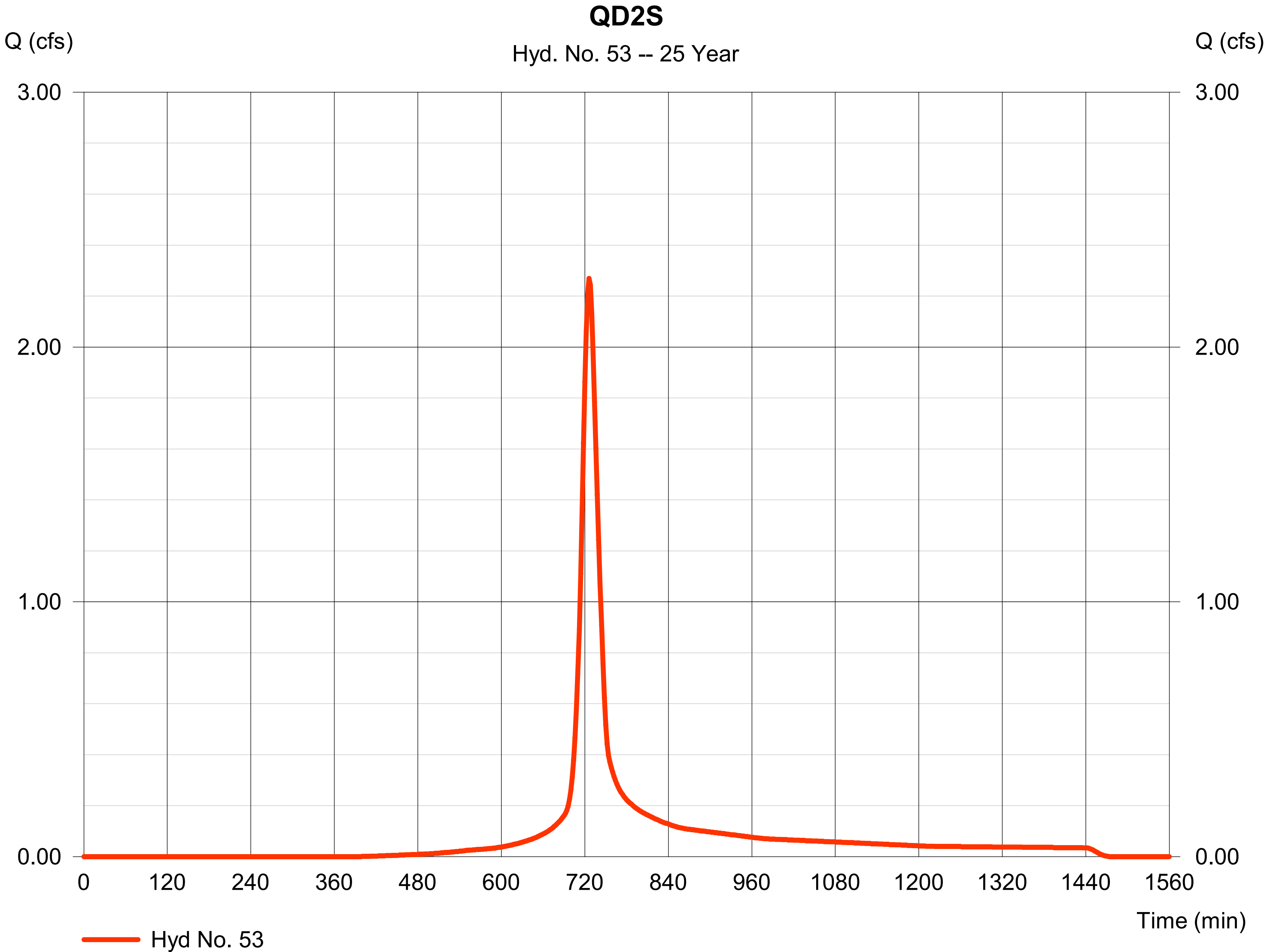
Modified Att-Kin routing method used.



Hyd. No. 53

QD2S

Hydrograph type	= SCS Runoff	Peak discharge	= 2.269 cfs
Storm frequency	= 25 yrs	Time to peak	= 726 min
Time interval	= 2 min	Hyd. volume	= 7,844 cuft
Drainage area	= 1.310 ac	Curve number	= 90
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 23.20 min
Total precip.	= 2.60 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

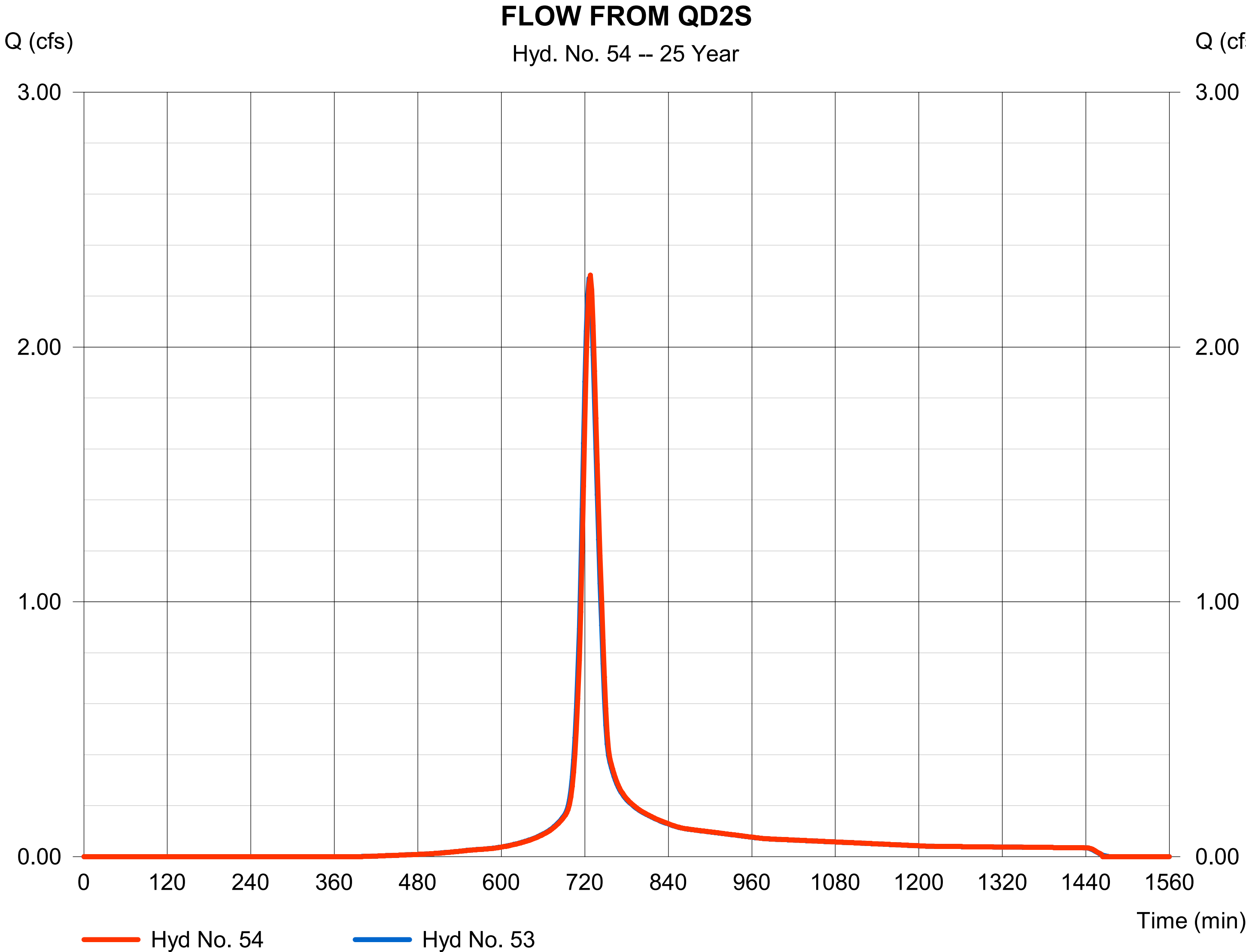


Hyd. No. 54

FLOW FROM QD2S

Hydrograph type	= Reach	Peak discharge	= 2.282 cfs
Storm frequency	= 25 yrs	Time to peak	= 728 min
Time interval	= 2 min	Hyd. volume	= 7,841 cuft
Inflow hyd. No.	= 53 - QD2S	Section type	= Circular
Reach length	= 28.7 ft	Channel slope	= 0.5 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.509	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.5412

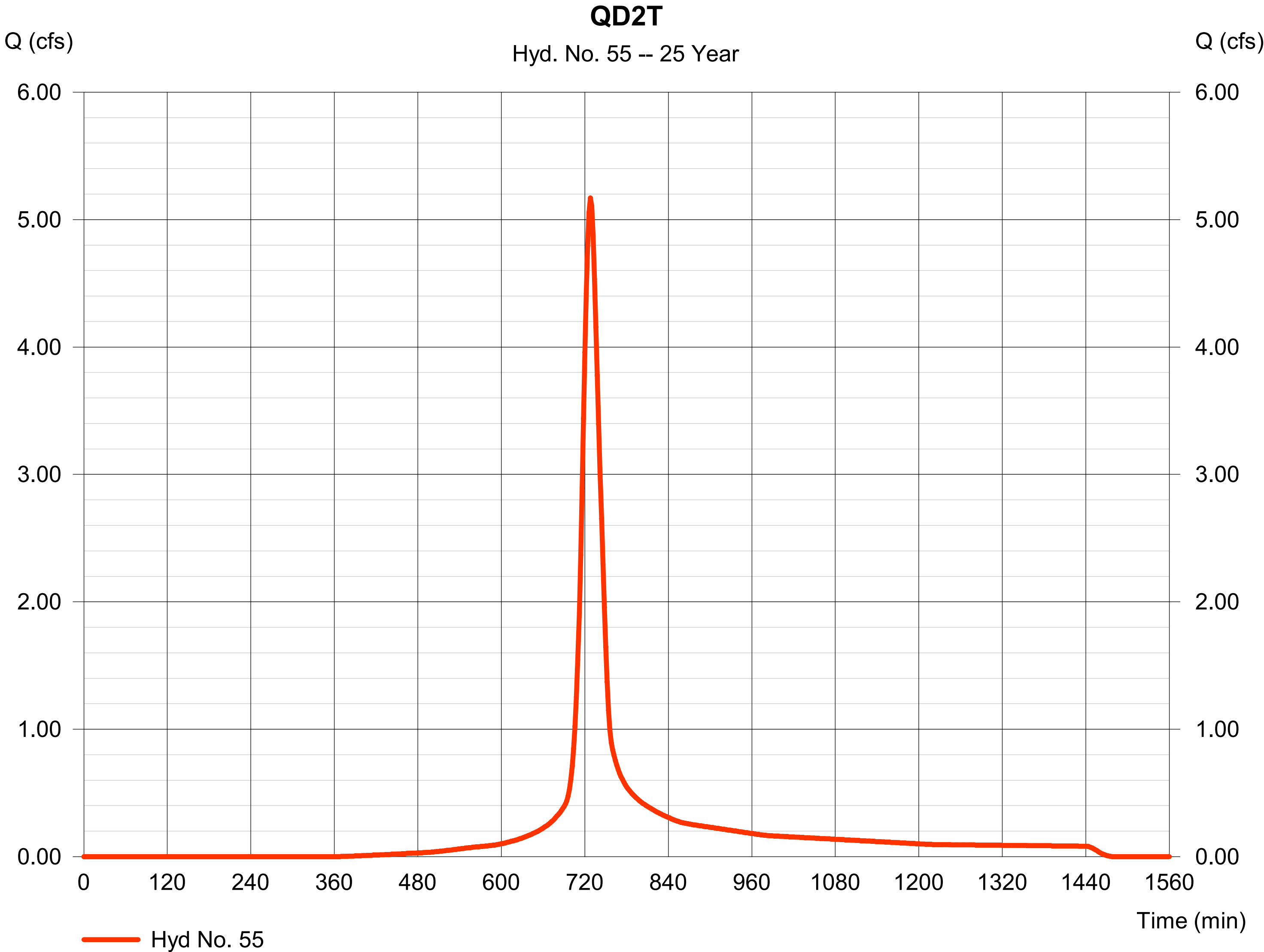
Modified Att-Kin routing method used.



Hyd. No. 55

QD2T

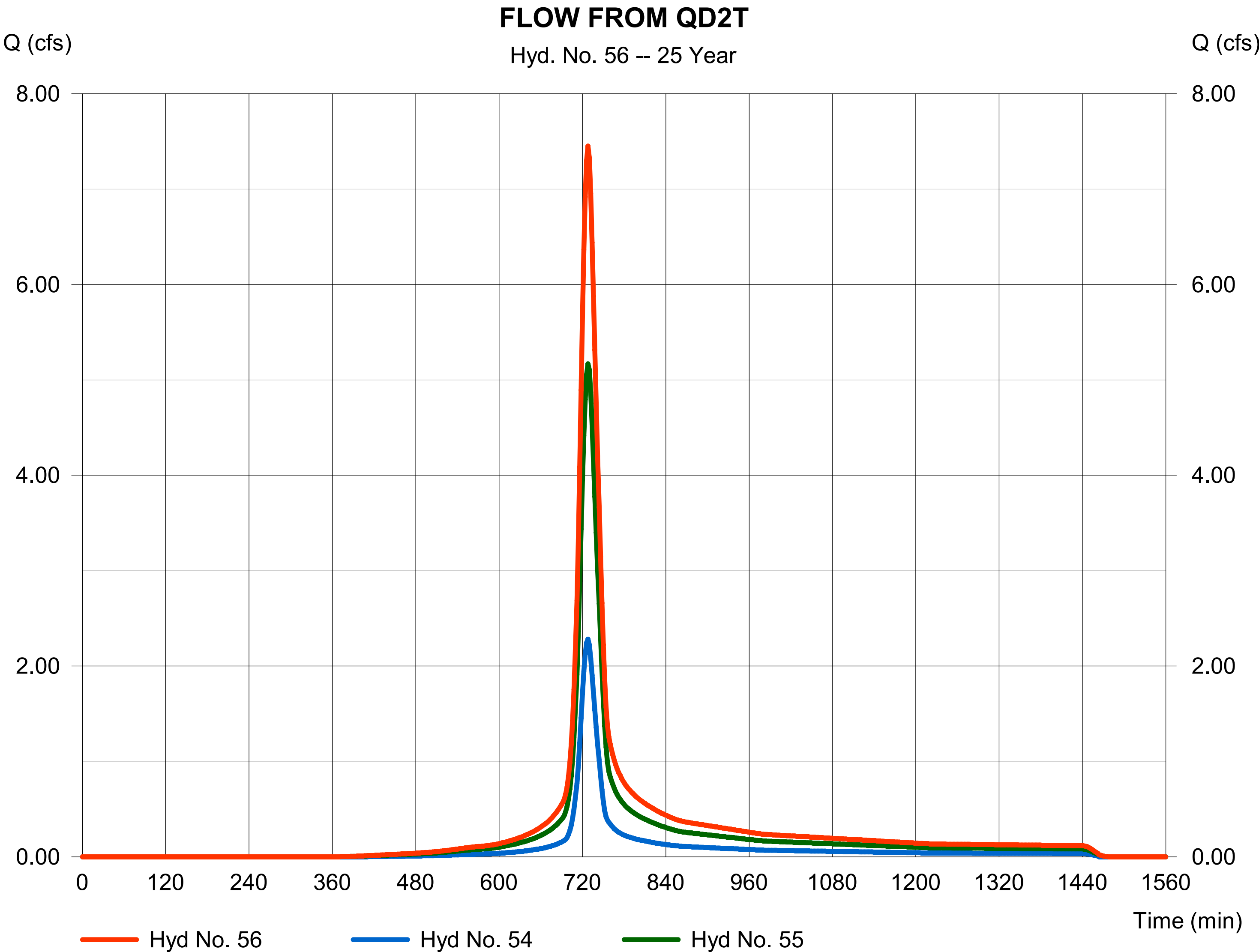
Hydrograph type	= SCS Runoff	Peak discharge	= 5.169 cfs
Storm frequency	= 25 yrs	Time to peak	= 728 min
Time interval	= 2 min	Hyd. volume	= 19,032 cuft
Drainage area	= 3.130 ac	Curve number	= 91
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 24.20 min
Total precip.	= 2.60 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hyd. No. 56

FLOW FROM QD2T

Hydrograph type	= Combine	Peak discharge	= 7.452 cfs
Storm frequency	= 25 yrs	Time to peak	= 728 min
Time interval	= 2 min	Hyd. volume	= 26,873 cuft
Inflow hyds.	= 54, 55	Contrib. drain. area	= 3.130 ac

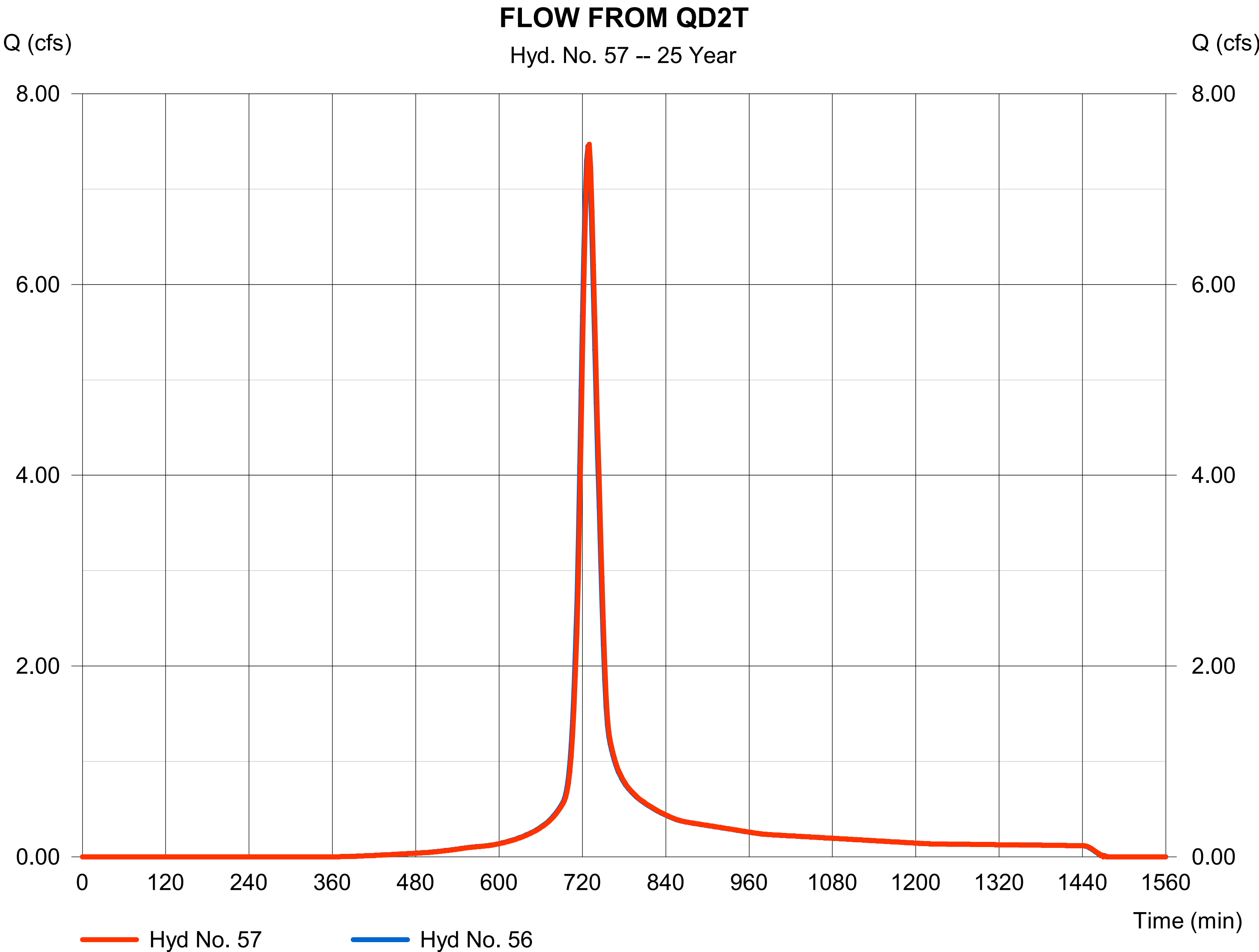


Hyd. No. 57

FLOW FROM QD2T

Hydrograph type	= Reach	Peak discharge	= 7.472 cfs
Storm frequency	= 25 yrs	Time to peak	= 730 min
Time interval	= 2 min	Hyd. volume	= 26,871 cuft
Inflow hyd. No.	= 56 - FLOW FROM QD2T	Section type	= Circular
Reach length	= 55.6 ft	Channel slope	= 0.5 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.509	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.7008

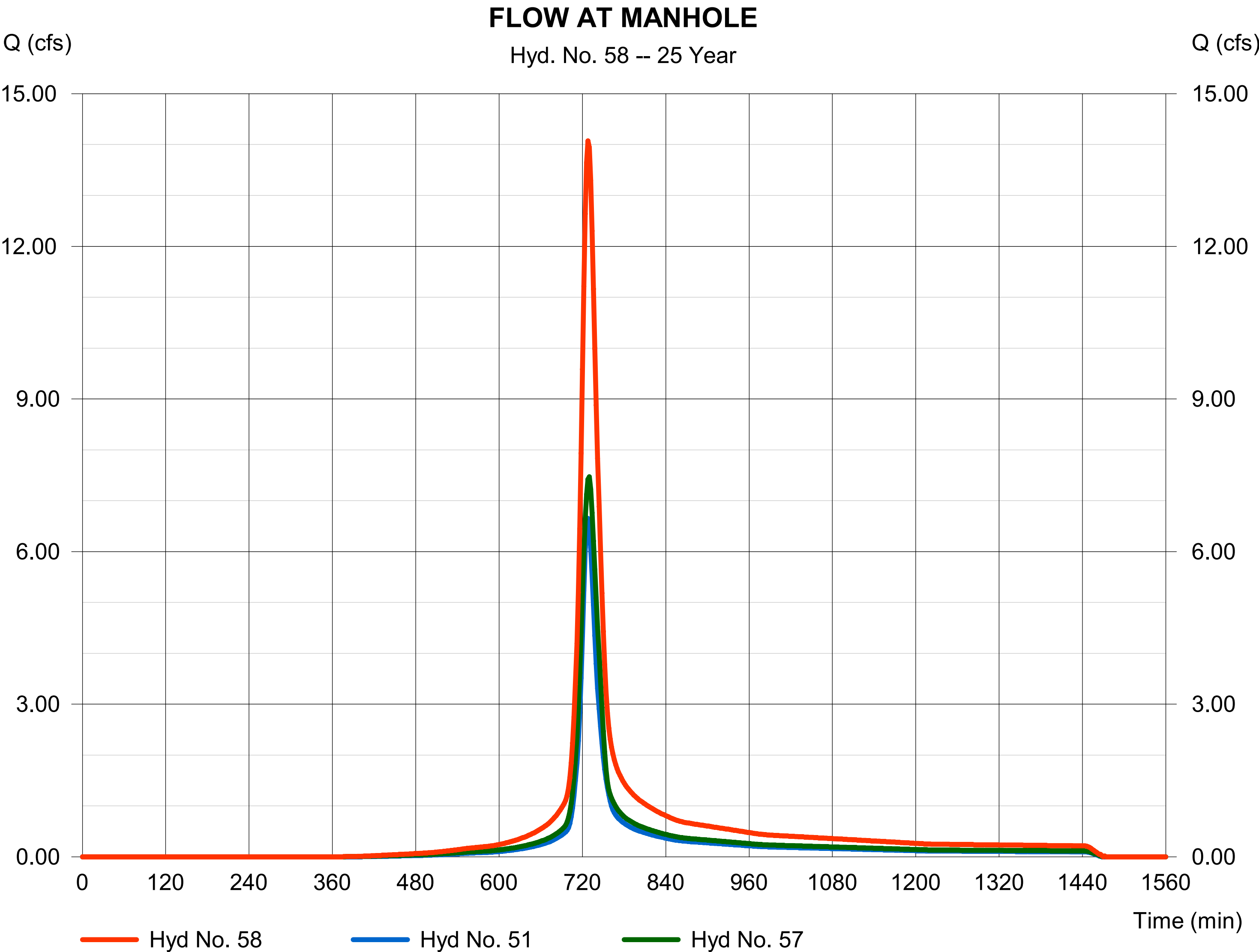
Modified Att-Kin routing method used.



Hyd. No. 58

FLOW AT MANHOLE

Hydrograph type	= Combine	Peak discharge	= 14.07 cfs
Storm frequency	= 25 yrs	Time to peak	= 728 min
Time interval	= 2 min	Hyd. volume	= 48,995 cuft
Inflow hyds.	= 51, 57	Contrib. drain. area	= 0.000 ac

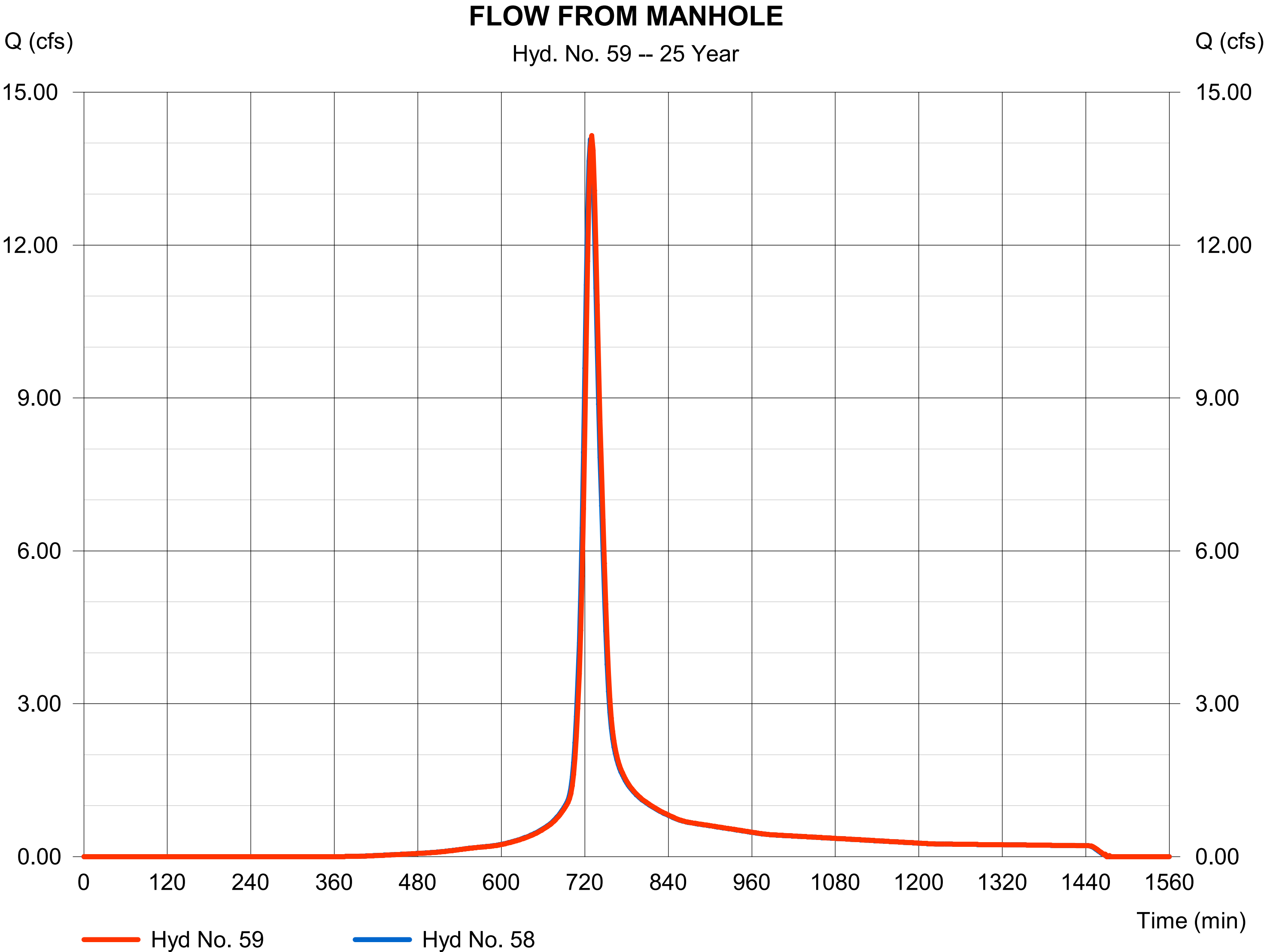


Hyd. No. 59

FLOW FROM MANHOLE

Hydrograph type	= Reach	Peak discharge	= 14.15 cfs
Storm frequency	= 25 yrs	Time to peak	= 730 min
Time interval	= 2 min	Hyd. volume	= 48,995 cuft
Inflow hyd. No.	= 58 - FLOW AT MANHOLE	Section type	= Circular
Reach length	= 141.5 ft	Channel slope	= 0.3 %
Manning's n	= 0.024	Bottom width	= 2.0 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.060	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.4073

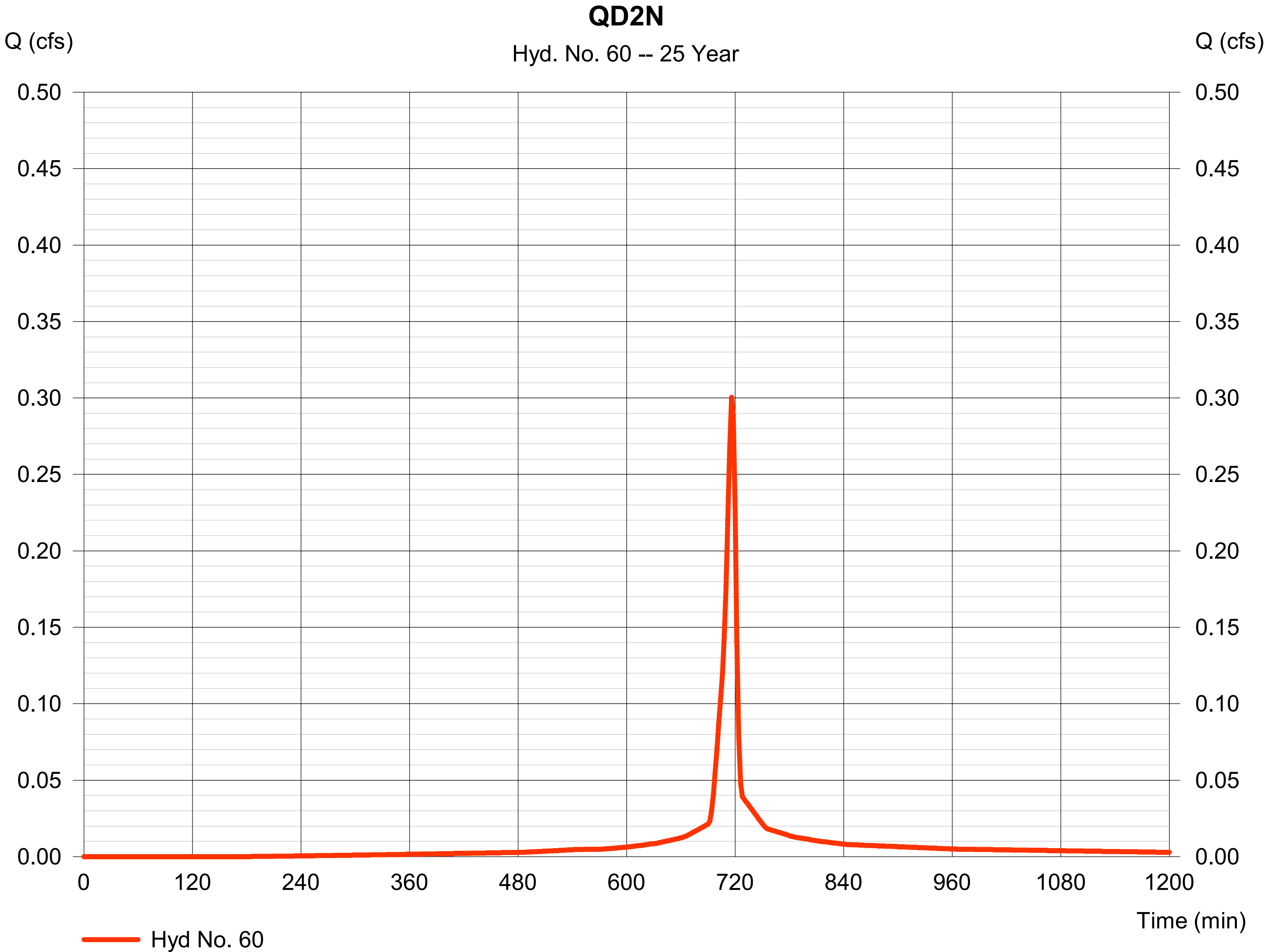
Modified Att-Kin routing method used.



Hyd. No. 60

QD2N

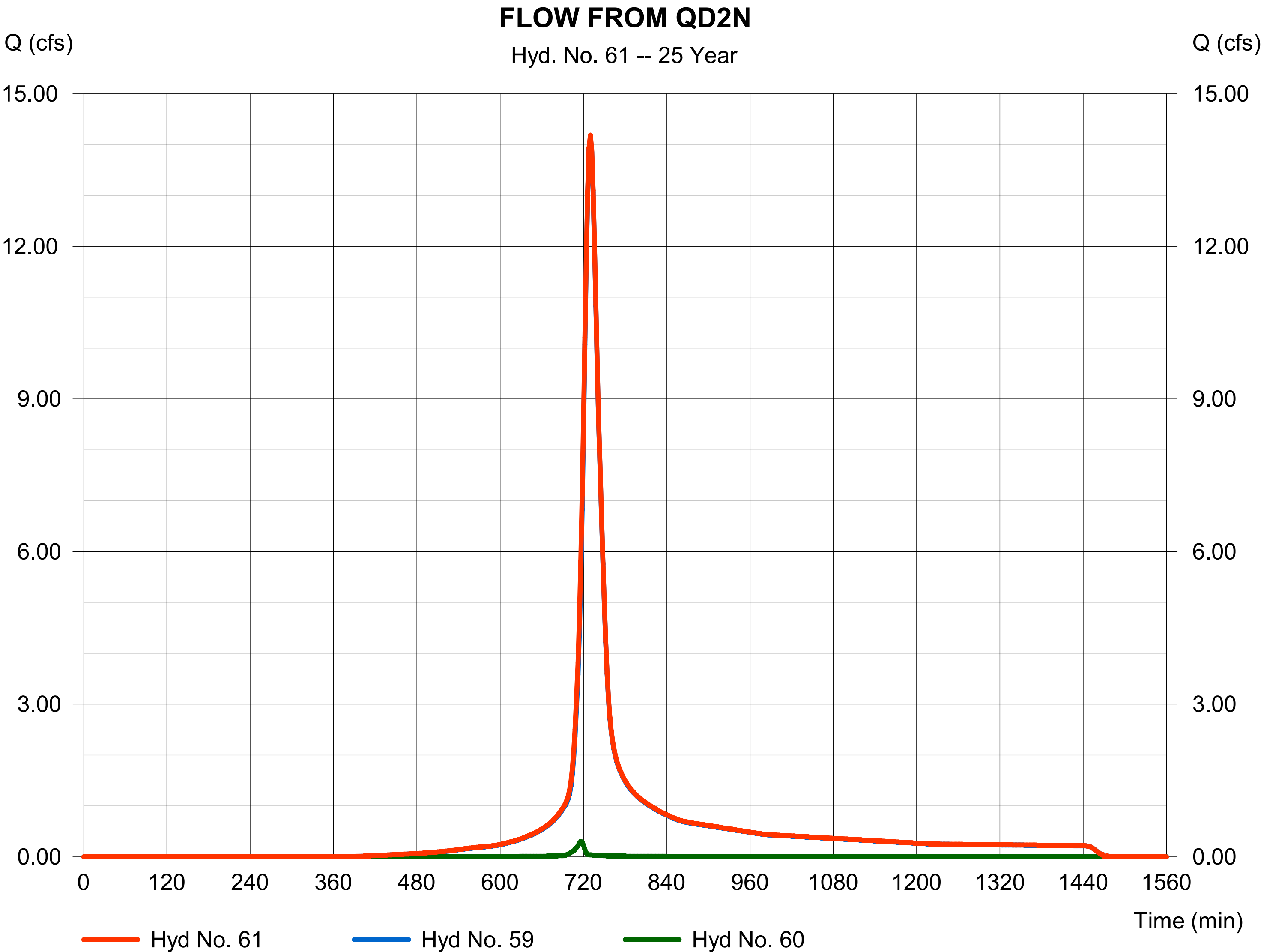
Hydrograph type	= SCS Runoff	Peak discharge	= 0.300 cfs
Storm frequency	= 25 yrs	Time to peak	= 716 min
Time interval	= 2 min	Hyd. volume	= 661 cuft
Drainage area	= 0.090 ac	Curve number	= 96
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 6.00 min
Total precip.	= 2.60 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hyd. No. 61

FLOW FROM QD2N

Hydrograph type	= Combine	Peak discharge	= 14.19 cfs
Storm frequency	= 25 yrs	Time to peak	= 730 min
Time interval	= 2 min	Hyd. volume	= 49,657 cuft
Inflow hyds.	= 59, 60	Contrib. drain. area	= 0.090 ac

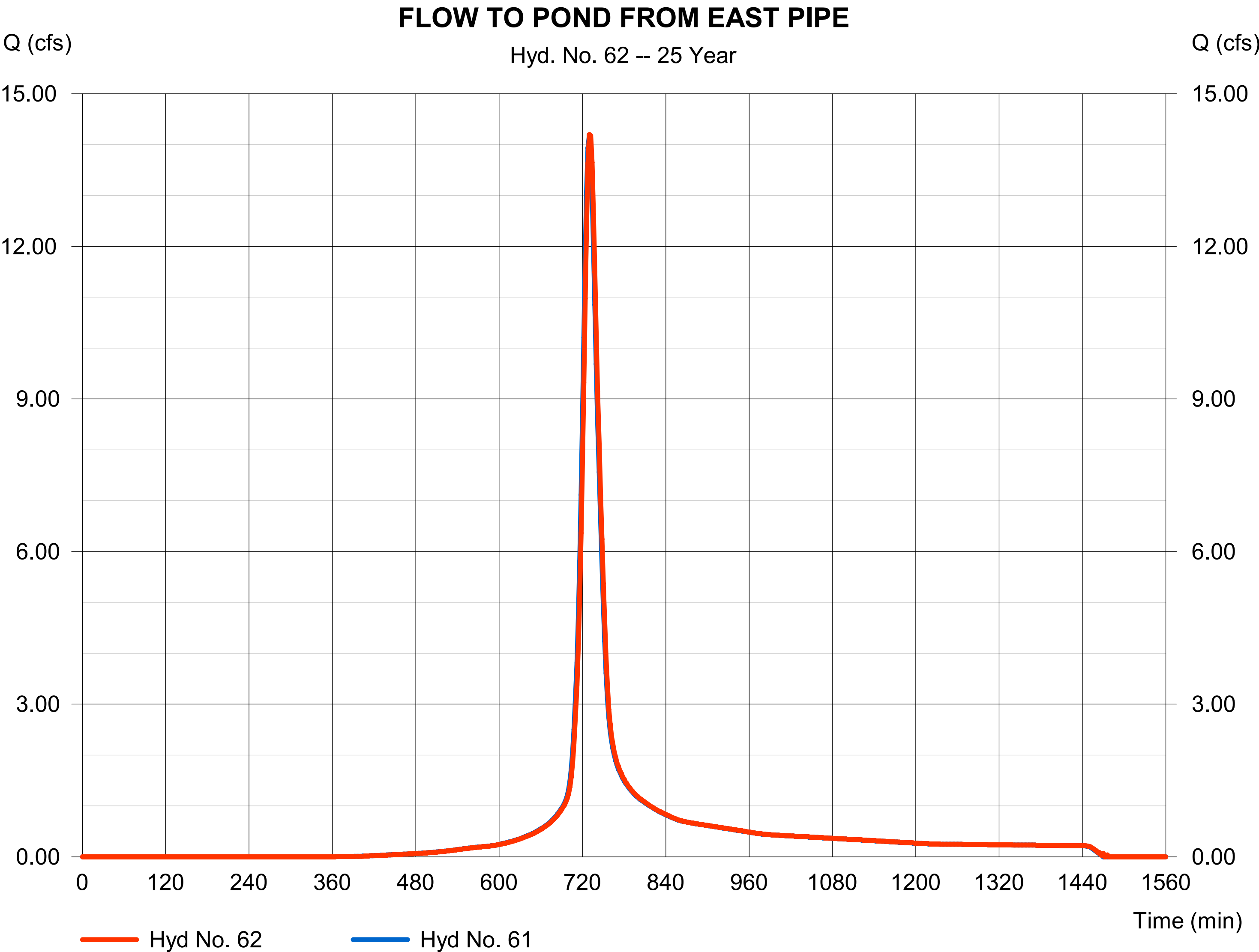


Hyd. No. 62

FLOW TO POND FROM EAST PIPE

Hydrograph type	= Reach	Peak discharge	= 14.20 cfs
Storm frequency	= 25 yrs	Time to peak	= 730 min
Time interval	= 2 min	Hyd. volume	= 49,663 cuft
Inflow hyd. No.	= 61 - FLOW FROM QD2N	Section type	= Circular
Reach length	= 23.1 ft	Channel slope	= 0.3 %
Manning's n	= 0.024	Bottom width	= 2.0 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.060	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.8724

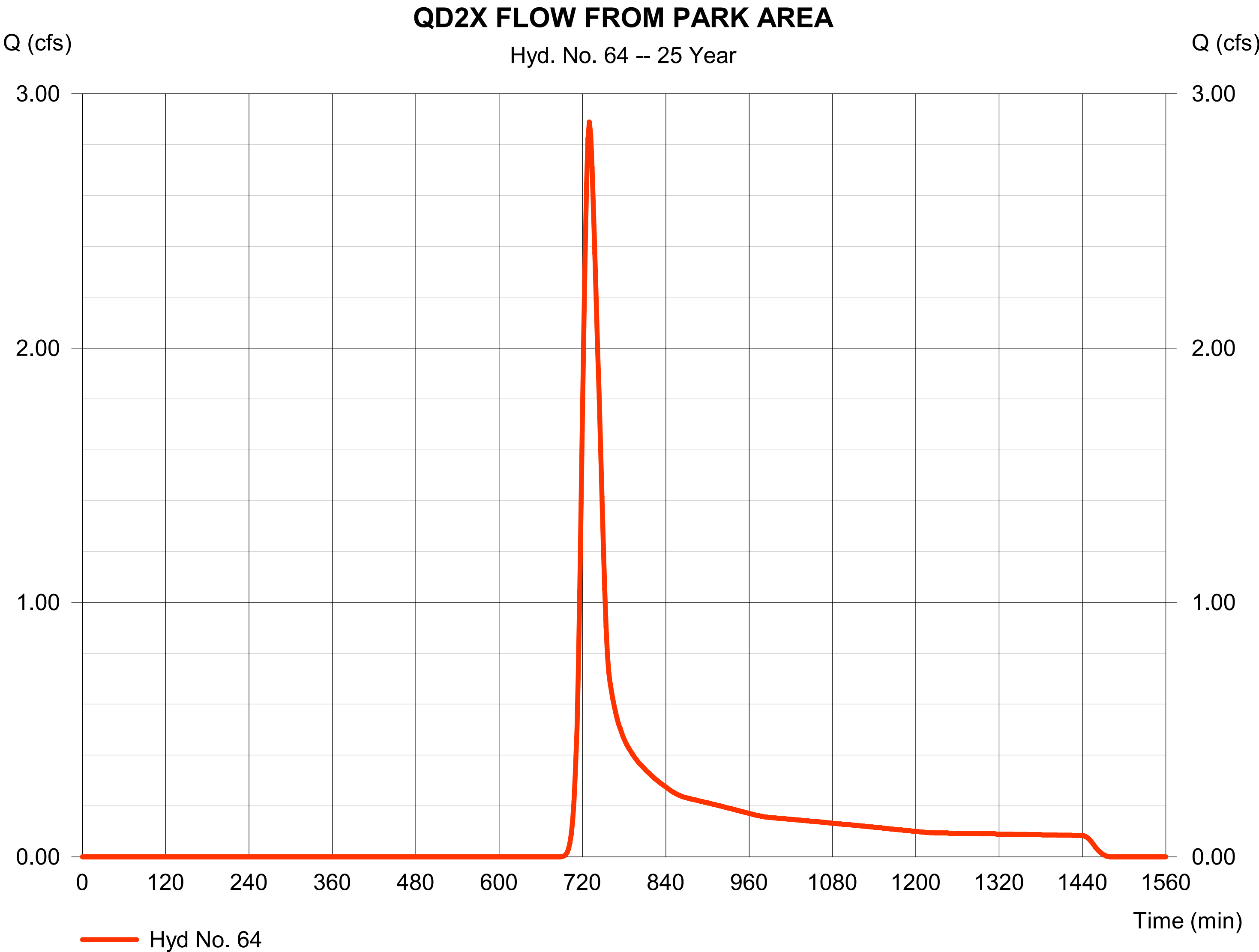
Modified Att-Kin routing method used.



Hyd. No. 64

QD2X FLOW FROM PARK AREA

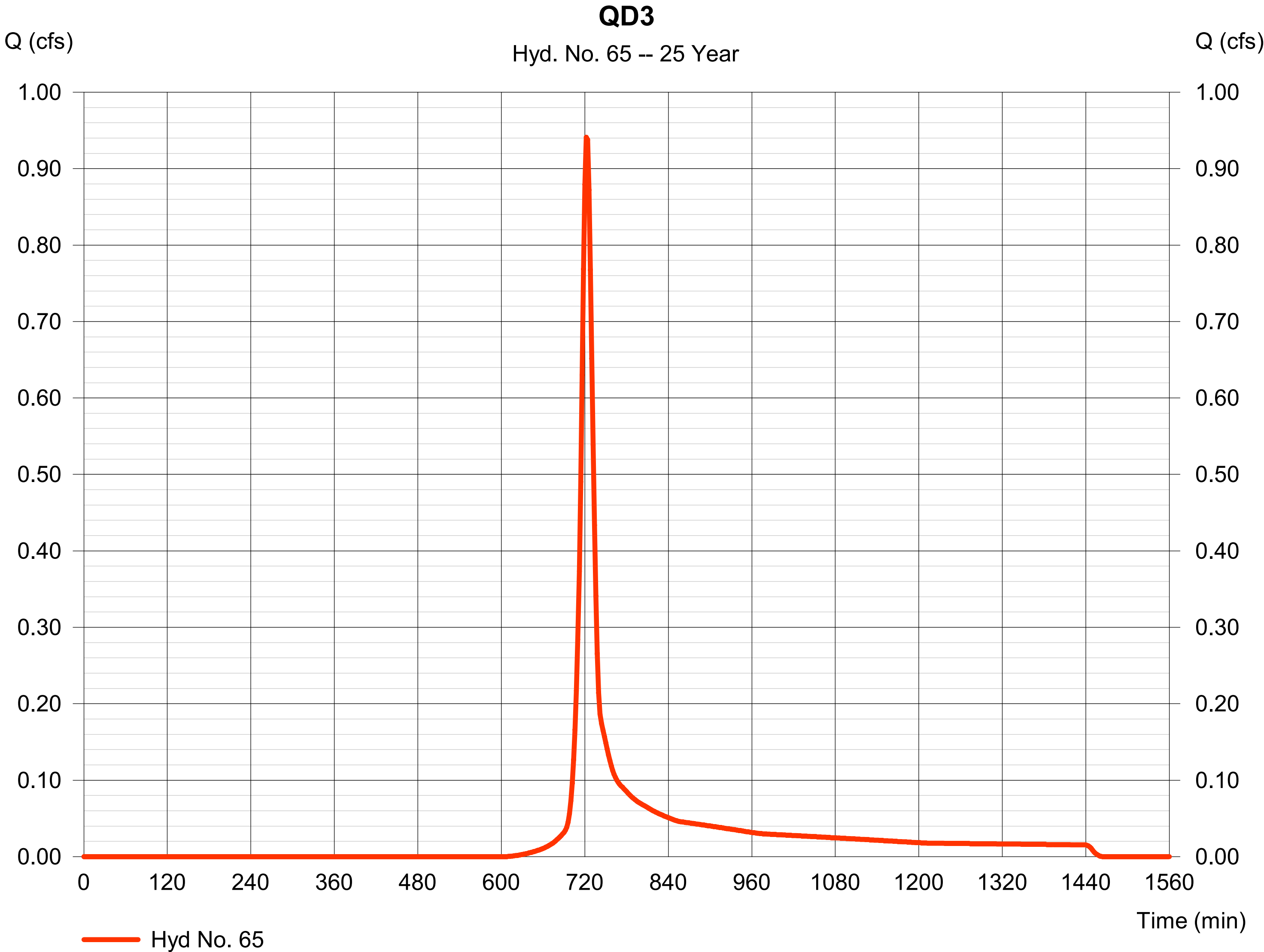
Hydrograph type	=	SCS Runoff	Peak discharge	=	2.889 cfs
Storm frequency	=	25 yrs	Time to peak	=	730 min
Time interval	=	2 min	Hyd. volume	=	12,005 cuft
Drainage area	=	5.050 ac	Curve number	=	74
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	25.00 min
Total precip.	=	2.60 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hyd. No. 65

QD3

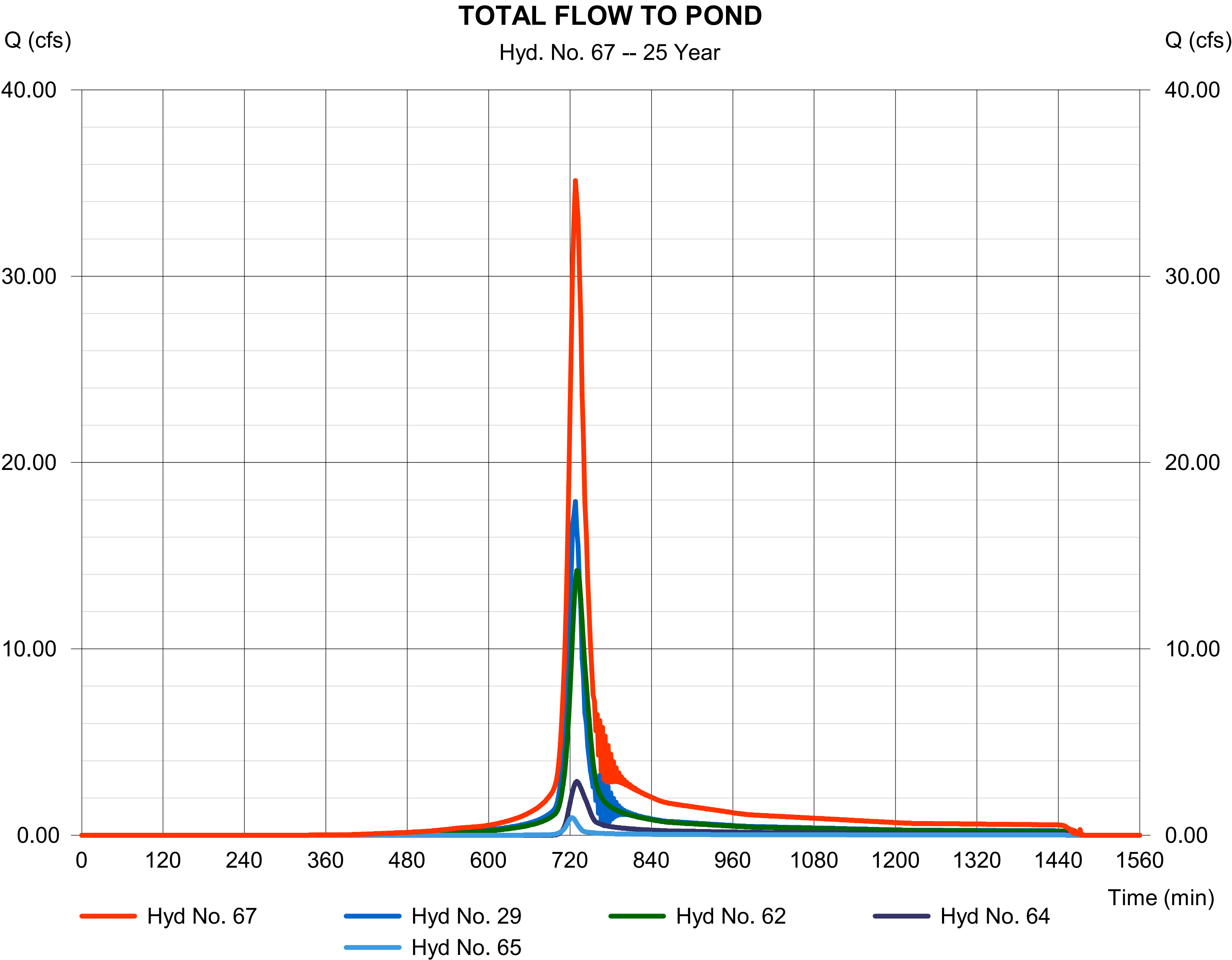
Hydrograph type	= SCS Runoff	Peak discharge	= 0.941 cfs
Storm frequency	= 25 yrs	Time to peak	= 722 min
Time interval	= 2 min	Hyd. volume	= 2,692 cuft
Drainage area	= 0.750 ac	Curve number	= 81
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 14.30 min
Total precip.	= 2.60 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hyd. No. 67

TOTAL FLOW TO POND

Hydrograph type	= Combine	Peak discharge	= 35.13 cfs
Storm frequency	= 25 yrs	Time to peak	= 728 min
Time interval	= 2 min	Hyd. volume	= 119,328 cuft
Inflow hyds.	= 29, 62, 64, 65	Contrib. drain. area	= 5.800 ac

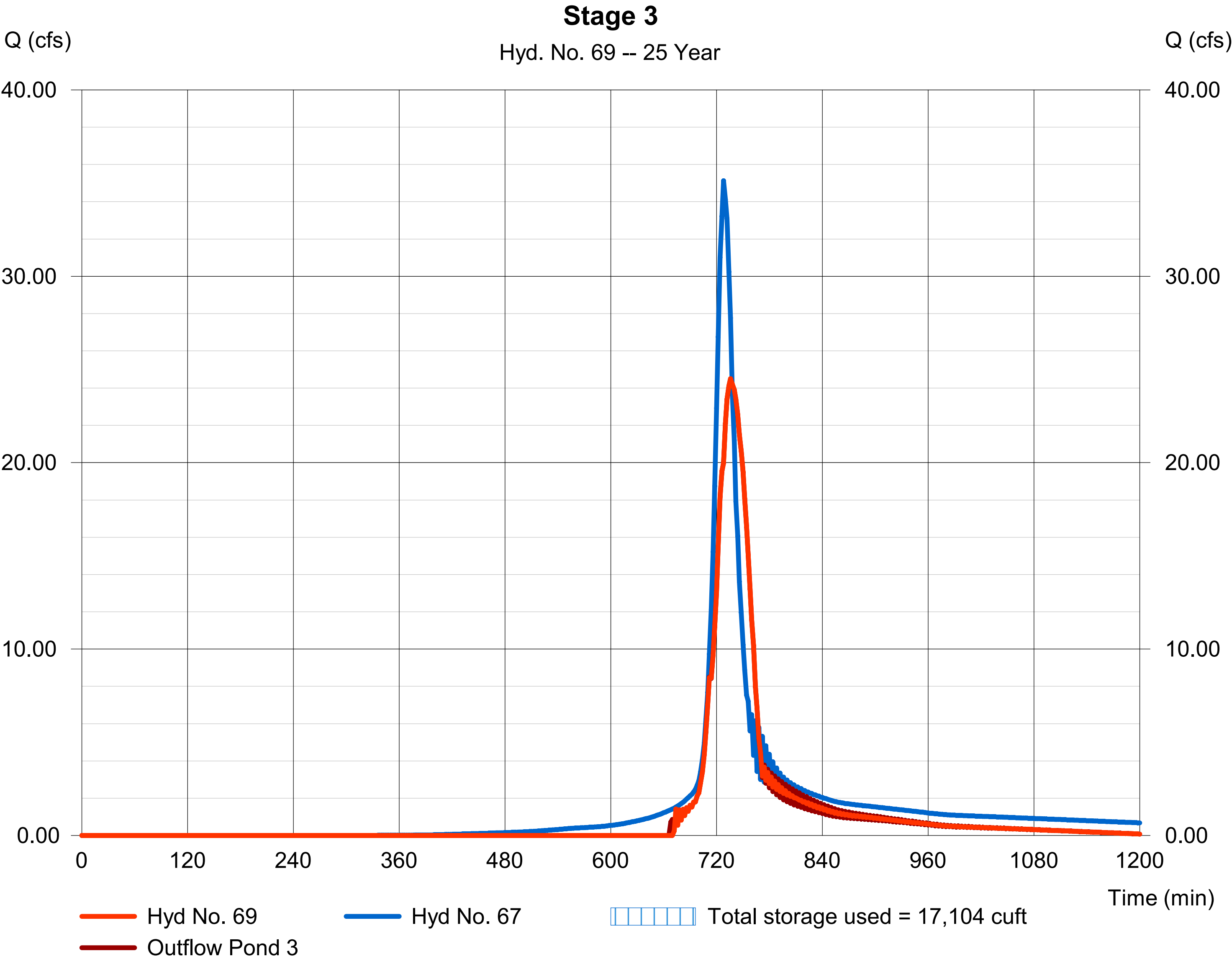


Hyd. No. 69

Stage 3

Hydrograph type	= Reservoir (Interconnected)	Peak discharge	= 24.51 cfs
Storm frequency	= 25 yrs	Time to peak	= 736 min
Time interval	= 2 min	Hyd. volume	= 84,038 cuft
Upper Pond	= Stage 3	Lower Pond	= outfall
Inflow hyd.	= 67 - TOTAL FLOW TO POND	Other Inflow hyd.	= None
Max. Elevation	= 6476.21 ft	Max. Elevation	= 6475.05 ft
Max. Storage	= 16,820 cuft	Max. Storage	= 284 cuft

Interconnected Pond Routing. Storage Indication method used.



Pond No. 3 - Stage 3

Pond Data

Contours -User-defined contour areas. Average end area method used for volume calculation. Begining Elevation = 6473.50 ft

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	6473.50	4,109	0	0
0.50	6474.00	4,552	2,165	2,165
0.99	6474.50	5,017	2,344	4,510
1.00	6475.00	13,806	94	4,604
1.50	6475.50	15,157	7,241	11,845
2.00	6475.50	17,016	8,043	19,888
2.49	6475.99	18,125	8,610	28,497
2.50	6476.00	28,001	231	28,728
3.00	6476.50	31,441	14,861	43,588

Culvert / Orifice Structures

	[A]	[B]	[C]	[PrfRsr]
Rise (in)	= 24.00	0.00	0.00	0.00
Span (in)	= 35.00	0.00	0.00	0.00
No. Barrels	= 1	0	0	0
Invert El. (ft)	= 6473.50	0.00	0.00	0.00
Length (ft)	= 25.00	0.00	0.00	0.00
Slope (%)	= 1.50	0.00	0.00	n/a
N-Value	= .024	.013	.013	n/a
Orifice Coeff.	= 0.60	0.60	0.60	0.60
Multi-Stage	= n/a	No	No	No

Weir Structures

	[A]	[B]	[C]	[D]
Crest Len (ft)	= 50.00	0.00	0.00	0.00
Crest El. (ft)	= 6476.00	0.00	0.00	0.00
Weir Coeff.	= 3.33	3.33	3.33	3.33
Weir Type	= Ciplti	---	---	---
Multi-Stage	= No	No	No	No
Exfil.(in/hr)	= 2.000 (by Contour)			
TW Elev. (ft)	= 0.00			

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

Stage / Storage / Discharge Table

Stage ft	Storage cuft	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	PrfRsr cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	User cfs	Total cfs
0.00	0	6473.50	0.00	---	---	---	0.00	---	---	---	0.000	---	0.000
0.05	217	6473.55	0.11 ic	---	---	---	0.00	---	---	---	0.021	---	0.131
0.10	433	6473.60	0.31 ic	---	---	---	0.00	---	---	---	0.042	---	0.357
0.15	650	6473.65	0.57 ic	---	---	---	0.00	---	---	---	0.063	---	0.634
0.20	866	6473.70	0.88 ic	---	---	---	0.00	---	---	---	0.084	---	0.967
0.25	1,083	6473.75	1.23 ic	---	---	---	0.00	---	---	---	0.105	---	1.339
0.30	1,299	6473.80	1.62 ic	---	---	---	0.00	---	---	---	0.126	---	1.749
0.35	1,516	6473.85	2.05 ic	---	---	---	0.00	---	---	---	0.148	---	2.196
0.40	1,732	6473.90	2.49 ic	---	---	---	0.00	---	---	---	0.169	---	2.662
0.45	1,949	6473.95	2.98 ic	---	---	---	0.00	---	---	---	0.190	---	3.170
0.50	2,165	6474.00	3.51 ic	---	---	---	0.00	---	---	---	0.211	---	3.722
0.55	2,400	6474.05	4.04 ic	---	---	---	0.00	---	---	---	0.213	---	4.250
0.60	2,634	6474.10	4.59 ic	---	---	---	0.00	---	---	---	0.215	---	4.809
0.65	2,869	6474.15	5.16 ic	---	---	---	0.00	---	---	---	0.217	---	5.373
0.70	3,103	6474.19	5.75 ic	---	---	---	0.00	---	---	---	0.219	---	5.971
0.75	3,337	6474.24	6.37 ic	---	---	---	0.00	---	---	---	0.222	---	6.596
0.79	3,572	6474.29	7.01 ic	---	---	---	0.00	---	---	---	0.224	---	7.236
0.84	3,806	6474.34	7.67 ic	---	---	---	0.00	---	---	---	0.226	---	7.895
0.89	4,041	6474.39	8.21 oc	---	---	---	0.00	---	---	---	0.228	---	8.442
0.94	4,275	6474.44	8.72 oc	---	---	---	0.00	---	---	---	0.230	---	8.955
0.99	4,510	6474.50	9.38 oc	---	---	---	0.00	---	---	---	0.232	---	9.610
0.99	4,519	6474.50	9.39 oc	---	---	---	0.00	---	---	---	0.273	---	9.661
0.99	4,528	6474.50	9.40 oc	---	---	---	0.00	---	---	---	0.314	---	9.712
0.99	4,538	6474.50	9.41 oc	---	---	---	0.00	---	---	---	0.354	---	9.763
0.99	4,547	6474.50	9.42 oc	---	---	---	0.00	---	---	---	0.395	---	9.814
0.99	4,557	6474.50	9.43 oc	---	---	---	0.00	---	---	---	0.436	---	9.865
1.00	4,566	6474.51	9.44 oc	---	---	---	0.00	---	---	---	0.476	---	9.916
1.00	4,576	6474.51	9.45 oc	---	---	---	0.00	---	---	---	0.517	---	9.968
1.00	4,585	6474.51	9.46 oc	---	---	---	0.00	---	---	---	0.558	---	10.02
1.00	4,594	6474.51	9.47 oc	---	---	---	0.00	---	---	---	0.598	---	10.07
1.00	4,604	6475.00	14.75 oc	---	---	---	0.00	---	---	---	0.639	---	15.39
1.05	5,328	6475.05	15.29 oc	---	---	---	0.00	---	---	---	0.645	---	15.94
1.10	6,052	6475.10	15.83 oc	---	---	---	0.00	---	---	---	0.652	---	16.48
1.15	6,776	6475.15	16.36 oc	---	---	---	0.00	---	---	---	0.658	---	17.02

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Stage 3

Stage / Storage / Discharge Table

Stage ft	Storage cuft	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	PrfRsr cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	User cfs	Total cfs
1.20	7,500	6475.20	16.90 oc	---	---	---	0.00	---	---	---	0.664	---	17.57
1.25	8,224	6475.25	17.44 oc	---	---	---	0.00	---	---	---	0.670	---	18.11
1.30	8,948	6475.30	17.98 oc	---	---	---	0.00	---	---	---	0.677	---	18.65
1.35	9,672	6475.35	18.52 oc	---	---	---	0.00	---	---	---	0.683	---	19.20
1.40	10,396	6475.40	19.05 oc	---	---	---	0.00	---	---	---	0.689	---	19.74
1.45	11,120	6475.45	19.59 oc	---	---	---	0.00	---	---	---	0.695	---	20.29
1.50	11,845	6475.50	18.75 oc	---	---	---	0.00	---	---	---	0.702	---	19.46
1.55	12,649	6475.55	19.96 oc	---	---	---	0.00	---	---	---	0.710	---	20.67
1.60	13,453	6475.60	21.11 oc	---	---	---	0.00	---	---	---	0.719	---	21.83
1.65	14,258	6475.65	22.17 oc	---	---	---	0.00	---	---	---	0.728	---	22.90
1.70	15,062	6475.70	23.21 oc	---	---	---	0.00	---	---	---	0.736	---	23.94
1.75	15,866	6475.75	24.19 oc	---	---	---	0.00	---	---	---	0.745	---	24.94
1.80	16,670	6475.80	25.14 oc	---	---	---	0.00	---	---	---	0.753	---	25.89
1.85	17,475	6475.85	26.06 oc	---	---	---	0.00	---	---	---	0.762	---	26.82
1.90	18,279	6475.90	26.93 oc	---	---	---	0.00	---	---	---	0.771	---	27.70
1.95	19,083	6475.95	27.79 oc	---	---	---	0.00	---	---	---	0.779	---	28.57
2.00	19,888	6475.50	18.75 oc	---	---	---	0.00	---	---	---	0.788	---	19.54
2.05	20,749	6475.55	19.94 oc	---	---	---	0.00	---	---	---	0.793	---	20.73
2.10	21,610	6475.60	21.07 oc	---	---	---	0.00	---	---	---	0.798	---	21.86
2.15	22,471	6475.65	22.11 oc	---	---	---	0.00	---	---	---	0.803	---	22.91
2.20	23,332	6475.69	23.12 oc	---	---	---	0.00	---	---	---	0.808	---	23.93
2.24	24,193	6475.74	24.10 oc	---	---	---	0.00	---	---	---	0.813	---	24.91
2.29	25,054	6475.79	25.03 oc	---	---	---	0.00	---	---	---	0.819	---	25.85
2.34	25,914	6475.84	25.93 oc	---	---	---	0.00	---	---	---	0.824	---	26.75
2.39	26,775	6475.89	26.81 oc	---	---	---	0.00	---	---	---	0.829	---	27.64
2.44	27,636	6475.94	27.63 oc	---	---	---	0.00	---	---	---	0.834	---	28.46
2.49	28,497	6475.99	28.49 oc	---	---	---	0.00	---	---	---	0.839	---	29.33
2.49	28,520	6475.99	28.50 oc	---	---	---	0.00	---	---	---	0.885	---	29.39
2.49	28,543	6475.99	28.52 oc	---	---	---	0.00	---	---	---	0.931	---	29.45
2.49	28,567	6475.99	28.54 oc	---	---	---	0.00	---	---	---	0.976	---	29.51
2.49	28,590	6475.99	28.55 oc	---	---	---	0.00	---	---	---	1.022	---	29.57
2.49	28,613	6476.00	28.57 oc	---	---	---	0.00	---	---	---	1.068	---	29.64
2.50	28,636	6476.00	28.58 oc	---	---	---	0.00	---	---	---	1.113	---	29.70
2.50	28,659	6476.00	28.60 oc	---	---	---	0.00	---	---	---	1.159	---	29.76
2.50	28,682	6476.00	28.62 oc	---	---	---	0.00	---	---	---	1.205	---	29.82
2.50	28,705	6476.00	28.63 oc	---	---	---	0.00	---	---	---	1.251	---	29.88
2.50	28,728	6476.00	28.65 oc	---	---	---	0.00	---	---	---	1.296	---	29.94
2.55	30,214	6476.05	29.45 oc	---	---	---	1.85	---	---	---	1.312	---	32.62
2.60	31,700	6476.10	30.24 oc	---	---	---	5.27	---	---	---	1.328	---	36.84
2.65	33,186	6476.15	30.99 oc	---	---	---	9.57	---	---	---	1.344	---	41.90
2.70	34,672	6476.20	31.74 oc	---	---	---	14.81	---	---	---	1.360	---	47.91
2.75	36,158	6476.25	32.47 oc	---	---	---	20.69	---	---	---	1.376	---	54.54
2.80	37,644	6476.30	33.18 oc	---	---	---	27.20	---	---	---	1.392	---	61.77
2.85	39,130	6476.35	33.88 oc	---	---	---	34.35	---	---	---	1.408	---	69.64
2.90	40,616	6476.40	34.55 oc	---	---	---	41.80	---	---	---	1.424	---	77.78
2.95	42,102	6476.45	35.23 oc	---	---	---	49.97	---	---	---	1.440	---	86.64
3.00	43,588	6476.50	35.91 oc	---	---	---	58.87	---	---	---	1.456	---	96.23

...End

Pond No. 4 - outfall

Pond Data

Contours -User-defined contour areas. Average end area method used for volume calculation. Begining Elevation = 6473.00 ft

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	6473.00	70	0	0
0.50	6473.50	102	43	43
1.00	6474.00	134	59	102
1.50	6474.50	169	76	178
2.00	6475.00	200	92	270
3.00	6476.00	400	300	570

Culvert / Orifice Structures

	[A]	[B]	[C]	[PrfRsr]
Rise (in)	= 0.00	0.00	0.00	0.00
Span (in)	= 0.00	0.00	0.00	0.00
No. Barrels	= 0	0	0	0
Invert El. (ft)	= 0.00	0.00	0.00	0.00
Length (ft)	= 0.00	0.00	0.00	0.00
Slope (%)	= 0.00	0.00	0.00	n/a
N-Value	= .013	.013	.013	n/a
Orifice Coeff.	= 0.60	0.60	0.60	0.60
Multi-Stage	= n/a	No	No	No

Weir Structures

	[A]	[B]	[C]	[D]
Crest Len (ft)	= 45.00	0.00	0.00	0.00
Crest El. (ft)	= 6474.75	0.00	0.00	0.00
Weir Coeff.	= 3.33	3.33	3.33	3.33
Weir Type	= Ciphti	---	---	---
Multi-Stage	= No	No	No	No
Exfil.(in/hr)	= 0.000 (by Wet area)			
TW Elev. (ft)	= 0.00			

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

Stage / Storage / Discharge Table

Stage ft	Storage cuft	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	PrfRsr cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	User cfs	Total cfs
0.00	0	6473.00	---	---	---	---	0.00	---	---	---	---	---	0.000
0.05	4	6473.05	---	---	---	---	0.00	---	---	---	---	---	0.000
0.10	9	6473.10	---	---	---	---	0.00	---	---	---	---	---	0.000
0.15	13	6473.15	---	---	---	---	0.00	---	---	---	---	---	0.000
0.20	17	6473.20	---	---	---	---	0.00	---	---	---	---	---	0.000
0.25	22	6473.25	---	---	---	---	0.00	---	---	---	---	---	0.000
0.30	26	6473.30	---	---	---	---	0.00	---	---	---	---	---	0.000
0.35	30	6473.35	---	---	---	---	0.00	---	---	---	---	---	0.000
0.40	34	6473.40	---	---	---	---	0.00	---	---	---	---	---	0.000
0.45	39	6473.45	---	---	---	---	0.00	---	---	---	---	---	0.000
0.50	43	6473.50	---	---	---	---	0.00	---	---	---	---	---	0.000
0.55	49	6473.55	---	---	---	---	0.00	---	---	---	---	---	0.000
0.60	55	6473.60	---	---	---	---	0.00	---	---	---	---	---	0.000
0.65	61	6473.65	---	---	---	---	0.00	---	---	---	---	---	0.000
0.70	67	6473.70	---	---	---	---	0.00	---	---	---	---	---	0.000
0.75	73	6473.75	---	---	---	---	0.00	---	---	---	---	---	0.000
0.80	78	6473.80	---	---	---	---	0.00	---	---	---	---	---	0.000
0.85	84	6473.85	---	---	---	---	0.00	---	---	---	---	---	0.000
0.90	90	6473.90	---	---	---	---	0.00	---	---	---	---	---	0.000
0.95	96	6473.95	---	---	---	---	0.00	---	---	---	---	---	0.000
1.00	102	6474.00	---	---	---	---	0.00	---	---	---	---	---	0.000
1.05	110	6474.05	---	---	---	---	0.00	---	---	---	---	---	0.000
1.10	117	6474.10	---	---	---	---	0.00	---	---	---	---	---	0.000
1.15	125	6474.15	---	---	---	---	0.00	---	---	---	---	---	0.000
1.20	132	6474.20	---	---	---	---	0.00	---	---	---	---	---	0.000
1.25	140	6474.25	---	---	---	---	0.00	---	---	---	---	---	0.000
1.30	147	6474.30	---	---	---	---	0.00	---	---	---	---	---	0.000
1.35	155	6474.35	---	---	---	---	0.00	---	---	---	---	---	0.000
1.40	163	6474.40	---	---	---	---	0.00	---	---	---	---	---	0.000
1.45	170	6474.45	---	---	---	---	0.00	---	---	---	---	---	0.000
1.50	178	6474.50	---	---	---	---	0.00	---	---	---	---	---	0.000
1.55	187	6474.55	---	---	---	---	0.00	---	---	---	---	---	0.000
1.60	196	6474.60	---	---	---	---	0.00	---	---	---	---	---	0.000
1.65	205	6474.65	---	---	---	---	0.00	---	---	---	---	---	0.000
1.70	215	6474.70	---	---	---	---	0.00	---	---	---	---	---	0.000
1.75	224	6474.75	---	---	---	---	0.00	---	---	---	---	---	0.000
1.80	233	6474.80	---	---	---	---	1.62	---	---	---	---	---	1.617

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outfall

Stage / Storage / Discharge Table

Stage ft	Storage cuft	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	PrfRsr cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	User cfs	Total cfs
1.85	242	6474.85	---	---	---	---	4.68	---	---	---	---	---	4.676
1.90	252	6474.90	---	---	---	---	8.53	---	---	---	---	---	8.528
1.95	261	6474.95	---	---	---	---	13.23	---	---	---	---	---	13.23
2.00	270	6475.00	---	---	---	---	18.73	---	---	---	---	---	18.73
2.10	300	6475.10	---	---	---	---	31.04	---	---	---	---	---	31.04
2.20	330	6475.20	---	---	---	---	45.26	---	---	---	---	---	45.26
2.30	360	6475.30	---	---	---	---	61.09	---	---	---	---	---	61.09
2.40	390	6475.40	---	---	---	---	78.51	---	---	---	---	---	78.51
2.50	420	6475.50	---	---	---	---	97.33	---	---	---	---	---	97.33
2.60	450	6475.60	---	---	---	---	117.65	---	---	---	---	---	117.65
2.70	480	6475.70	---	---	---	---	139.01	---	---	---	---	---	139.01
2.80	510	6475.80	---	---	---	---	161.41	---	---	---	---	---	161.41
2.90	540	6475.90	---	---	---	---	185.01	---	---	---	---	---	185.01
3.00	570	6476.00	---	---	---	---	209.42	---	---	---	---	---	209.42

...End

Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	3.851	2	722	11,022	-----	-----	-----	QD2A
2	Reach	3.869	2	724	11,021	1	-----	-----	FLOW FROM QD2A
3	SCS Runoff	1.822	2	724	5,729	-----	-----	-----	QD2B
4	Combine	5.691	2	724	16,750	2, 3	-----	-----	FLOW FROM QD2B
5	Reach	5.712	2	724	16,749	4	-----	-----	FLOW FROM QD2B
6	SCS Runoff	1.896	2	718	4,629	-----	-----	-----	QD2C
7	Combine	6.978	2	722	21,378	5, 6	-----	-----	FLOW FROM QD2C
8	Reach	7.017	2	724	21,378	7	-----	-----	FLOW FROM QD2C
9	SCS Runoff	3.440	2	724	10,928	-----	-----	-----	QD2D
10	Combine	10.46	2	724	32,306	8, 9	-----	-----	FLOW FROM QD2D
11	Reach	10.47	2	726	32,305	10	-----	-----	FLOW FROM QD2D
12	SCS Runoff	2.854	2	720	7,475	-----	-----	-----	QD2E
13	Reach	2.877	2	722	7,473	12	-----	-----	FLOW FROM QD2E
14	SCS Runoff	1.757	2	716	3,624	-----	-----	-----	QD2F
15	Combine	4.108	2	718	11,097	13, 14	-----	-----	FLOW FROM QD2F
16	Reach	4.196	2	720	11,097	15	-----	-----	FLOW FROM QD2F
18	SCS Runoff	2.066	2	720	5,410	-----	-----	-----	QD2G
19	Combine	15.83	2	722	48,813	11, 16, 18	-----	-----	FLOW FROM QD2G
20	Reach	16.03	2	724	48,815	19	-----	-----	FLOW FROM QD2G
21	SCS Runoff	1.631	2	720	4,271	-----	-----	-----	QD2J
22	Combine	17.47	2	724	53,086	20, 21	-----	-----	FLOW FROM QD2J
23	Reach	17.51	2	724	53,091	22	-----	-----	FLOW FROM QD2J
24	SCS Runoff	2.446	2	720	6,407	-----	-----	-----	QD2M
25	Combine	19.67	2	724	59,497	23, 24	-----	-----	FLOW FROM QD2M
26	Reach	19.81	2	726	59,501	25	-----	-----	FLOW FROM QD2M
27	SCS Runoff	1.992	2	718	4,625	-----	-----	-----	QD2O
28	Combine	20.72	2	726	64,126	26, 27	-----	-----	FLOW FROM QD2O
29	Reach	21.36	2	728	64,321	28	-----	-----	FLOW TO POND FROM WEST PIPE
31	SCS Runoff	1.906	2	726	6,605	-----	-----	-----	QD2P
32	Reach	1.915	2	728	6,601	31	-----	-----	FLOW FROM QD2P
33	SCS Runoff	0.118	2	716	273	-----	-----	-----	QD2H
34	Combine	1.930	2	728	6,875	32, 33	-----	-----	FLOW FROM QD2H
35	Reach	1.903	2	730	6,874	34	-----	-----	FLOW FROM QD2H
2024-06-21 PROPOSED.gpw					Return Period: 50 Year			Monday, 06 / 24 / 2024	

Hydrograph Summary Report

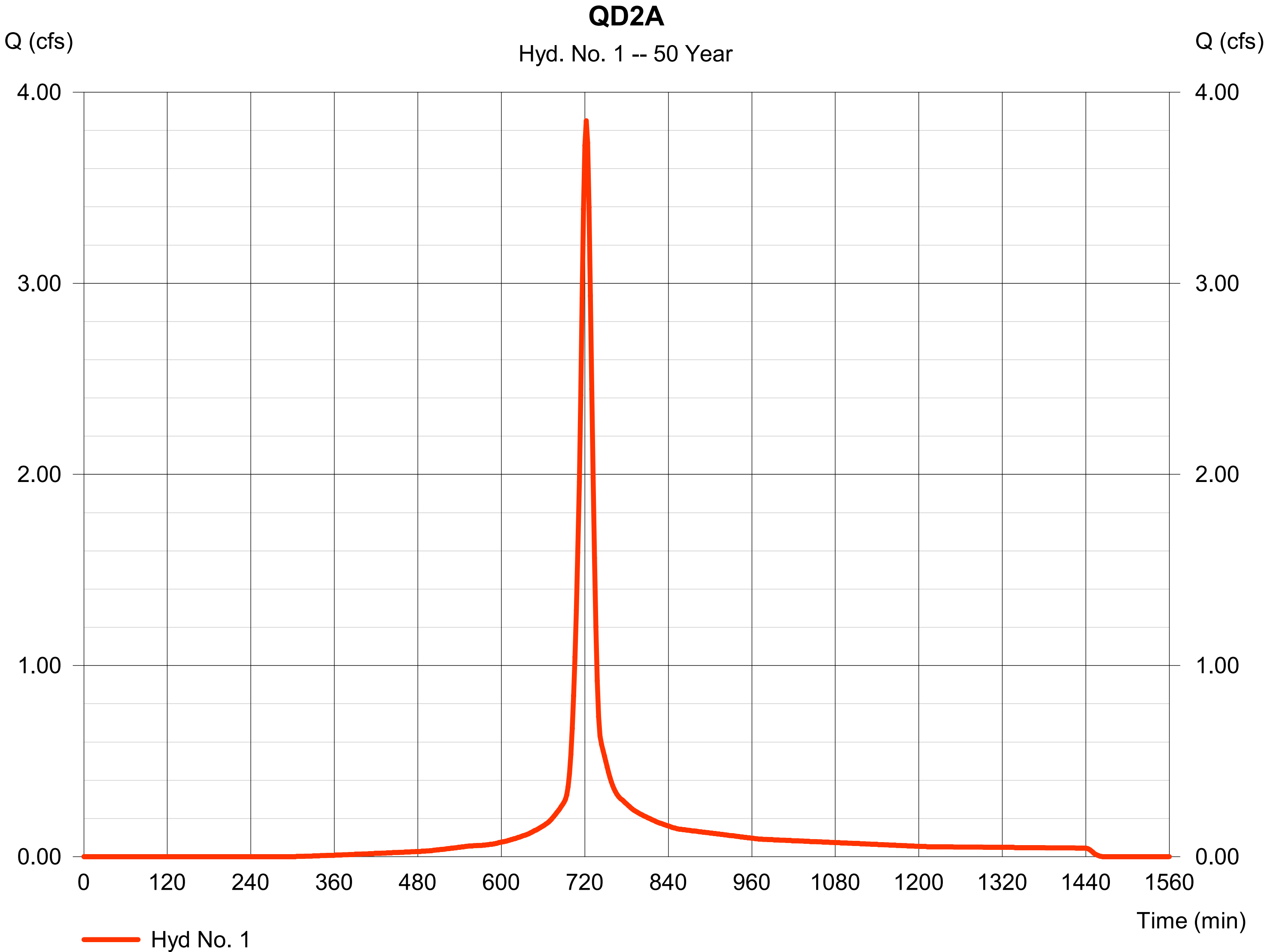
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
36	SCS Runoff	0.761	2	720	1,993	-----	-----	-----	QD2I
37	Combine	2.306	2	726	8,867	35, 36	-----	-----	FLOW FROM QD2I
38	Reach	2.306	2	728	8,867	37	-----	-----	FLOW FROM QD2I
40	SCS Runoff	2.481	2	724	7,800	-----	-----	-----	QD2Q
41	Reach	2.494	2	726	7,798	40	-----	-----	FLOW FROM QD2Q
42	SCS Runoff	2.338	2	720	6,122	-----	-----	-----	QD2R
43	Combine	4.593	2	722	13,920	41, 42	-----	-----	FLOW FROM QD2R
44	Reach	4.621	2	724	13,919	43	-----	-----	FLOW FROM QD2R
46	SCS Runoff	0.405	2	722	1,144	-----	-----	-----	QD2K
47	Combine	7.186	2	724	23,931	38, 44, 46	-----	-----	FLOW FROM QD2K
48	Reach	7.219	2	726	23,931	47	-----	-----	FLOW FROM QD2K
49	SCS Runoff	0.714	2	722	2,019	-----	-----	-----	QD2L
50	Combine	7.853	2	726	25,951	48, 49	-----	-----	FLOW FROM QD2L
51	Reach	7.852	2	728	25,950	50	-----	-----	FLOW FROM QD2L
53	SCS Runoff	2.656	2	726	9,204	-----	-----	-----	QD2S
54	Reach	2.670	2	728	9,203	53	-----	-----	FLOW FROM QD2S
55	SCS Runoff	6.017	2	728	22,229	-----	-----	-----	QD2T
56	Combine	8.687	2	728	31,431	54, 55	-----	-----	FLOW FROM QD2T
57	Reach	8.703	2	730	31,431	56	-----	-----	FLOW FROM QD2T
58	Combine	16.52	2	728	57,382	51, 57	-----	-----	FLOW AT MANHOLE
59	Reach	16.57	2	730	57,385	58	-----	-----	FLOW FROM MANHOLE
60	SCS Runoff	0.340	2	716	755	-----	-----	-----	QD2N
61	Combine	16.61	2	730	58,140	59, 60	-----	-----	FLOW FROM QD2N
62	Reach	16.69	2	730	58,149	61	-----	-----	FLOW TO POND FROM EAST PIPE
64	SCS Runoff	3.847	2	730	15,368	-----	-----	-----	QD2X FLOW FROM PARK AREA
65	SCS Runoff	1.166	2	722	3,304	-----	-----	-----	QD3
67	Combine	42.27	2	728	141,142	29, 62, 64, 65,	-----	-----	TOTAL FLOW TO POND
69	Reservoir(i)	26.68	2	752	104,632	67	6476.25	23,586	Stage 3
2024-06-21 PROPOSED.gpw					Return Period: 50 Year			Monday, 06 / 24 / 2024	

Hyd. No. 1

QD2A

Hydrograph type	= SCS Runoff	Peak discharge	= 3.851 cfs
Storm frequency	= 50 yrs	Time to peak	= 722 min
Time interval	= 2 min	Hyd. volume	= 11,022 cuft
Drainage area	= 1.500 ac	Curve number	= 92
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 14.00 min
Total precip.	= 2.91 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

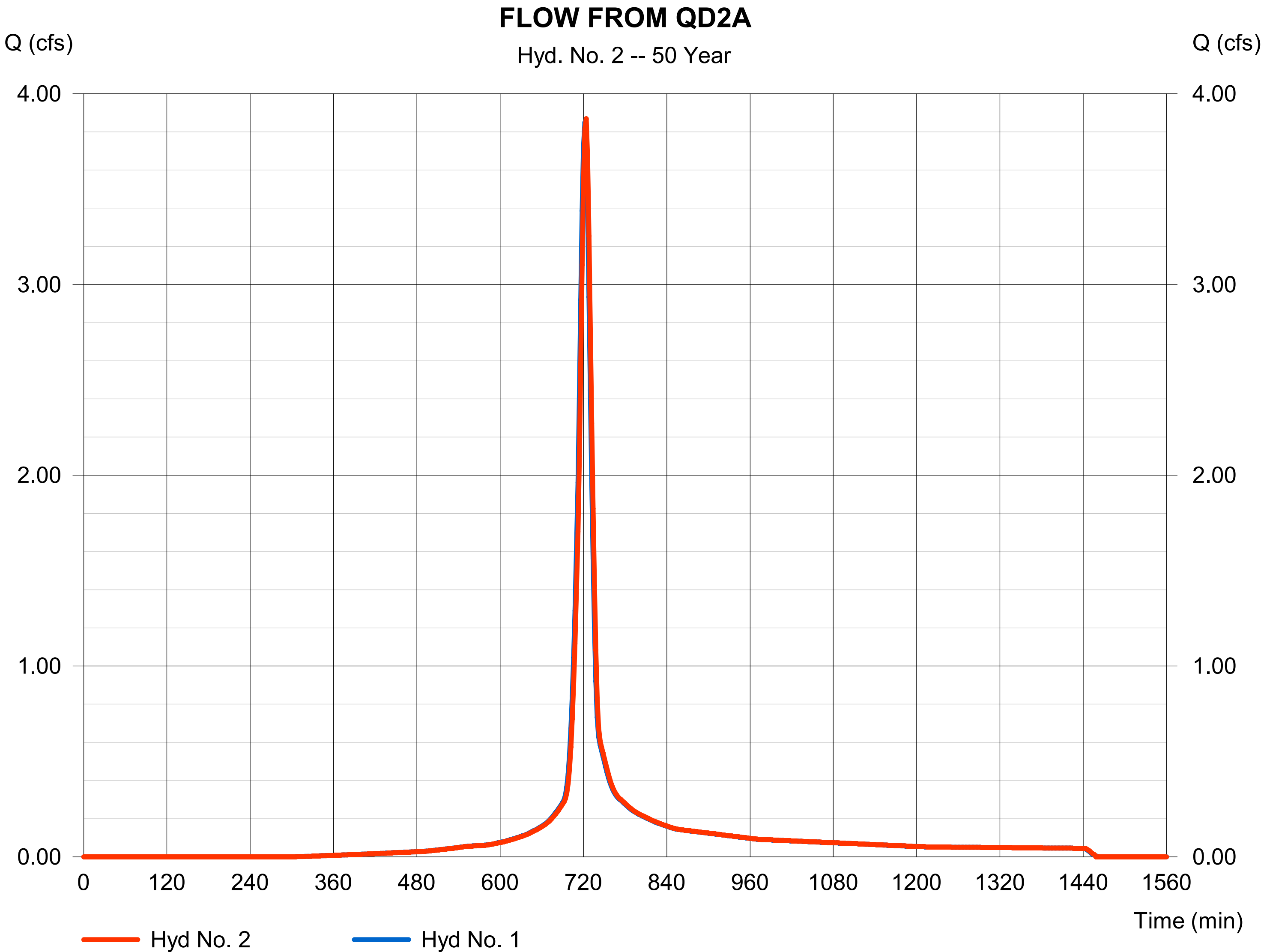


Hyd. No. 2

FLOW FROM QD2A

Hydrograph type	= Reach	Peak discharge	= 3.869 cfs
Storm frequency	= 50 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 11,021 cuft
Inflow hyd. No.	= 1 - QD2A	Section type	= Circular
Reach length	= 47.7 ft	Channel slope	= 0.9 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 3.438	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.5478

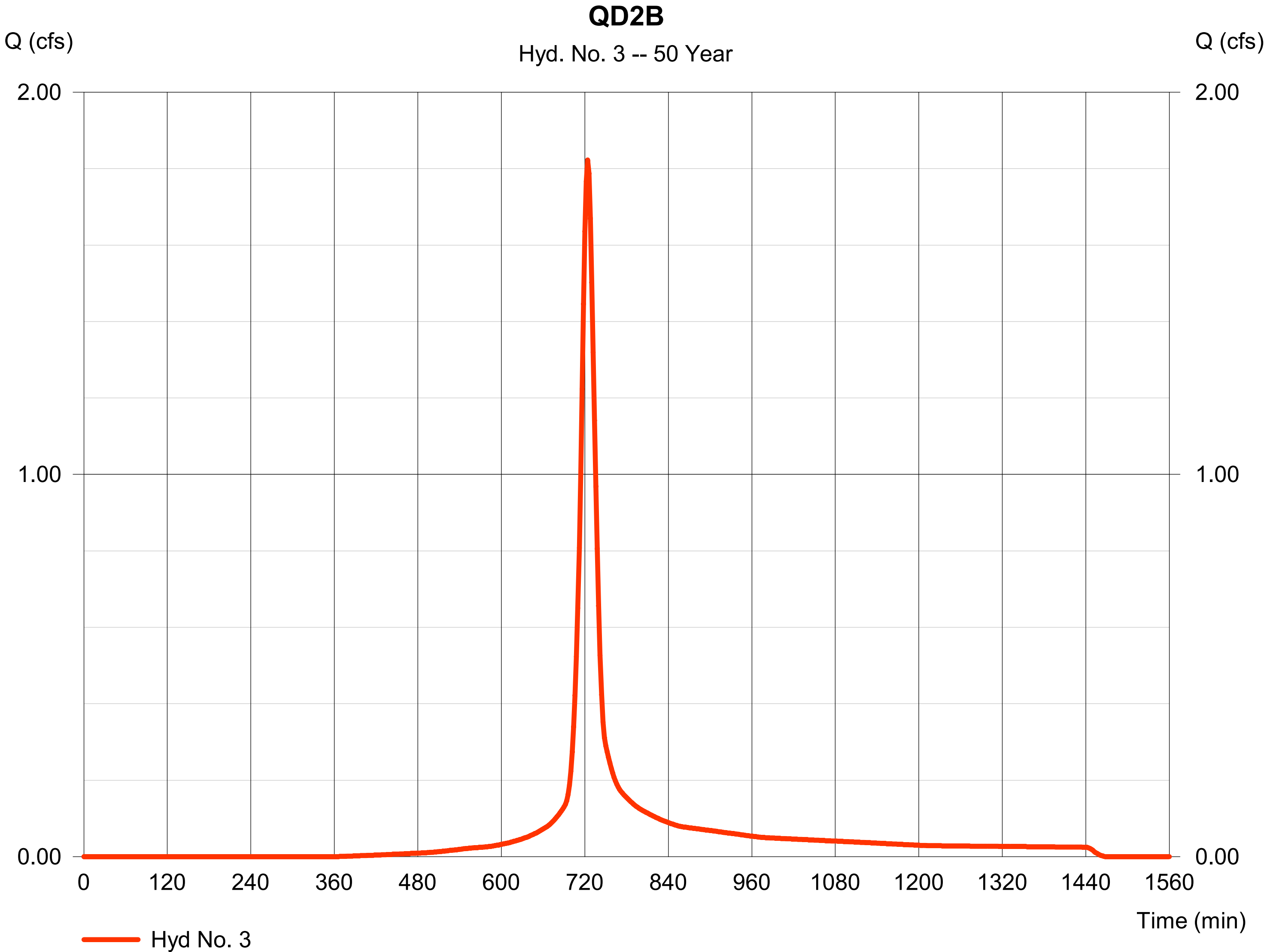
Modified Att-Kin routing method used.



Hyd. No. 3

QD2B

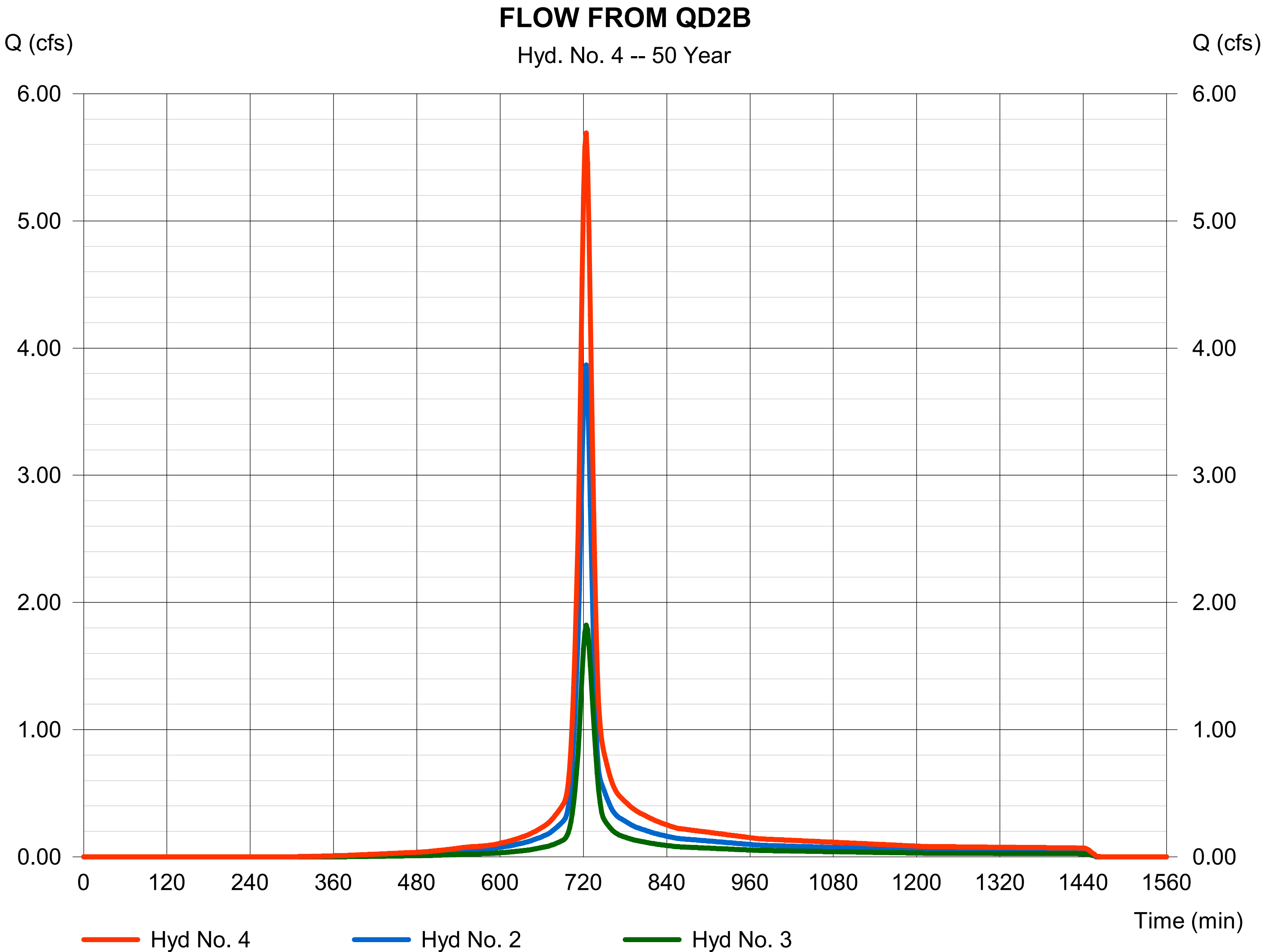
Hydrograph type	= SCS Runoff	Peak discharge	= 1.822 cfs
Storm frequency	= 50 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 5,729 cuft
Drainage area	= 0.830 ac	Curve number	= 90
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 16.90 min
Total precip.	= 2.91 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hyd. No. 4

FLOW FROM QD2B

Hydrograph type	= Combine	Peak discharge	= 5.691 cfs
Storm frequency	= 50 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 16,750 cuft
Inflow hyds.	= 2, 3	Contrib. drain. area	= 0.830 ac

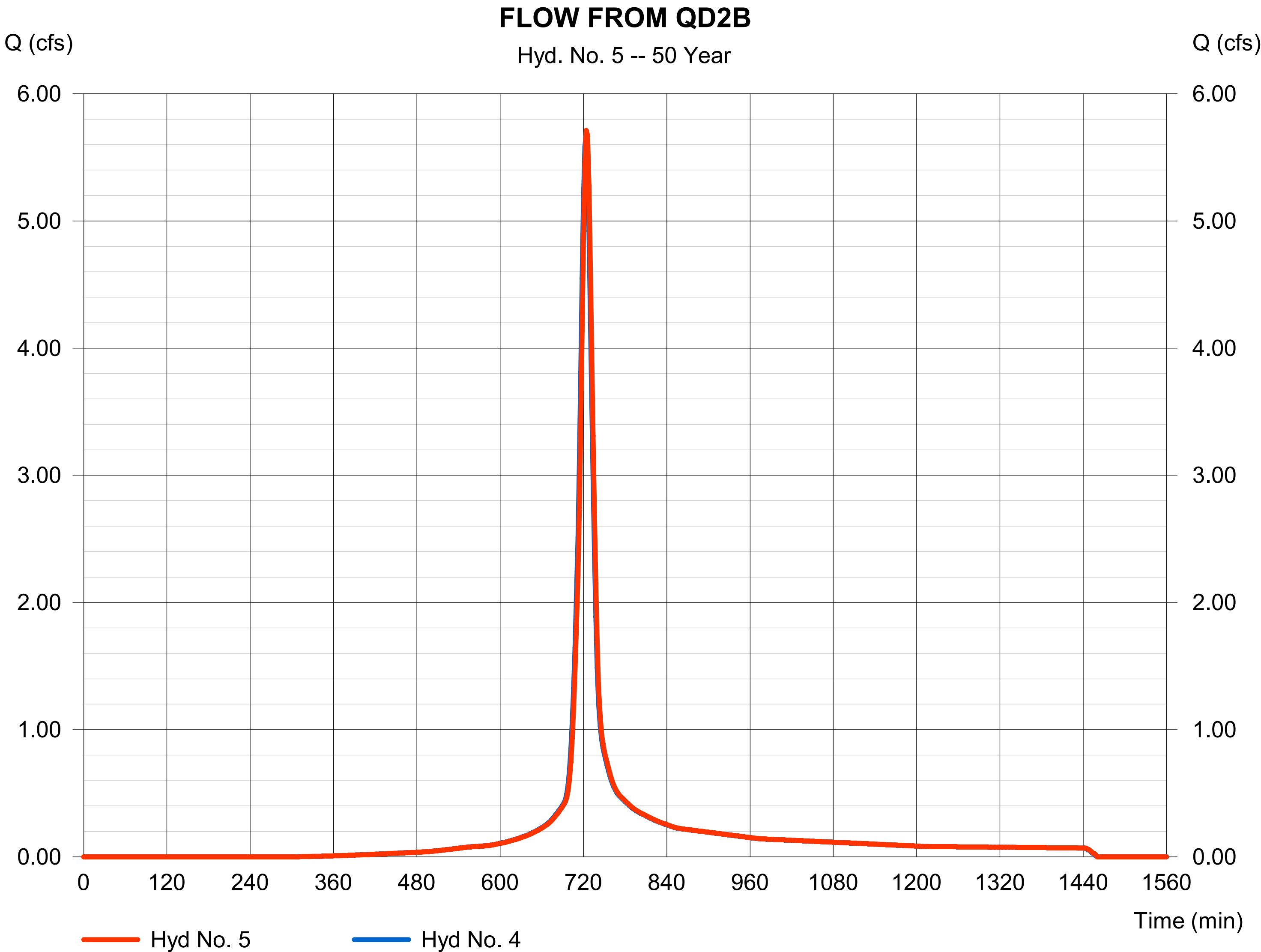


Hyd. No. 5

FLOW FROM QD2B

Hydrograph type	= Reach	Peak discharge	= 5.712 cfs
Storm frequency	= 50 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 16,749 cuft
Inflow hyd. No.	= 4 - FLOW FROM QD2B	Section type	= Circular
Reach length	= 32.2 ft	Channel slope	= 0.9 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 3.438	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.7649

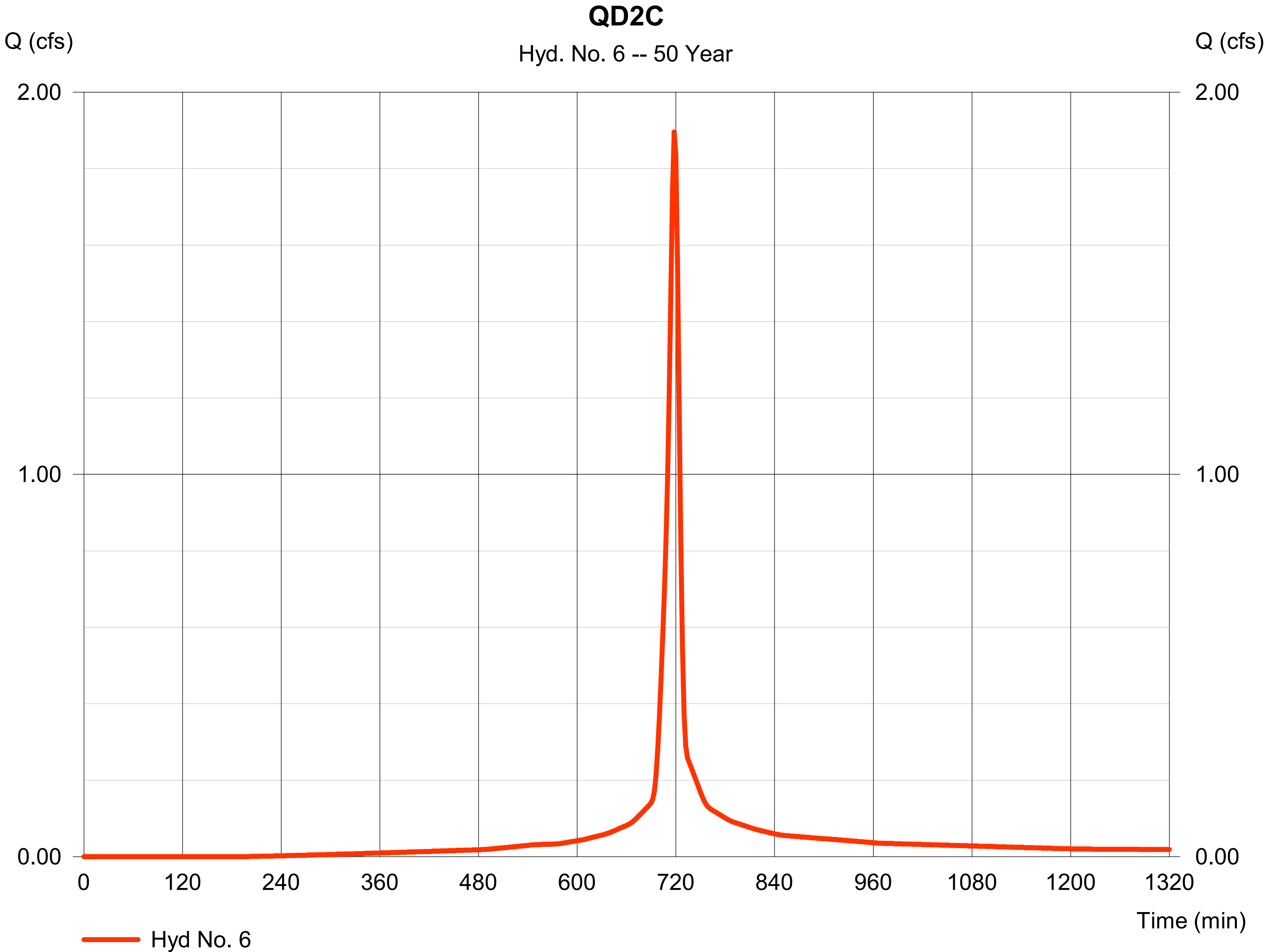
Modified Att-Kin routing method used.



Hyd. No. 6

QD2C

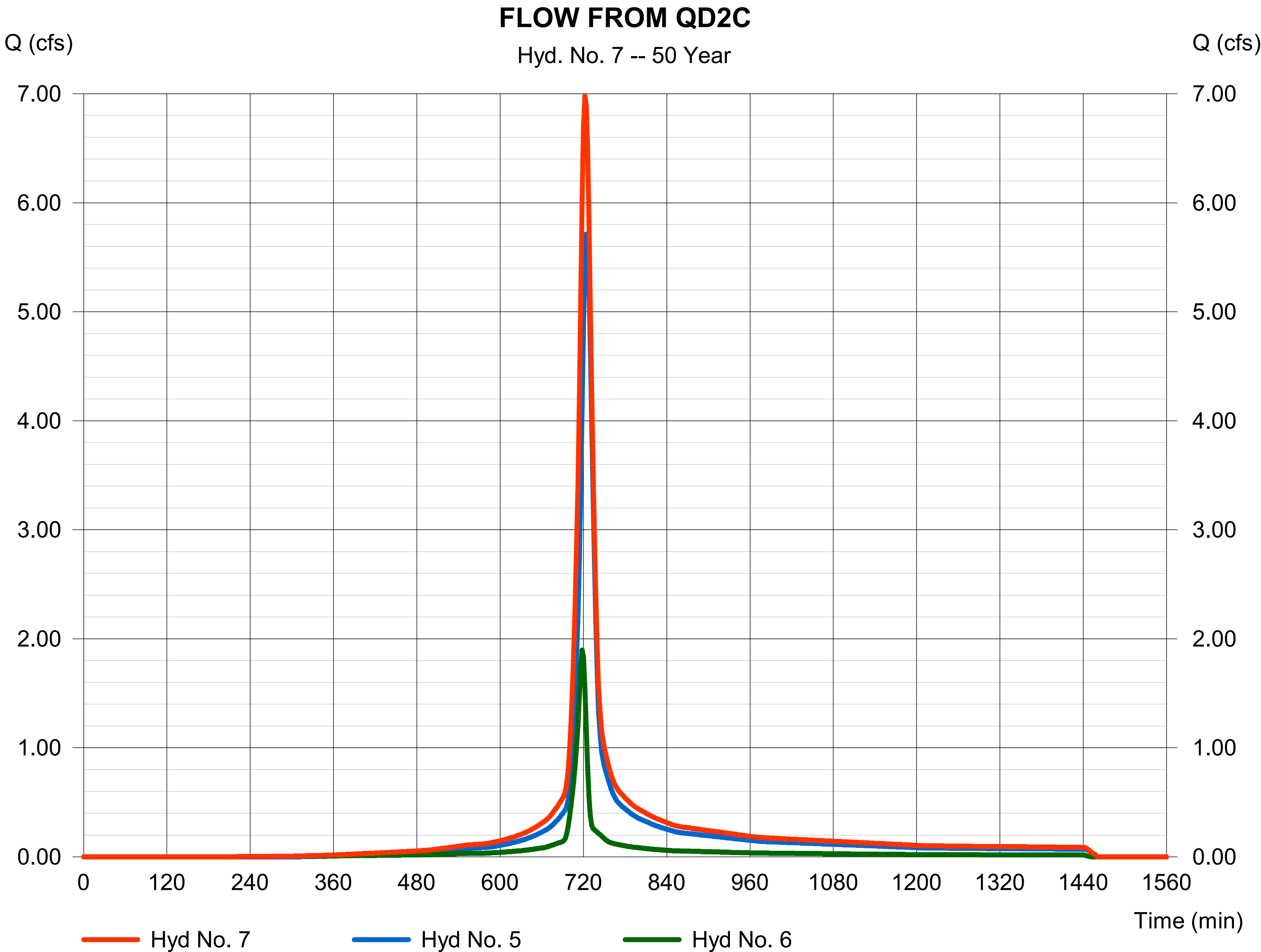
Hydrograph type	= SCS Runoff	Peak discharge	= 1.896 cfs
Storm frequency	= 50 yrs	Time to peak	= 718 min
Time interval	= 2 min	Hyd. volume	= 4,629 cuft
Drainage area	= 0.540 ac	Curve number	= 95
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 9.50 min
Total precip.	= 2.91 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hyd. No. 7

FLOW FROM QD2C

Hydrograph type	= Combine	Peak discharge	= 6.978 cfs
Storm frequency	= 50 yrs	Time to peak	= 722 min
Time interval	= 2 min	Hyd. volume	= 21,378 cuft
Inflow hyds.	= 5, 6	Contrib. drain. area	= 0.540 ac

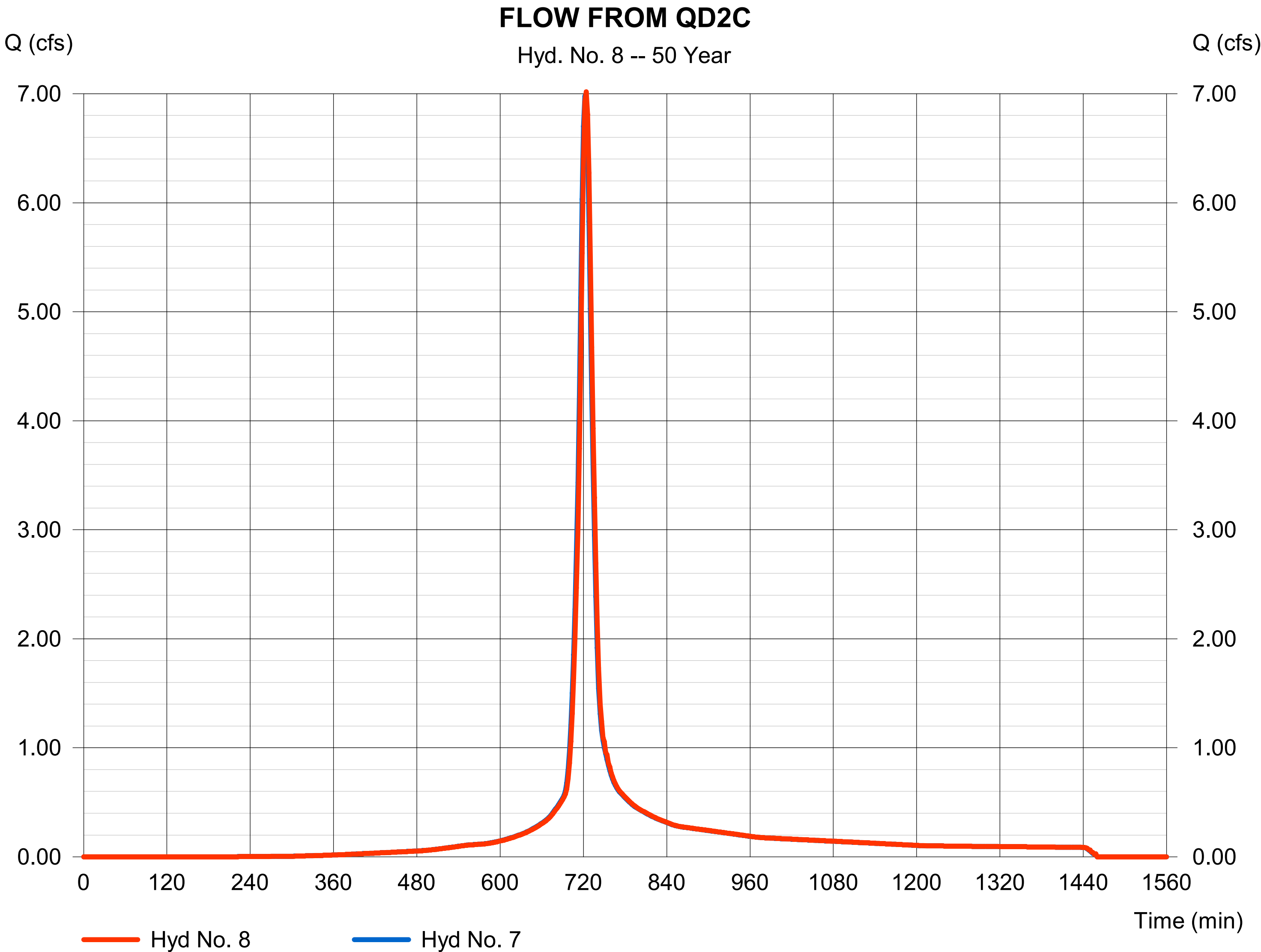


Hyd. No. 8

FLOW FROM QD2C

Hydrograph type	= Reach	Peak discharge	= 7.017 cfs
Storm frequency	= 50 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 21,378 cuft
Inflow hyd. No.	= 7 - FLOW FROM QD2C	Section type	= Circular
Reach length	= 41.0 ft	Channel slope	= 0.9 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 3.438	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.7571

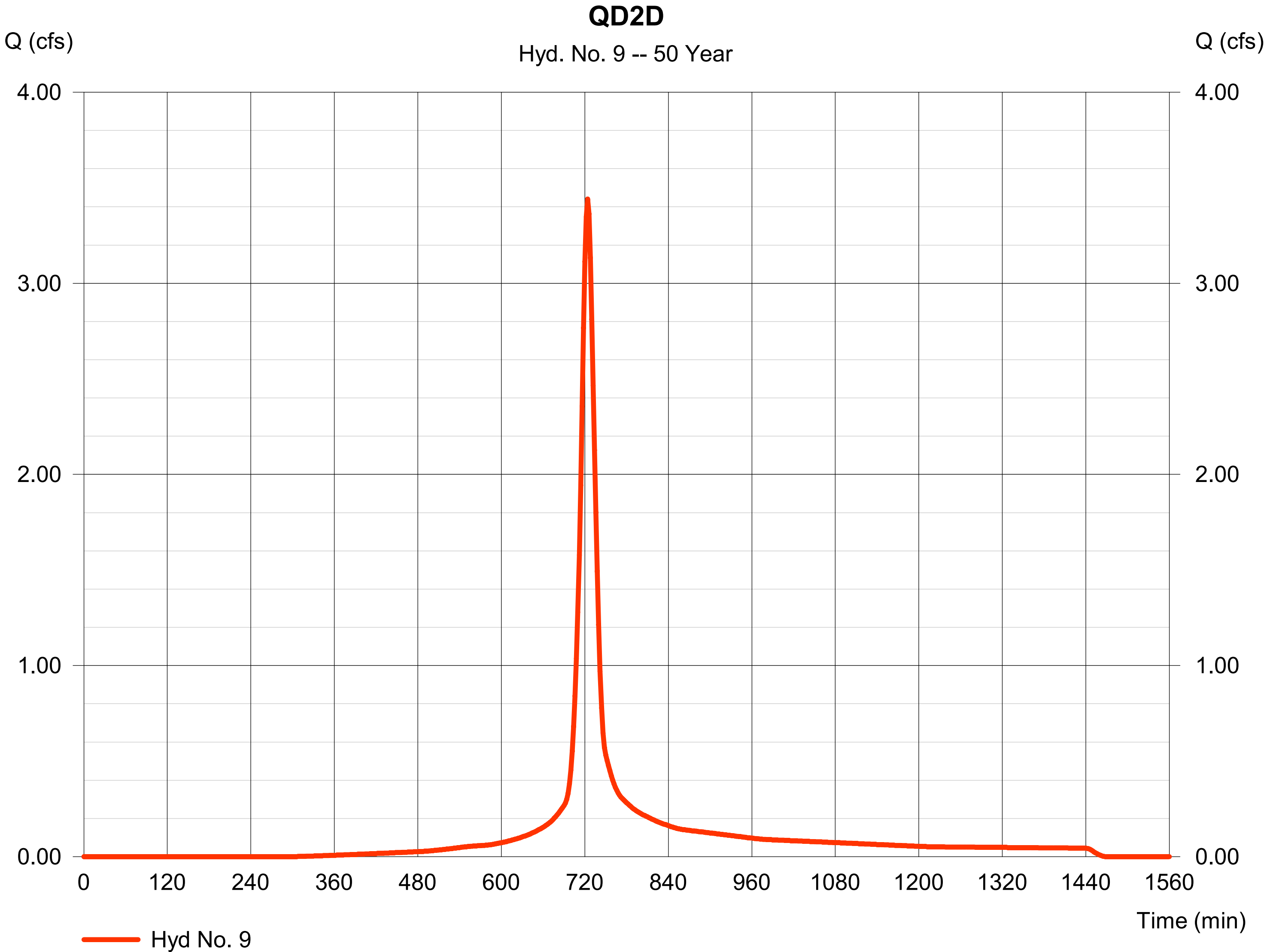
Modified Att-Kin routing method used.



Hyd. No. 9

QD2D

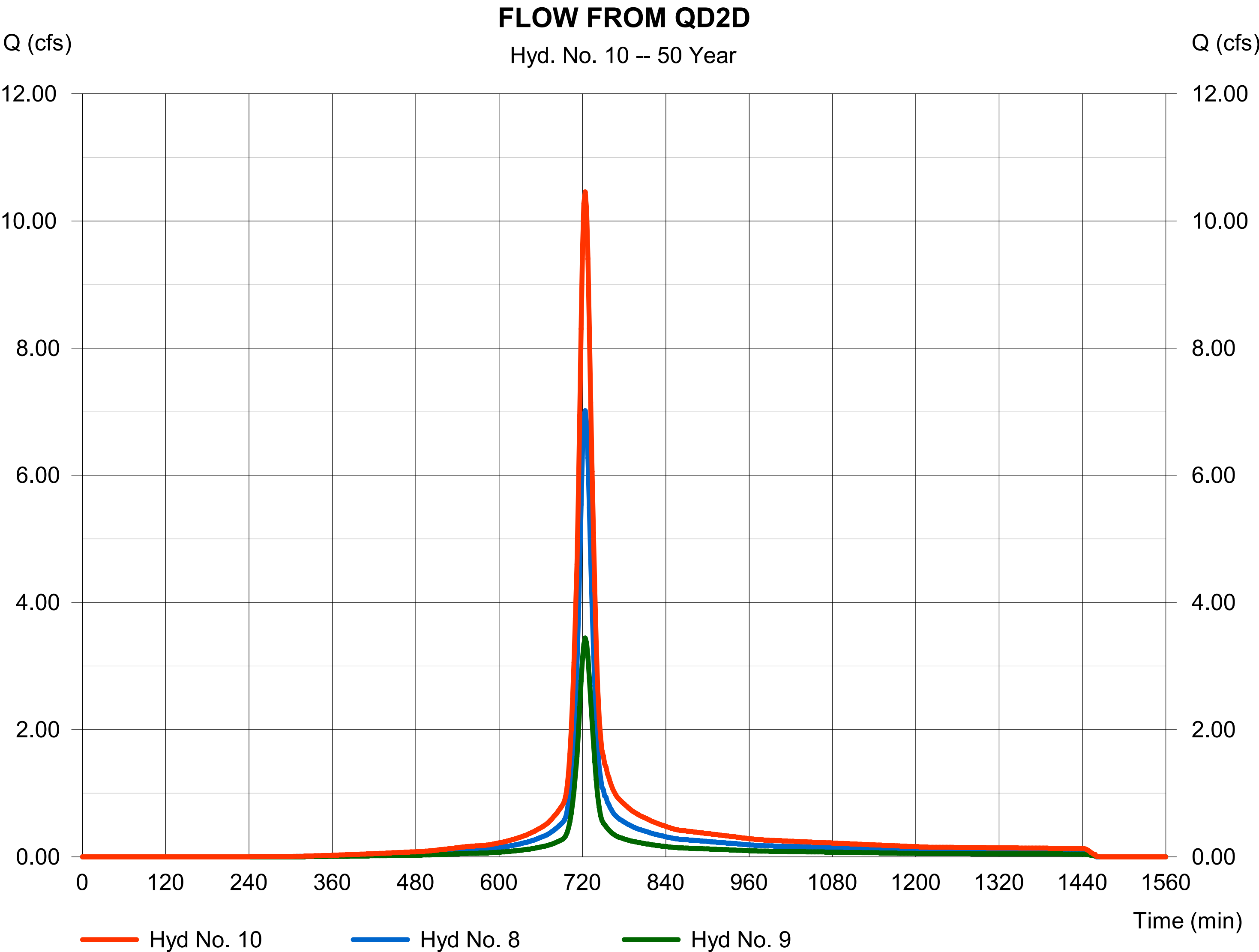
Hydrograph type	=	SCS Runoff	Peak discharge	=	3.440 cfs
Storm frequency	=	50 yrs	Time to peak	=	724 min
Time interval	=	2 min	Hyd. volume	=	10,928 cuft
Drainage area	=	1.450 ac	Curve number	=	92
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	18.60 min
Total precip.	=	2.91 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hyd. No. 10

FLOW FROM QD2D

Hydrograph type	= Combine	Peak discharge	= 10.46 cfs
Storm frequency	= 50 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 32,306 cuft
Inflow hyds.	= 8, 9	Contrib. drain. area	= 1.450 ac

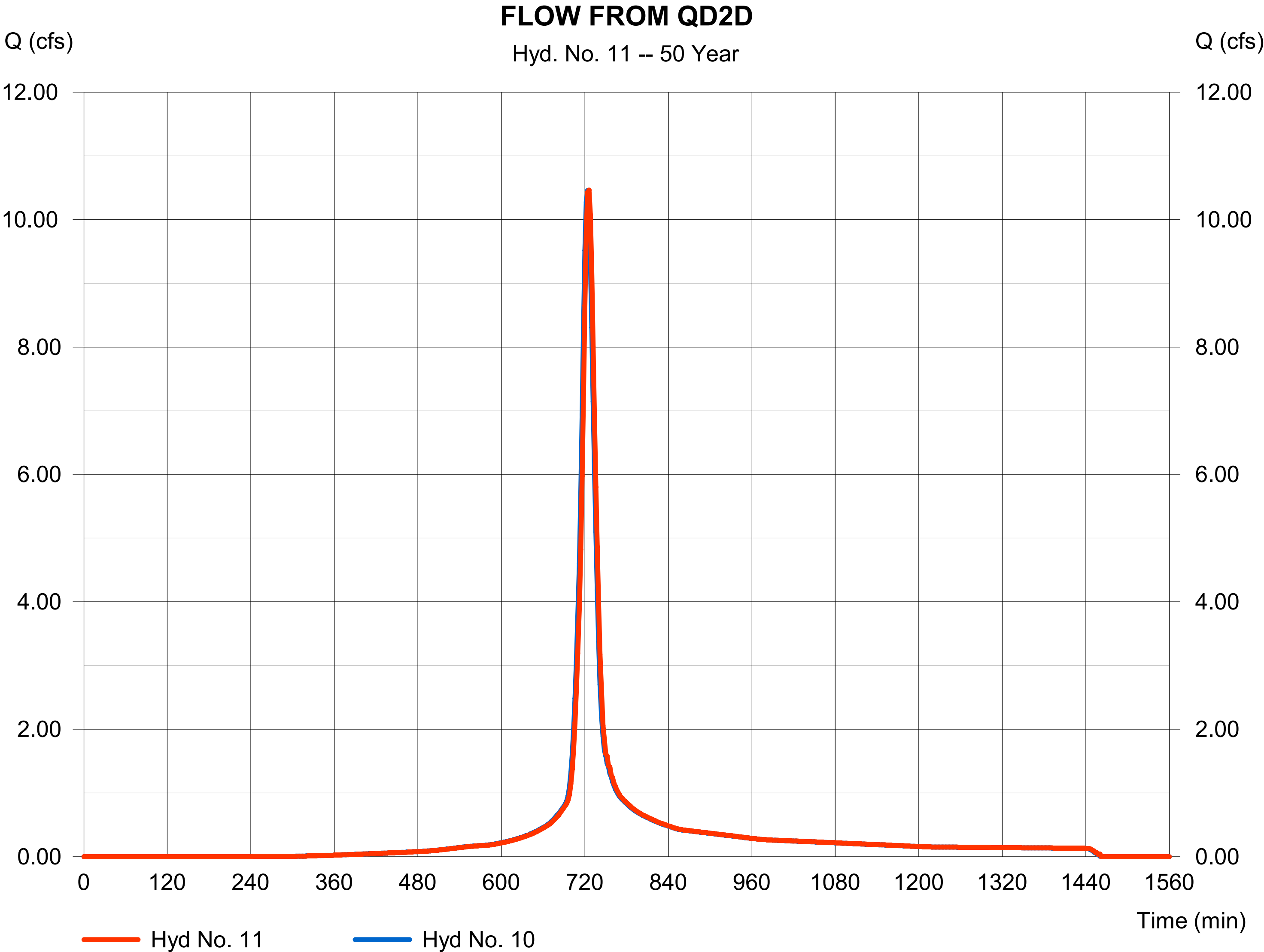


Hyd. No. 11

FLOW FROM QD2D

Hydrograph type	= Reach	Peak discharge	= 10.47 cfs
Storm frequency	= 50 yrs	Time to peak	= 726 min
Time interval	= 2 min	Hyd. volume	= 32,305 cuft
Inflow hyd. No.	= 10 - FLOW FROM QD2D	Section type	= Circular
Reach length	= 225.0 ft	Channel slope	= 0.9 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 3.438	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.3272

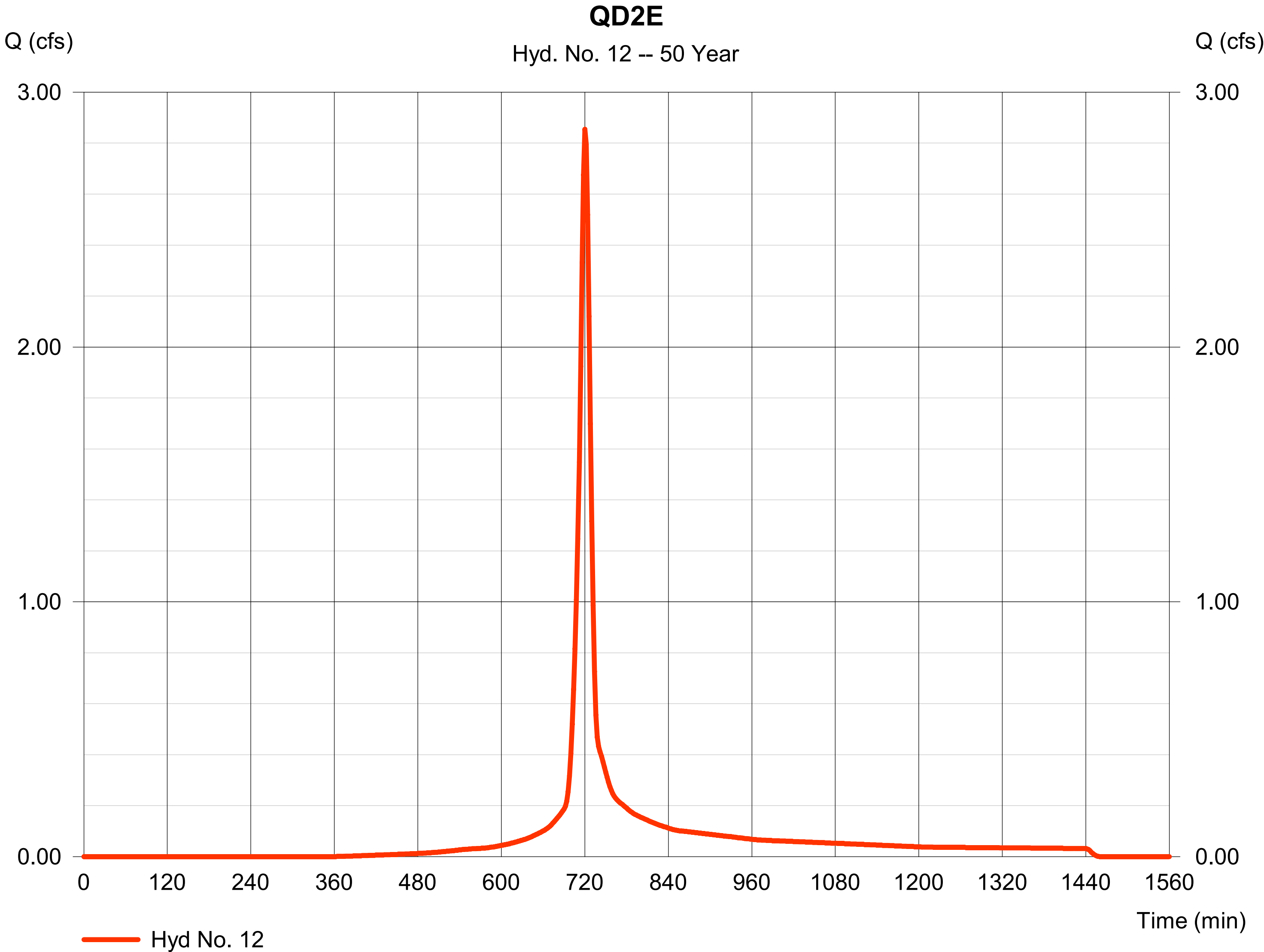
Modified Att-Kin routing method used.



Hyd. No. 12

QD2E

Hydrograph type	= SCS Runoff	Peak discharge	= 2.854 cfs
Storm frequency	= 50 yrs	Time to peak	= 720 min
Time interval	= 2 min	Hyd. volume	= 7,475 cuft
Drainage area	= 1.050 ac	Curve number	= 90
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 11.90 min
Total precip.	= 2.91 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

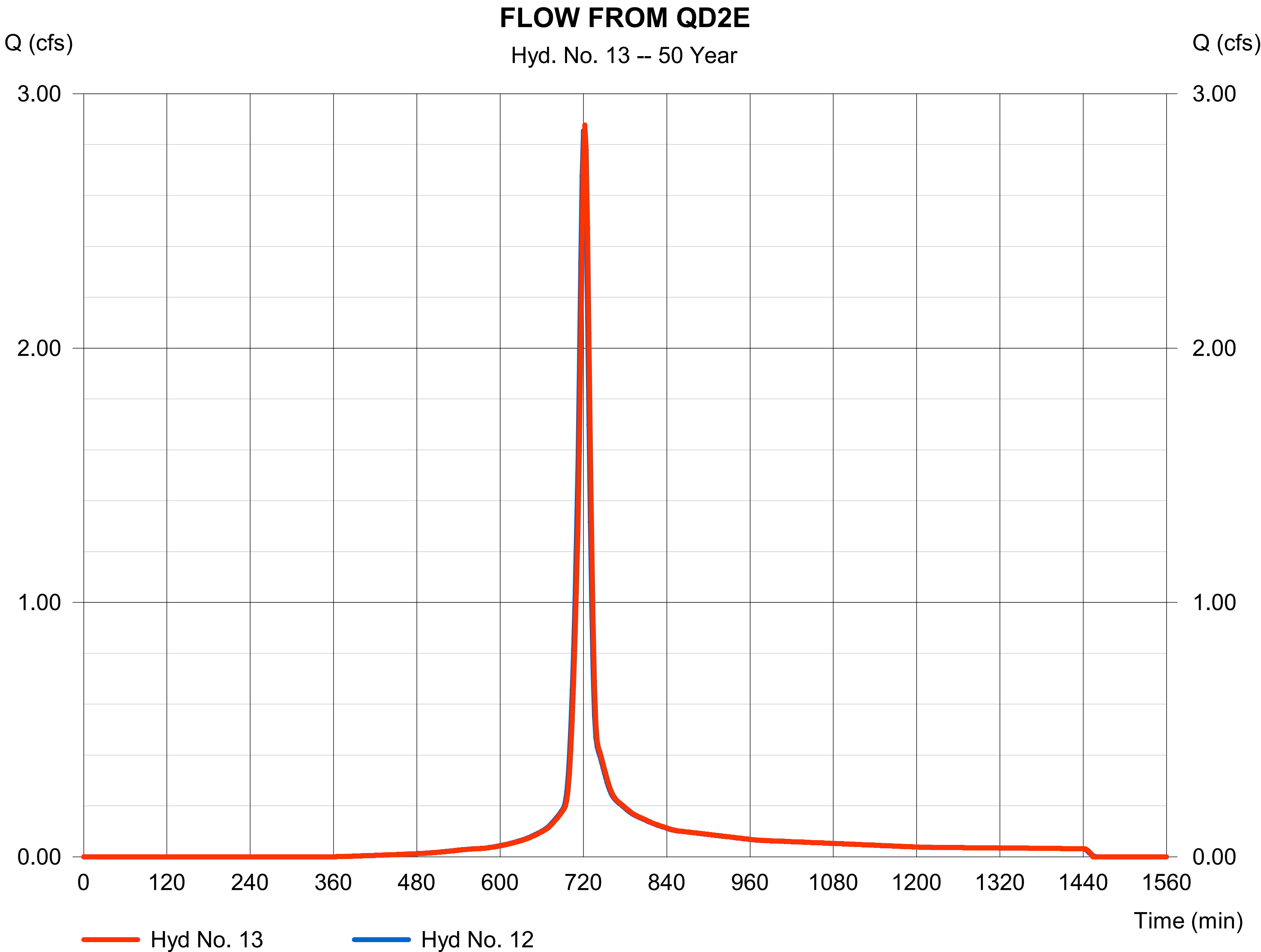


Hyd. No. 13

FLOW FROM QD2E

Hydrograph type	= Reach	Peak discharge	= 2.877 cfs
Storm frequency	= 50 yrs	Time to peak	= 722 min
Time interval	= 2 min	Hyd. volume	= 7,473 cuft
Inflow hyd. No.	= 12 - QD2E	Section type	= Circular
Reach length	= 84.5 ft	Channel slope	= 0.5 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.534	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.1782

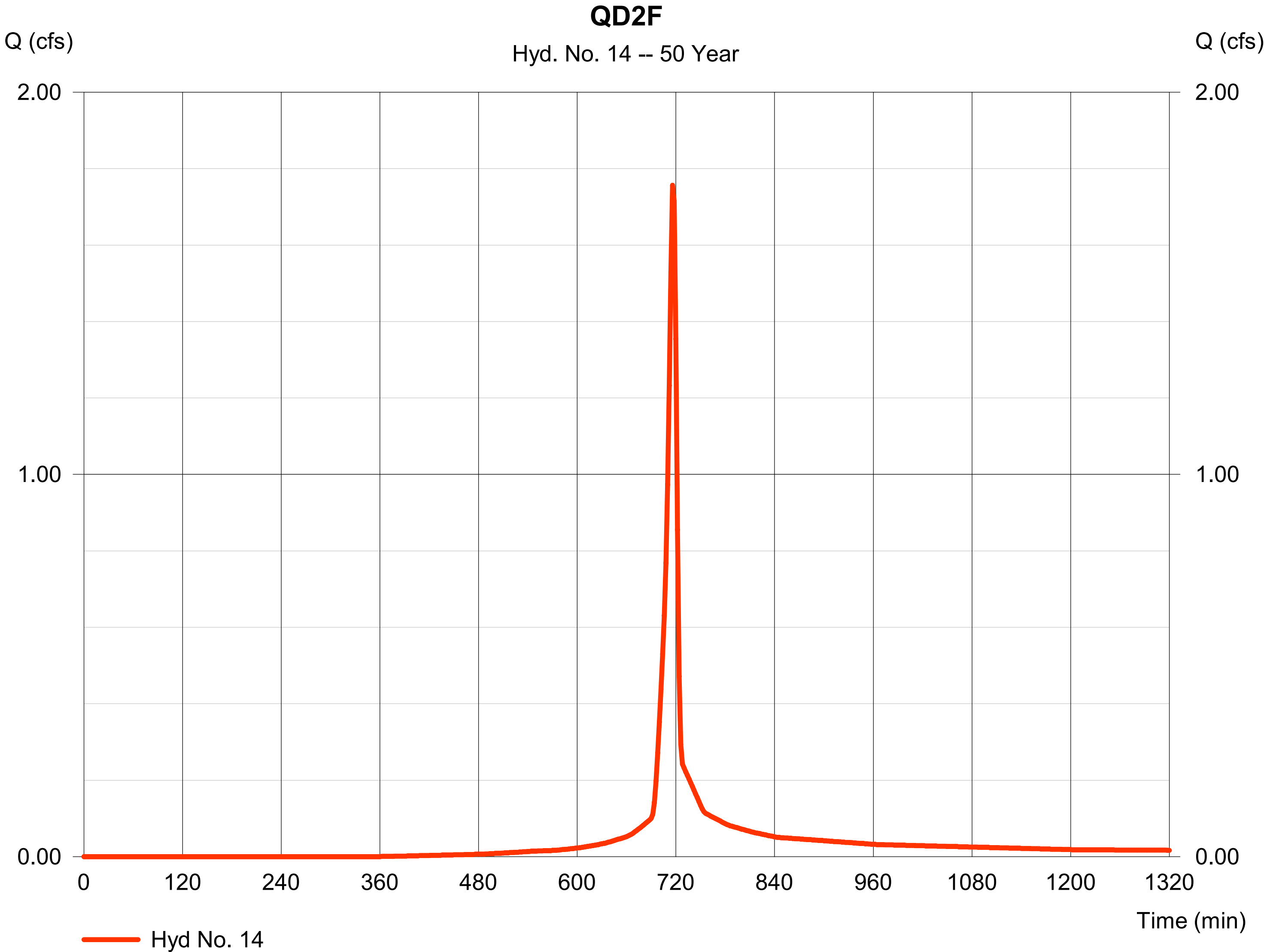
Modified Att-Kin routing method used.



Hyd. No. 14

QD2F

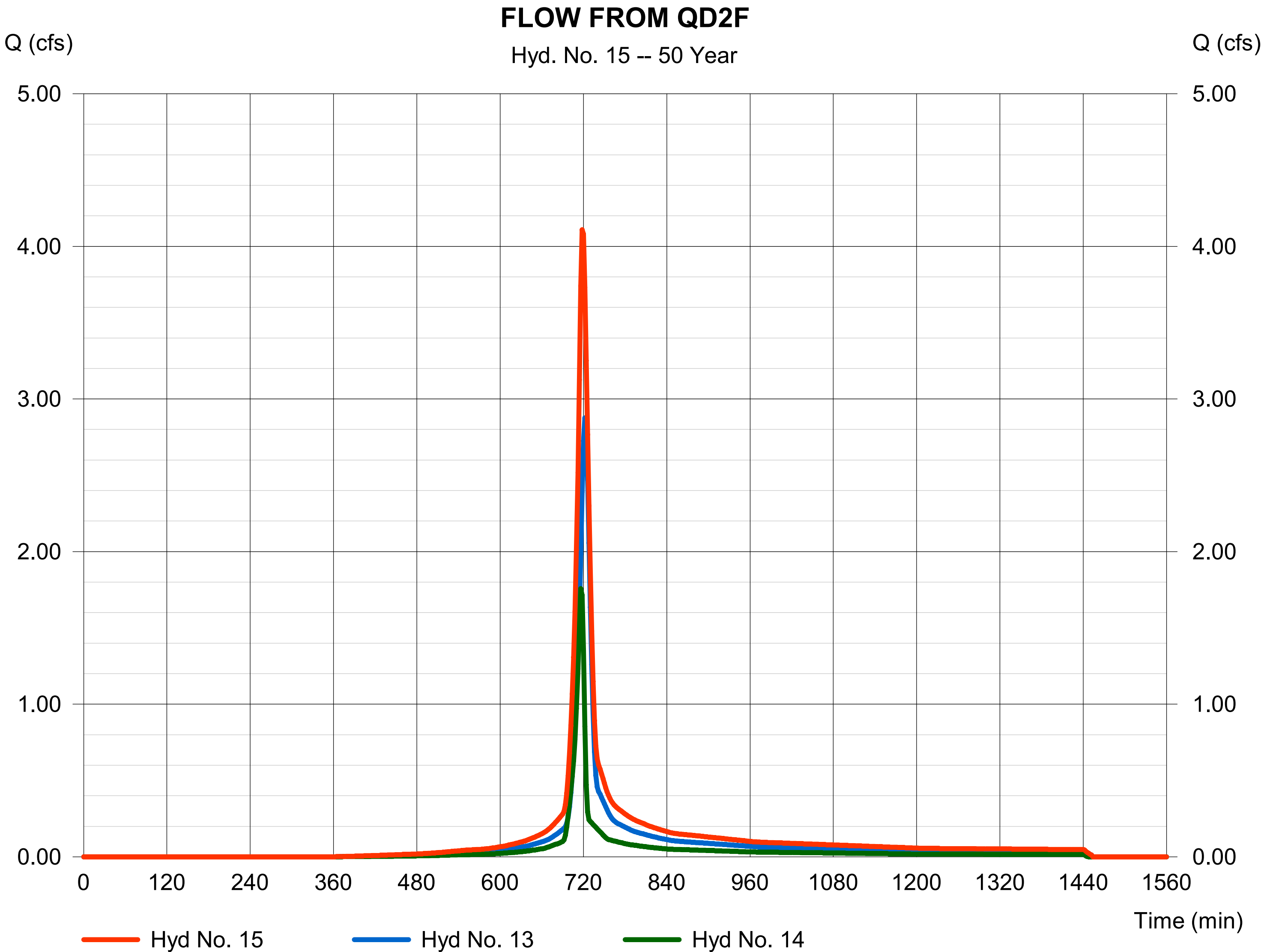
Hydrograph type	= SCS Runoff	Peak discharge	= 1.757 cfs
Storm frequency	= 50 yrs	Time to peak	= 716 min
Time interval	= 2 min	Hyd. volume	= 3,624 cuft
Drainage area	= 0.560 ac	Curve number	= 90
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 6.00 min
Total precip.	= 2.91 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hyd. No. 15

FLOW FROM QD2F

Hydrograph type	= Combine	Peak discharge	= 4.108 cfs
Storm frequency	= 50 yrs	Time to peak	= 718 min
Time interval	= 2 min	Hyd. volume	= 11,097 cuft
Inflow hyds.	= 13, 14	Contrib. drain. area	= 0.560 ac

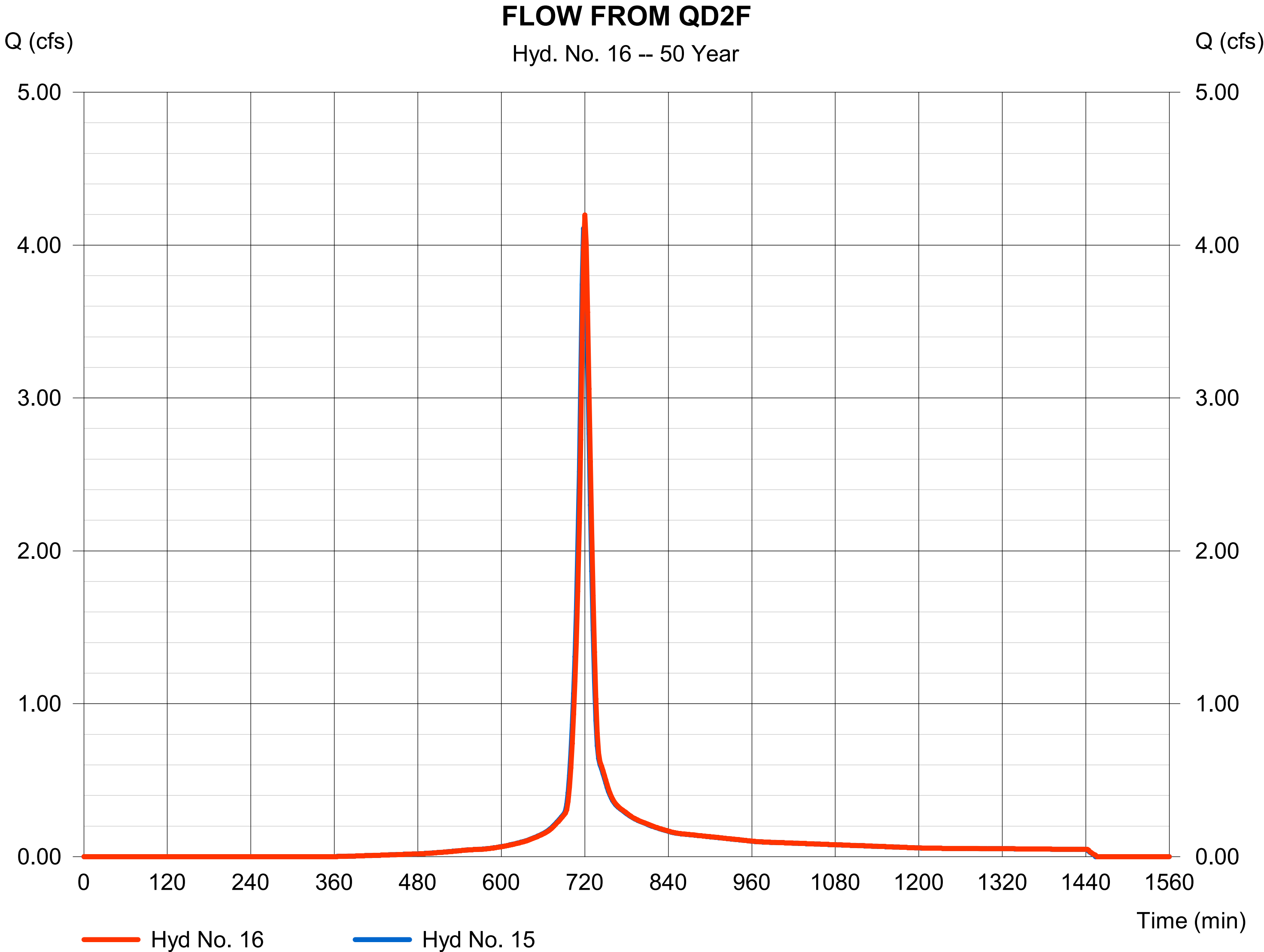


Hyd. No. 16

FLOW FROM QD2F

Hydrograph type	= Reach	Peak discharge	= 4.196 cfs
Storm frequency	= 50 yrs	Time to peak	= 720 min
Time interval	= 2 min	Hyd. volume	= 11,097 cuft
Inflow hyd. No.	= 15 - FLOW FROM QD2F	Section type	= Circular
Reach length	= 40.2 ft	Channel slope	= 0.5 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.534	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.6255

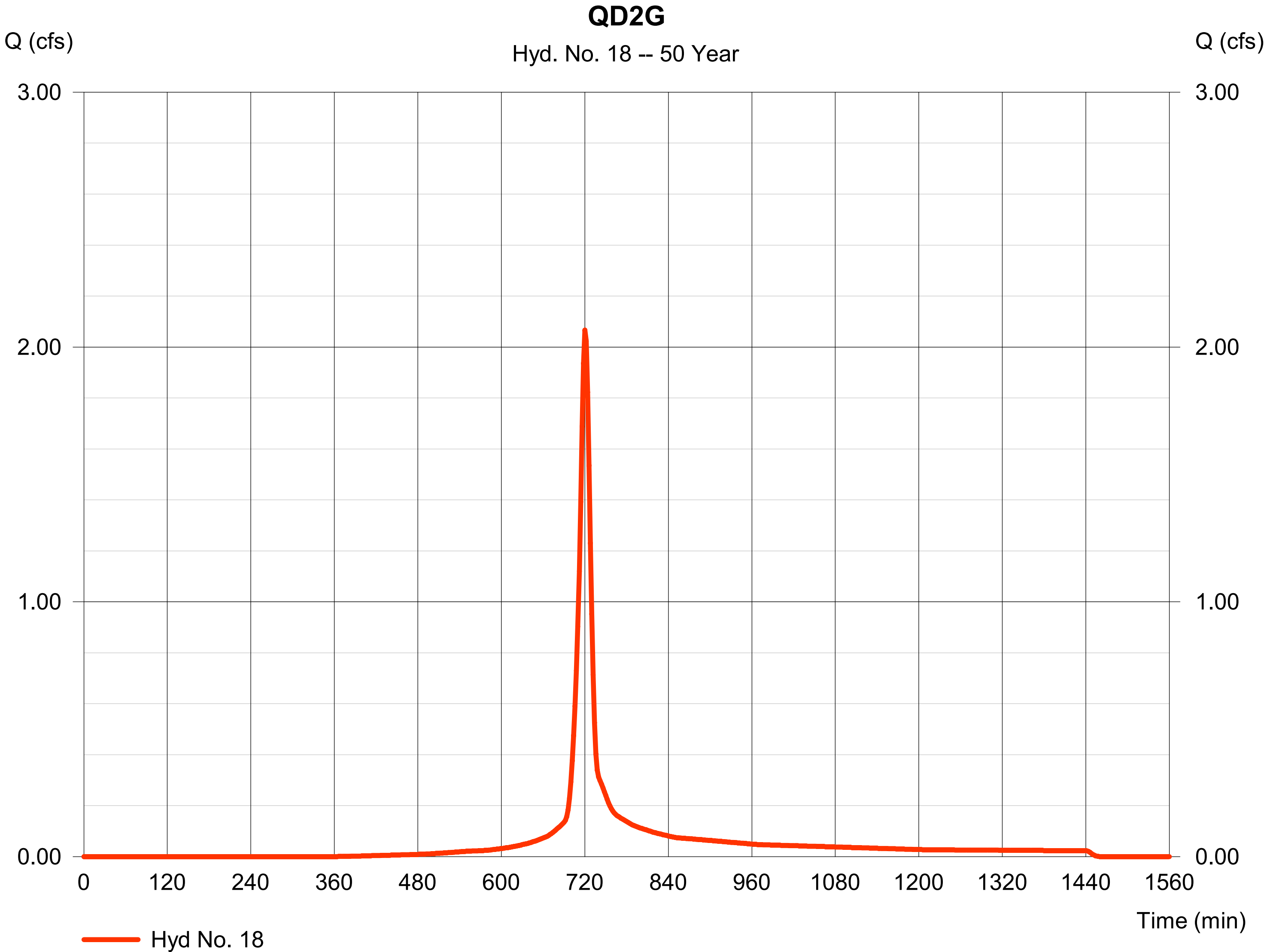
Modified Att-Kin routing method used.



Hyd. No. 18

QD2G

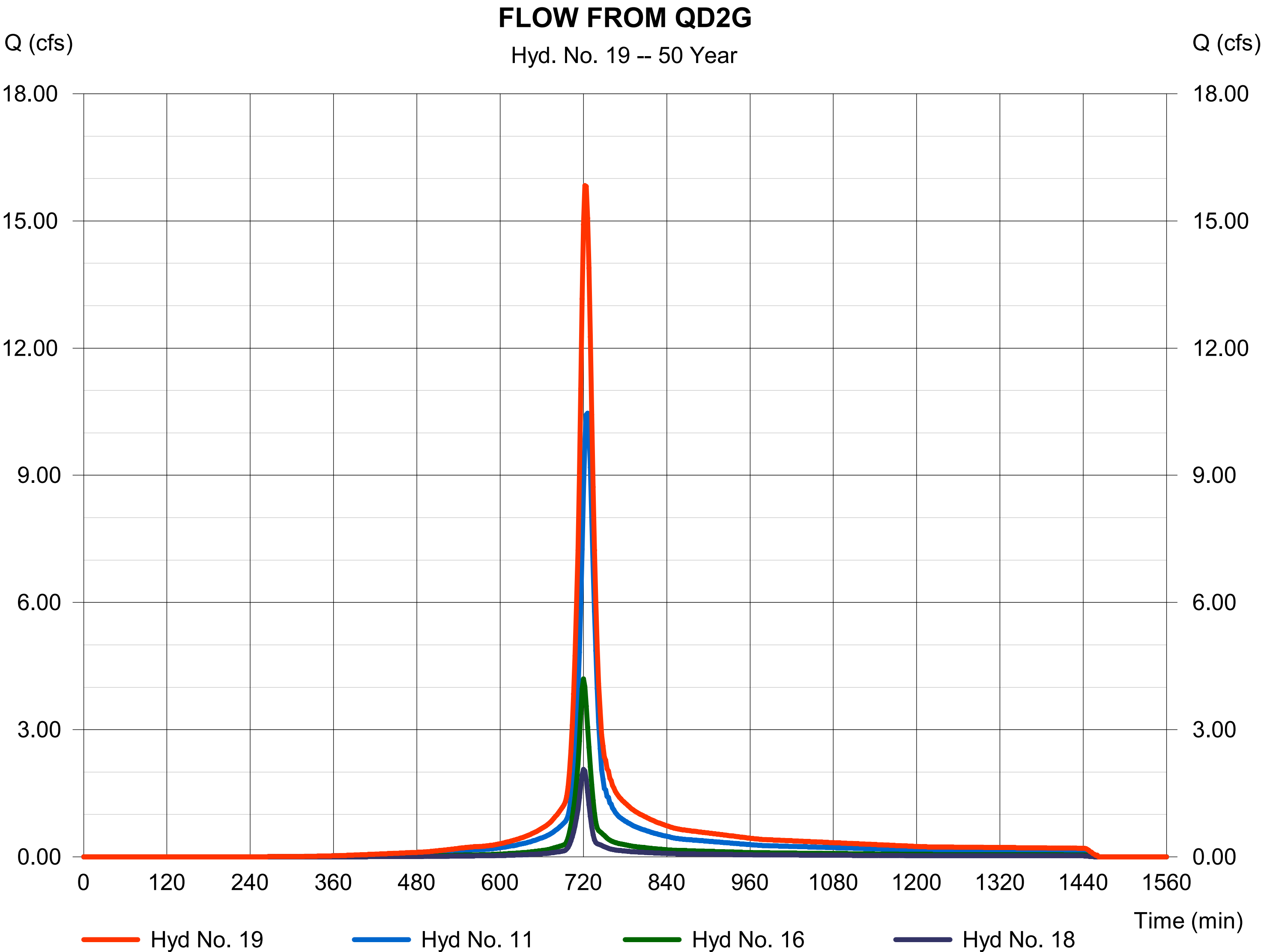
Hydrograph type	= SCS Runoff	Peak discharge	= 2.066 cfs
Storm frequency	= 50 yrs	Time to peak	= 720 min
Time interval	= 2 min	Hyd. volume	= 5,410 cuft
Drainage area	= 0.760 ac	Curve number	= 90
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 12.80 min
Total precip.	= 2.91 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hyd. No. 19

FLOW FROM QD2G

Hydrograph type	= Combine	Peak discharge	= 15.83 cfs
Storm frequency	= 50 yrs	Time to peak	= 722 min
Time interval	= 2 min	Hyd. volume	= 48,813 cuft
Inflow hyds.	= 11, 16, 18	Contrib. drain. area	= 0.760 ac

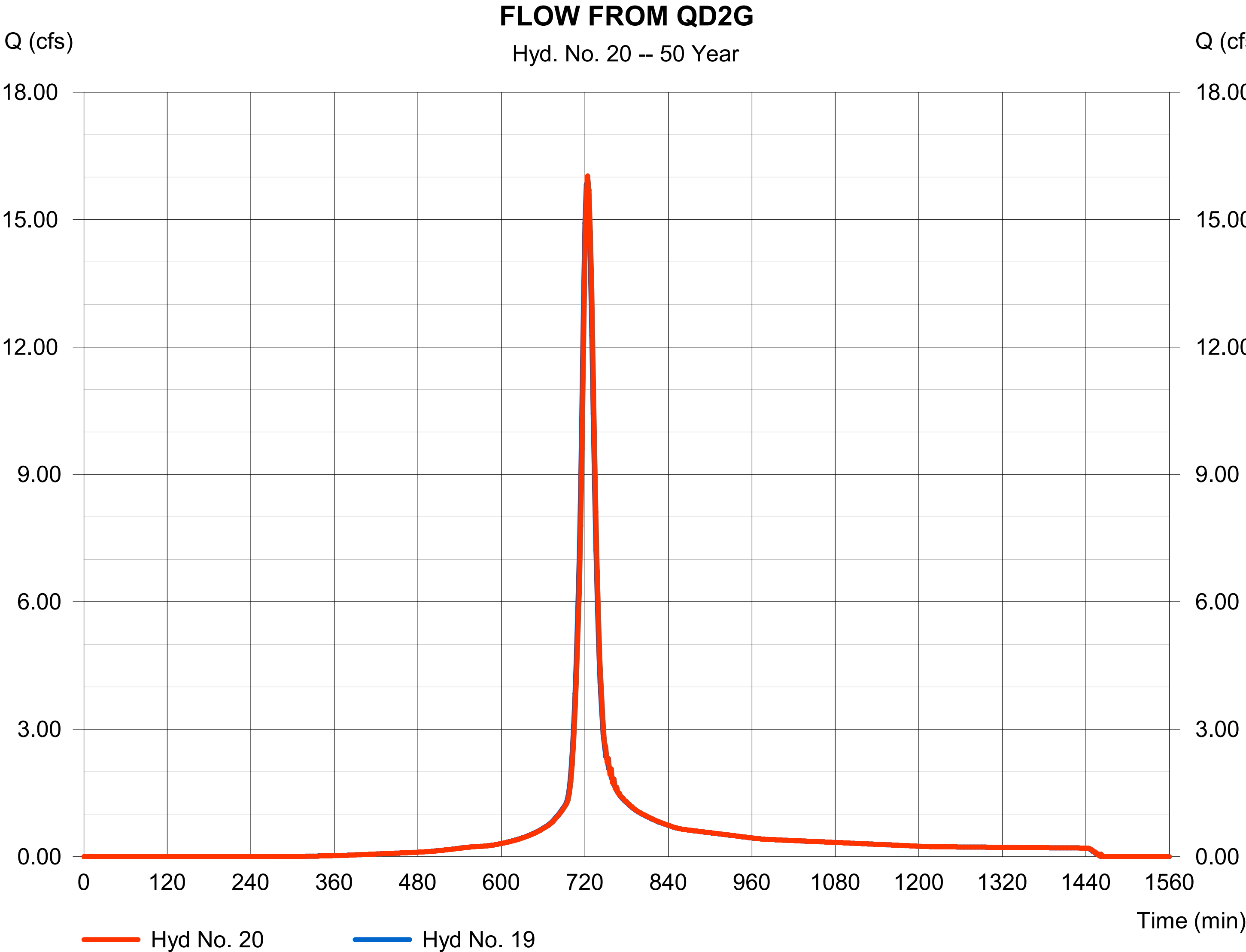


Hyd. No. 20

FLOW FROM QD2G

Hydrograph type	= Reach	Peak discharge	= 16.03 cfs
Storm frequency	= 50 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 48,815 cuft
Inflow hyd. No.	= 19 - FLOW FROM QD2G	Section type	= Circular
Reach length	= 176.6 ft	Channel slope	= 1.0 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 3.584	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.5838

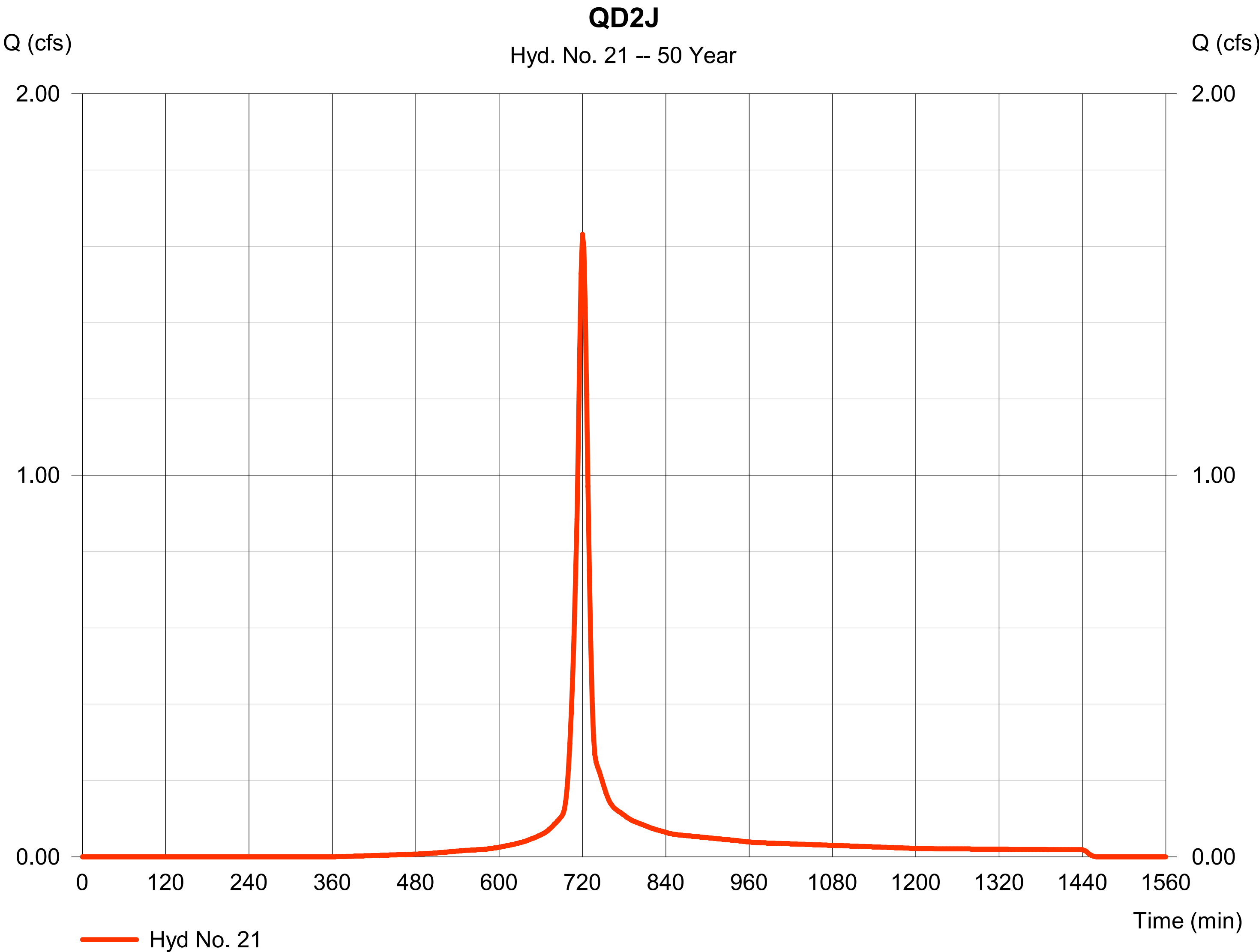
Modified Att-Kin routing method used.



Hyd. No. 21

QD2J

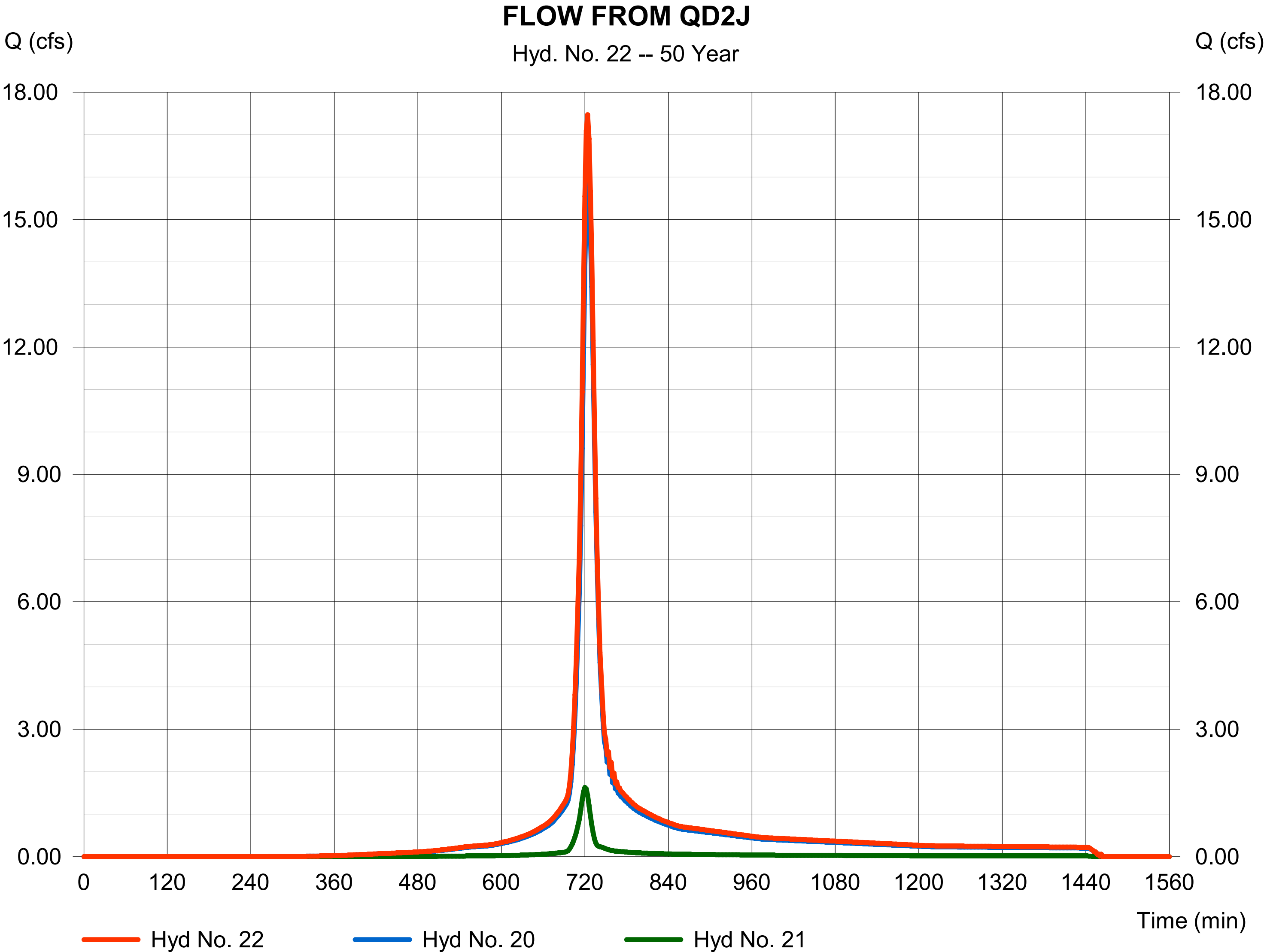
Hydrograph type	= SCS Runoff	Peak discharge	= 1.631 cfs
Storm frequency	= 50 yrs	Time to peak	= 720 min
Time interval	= 2 min	Hyd. volume	= 4,271 cuft
Drainage area	= 0.600 ac	Curve number	= 90
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 12.20 min
Total precip.	= 2.91 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hyd. No. 22

FLOW FROM QD2J

Hydrograph type	= Combine	Peak discharge	= 17.47 cfs
Storm frequency	= 50 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 53,086 cuft
Inflow hyds.	= 20, 21	Contrib. drain. area	= 0.600 ac

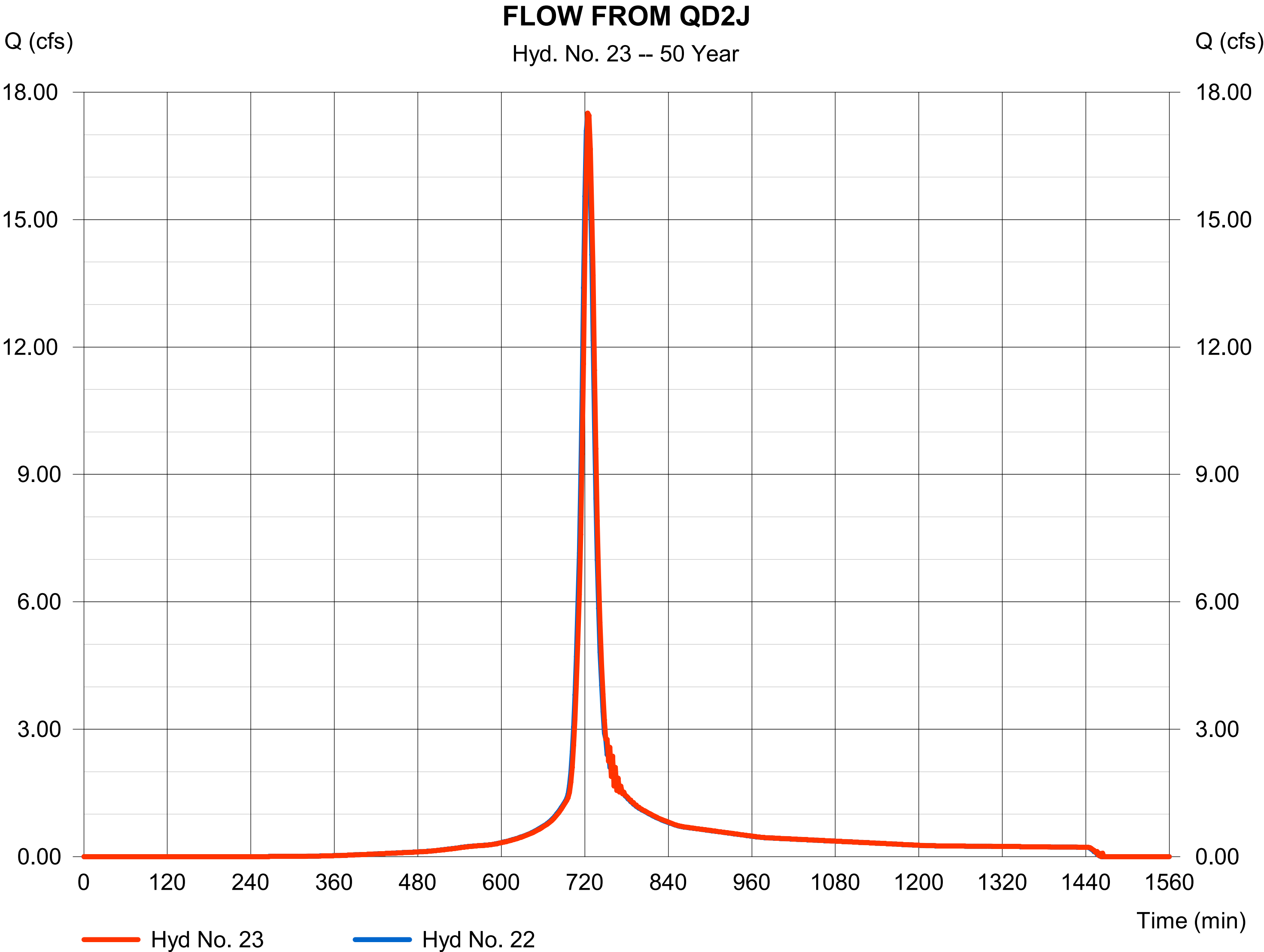


Hyd. No. 23

FLOW FROM QD2J

Hydrograph type	= Reach	Peak discharge	= 17.51 cfs
Storm frequency	= 50 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 53,091 cuft
Inflow hyd. No.	= 22 - FLOW FROM QD2J	Section type	= Circular
Reach length	= 158.4 ft	Channel slope	= 0.5 %
Manning's n	= 0.024	Bottom width	= 2.0 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.659	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.4494

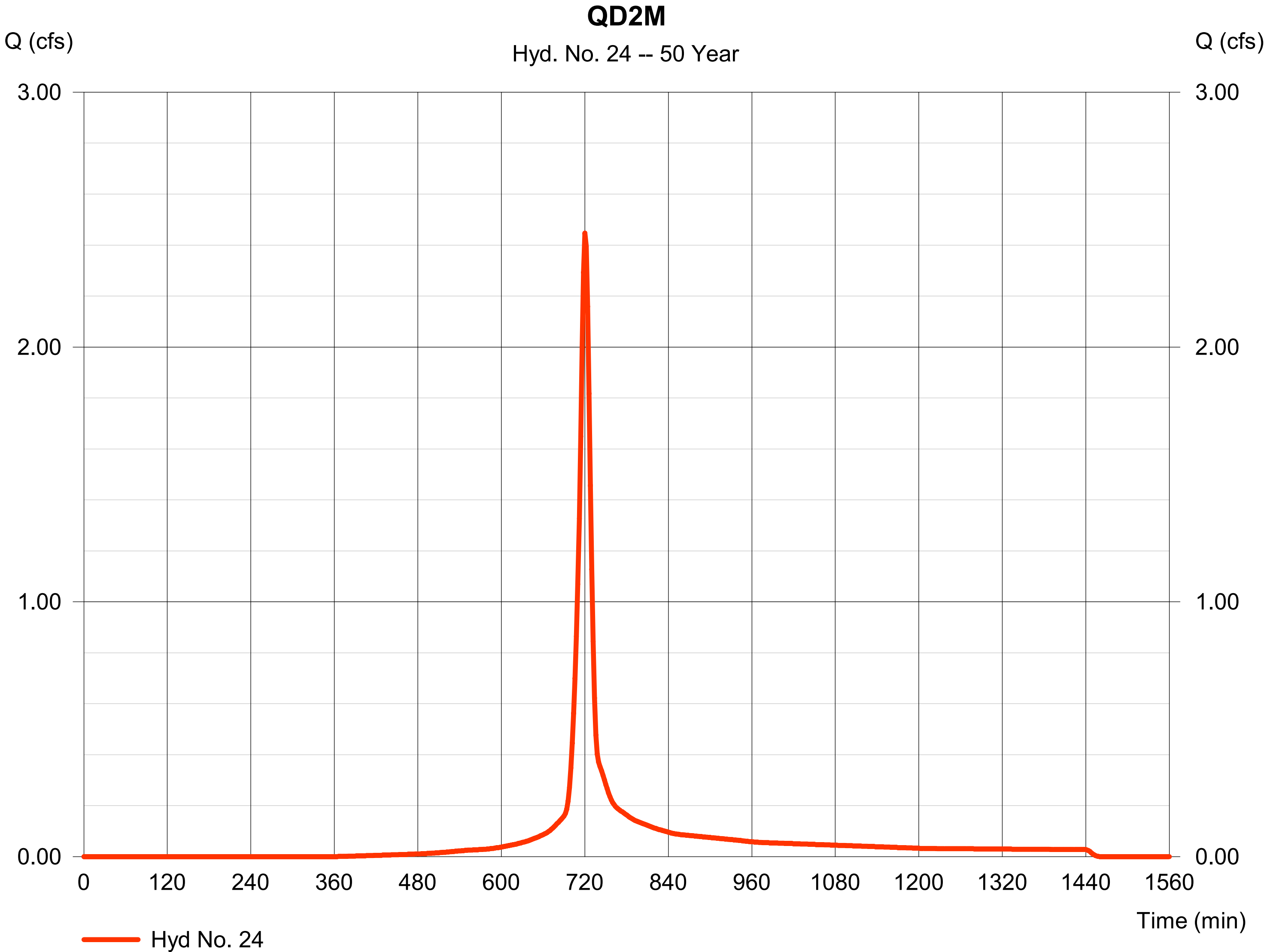
Modified Att-Kin routing method used.



Hyd. No. 24

QD2M

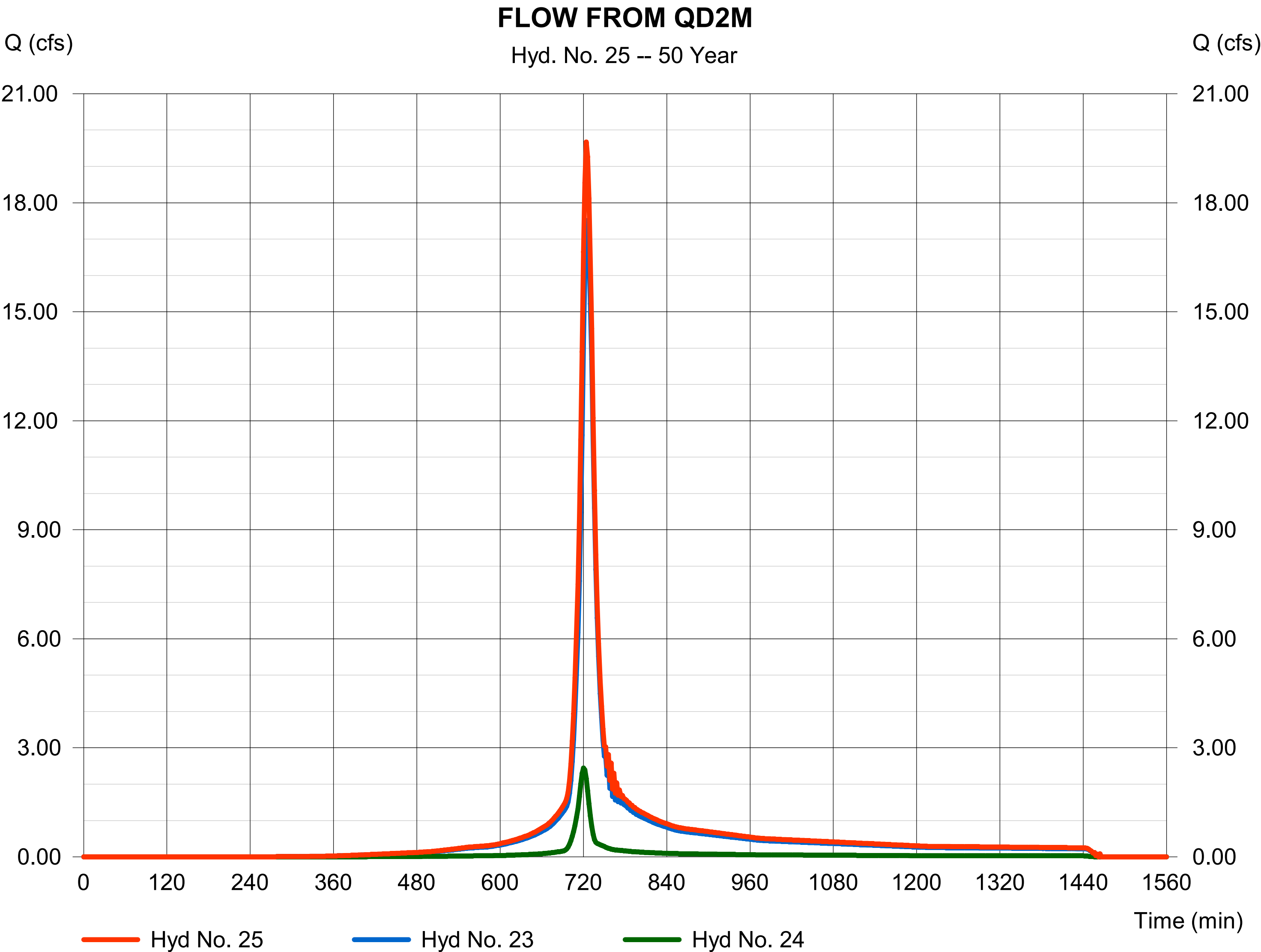
Hydrograph type	= SCS Runoff	Peak discharge	= 2.446 cfs
Storm frequency	= 50 yrs	Time to peak	= 720 min
Time interval	= 2 min	Hyd. volume	= 6,407 cuft
Drainage area	= 0.900 ac	Curve number	= 90
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 11.90 min
Total precip.	= 2.91 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hyd. No. 25

FLOW FROM QD2M

Hydrograph type	= Combine	Peak discharge	= 19.67 cfs
Storm frequency	= 50 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 59,497 cuft
Inflow hyds.	= 23, 24	Contrib. drain. area	= 0.900 ac

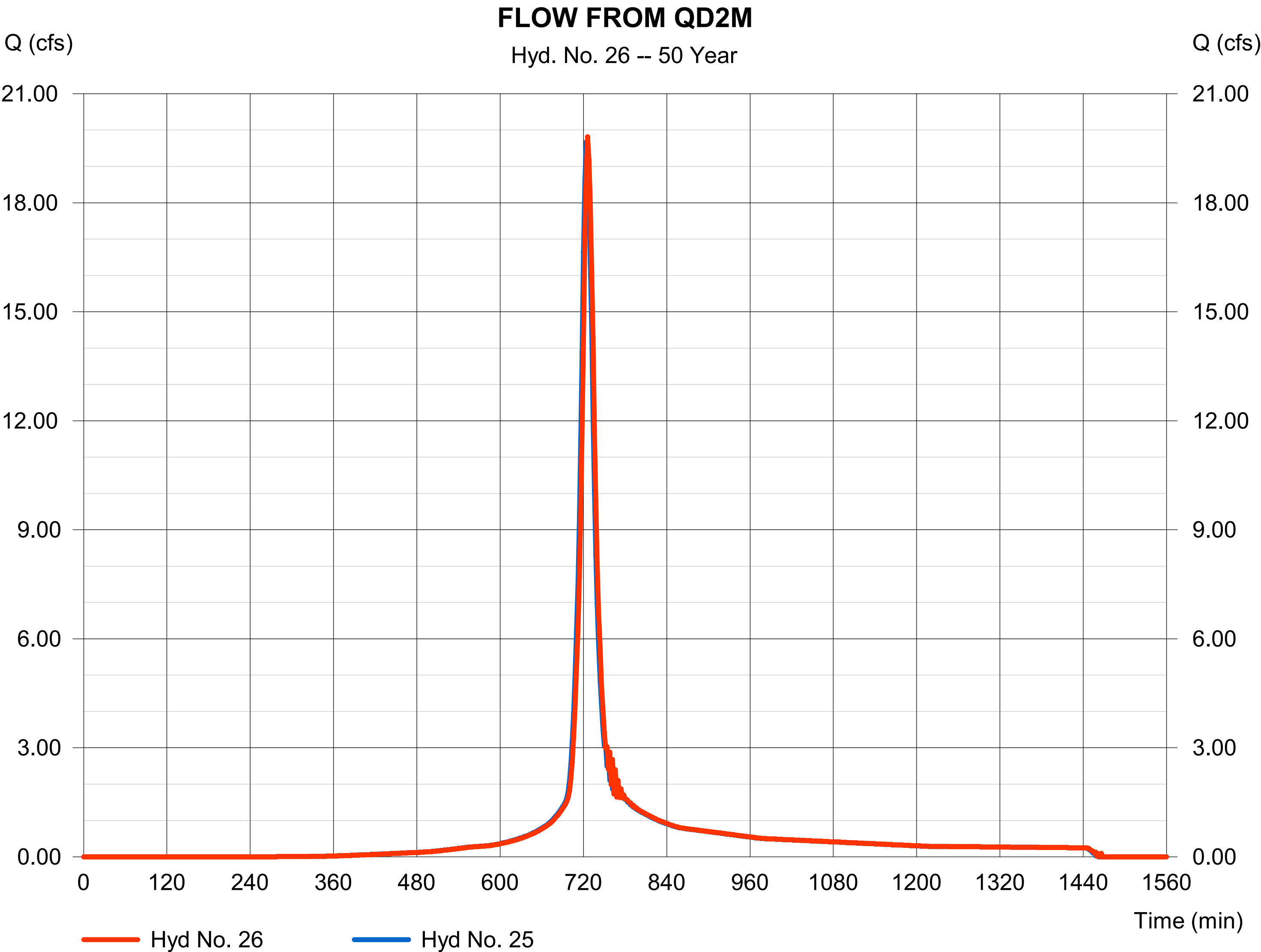


Hyd. No. 26

FLOW FROM QD2M

Hydrograph type	= Reach	Peak discharge	= 19.81 cfs
Storm frequency	= 50 yrs	Time to peak	= 726 min
Time interval	= 2 min	Hyd. volume	= 59,501 cuft
Inflow hyd. No.	= 25 - FLOW FROM QD2M	Section type	= Circular
Reach length	= 336.3 ft	Channel slope	= 0.3 %
Manning's n	= 0.024	Bottom width	= 2.0 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.060	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.1653

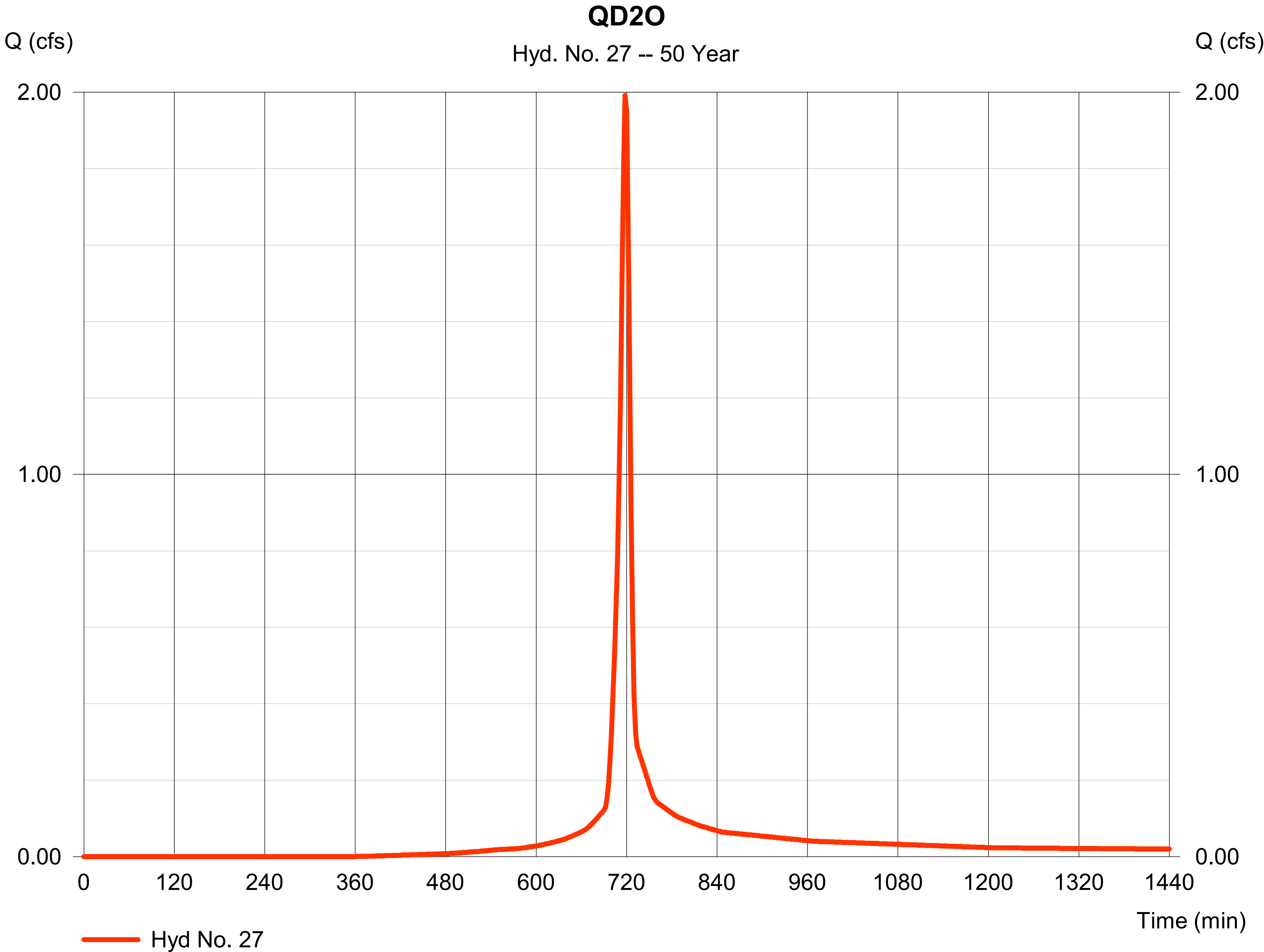
Modified Att-Kin routing method used.



Hyd. No. 27

QD2O

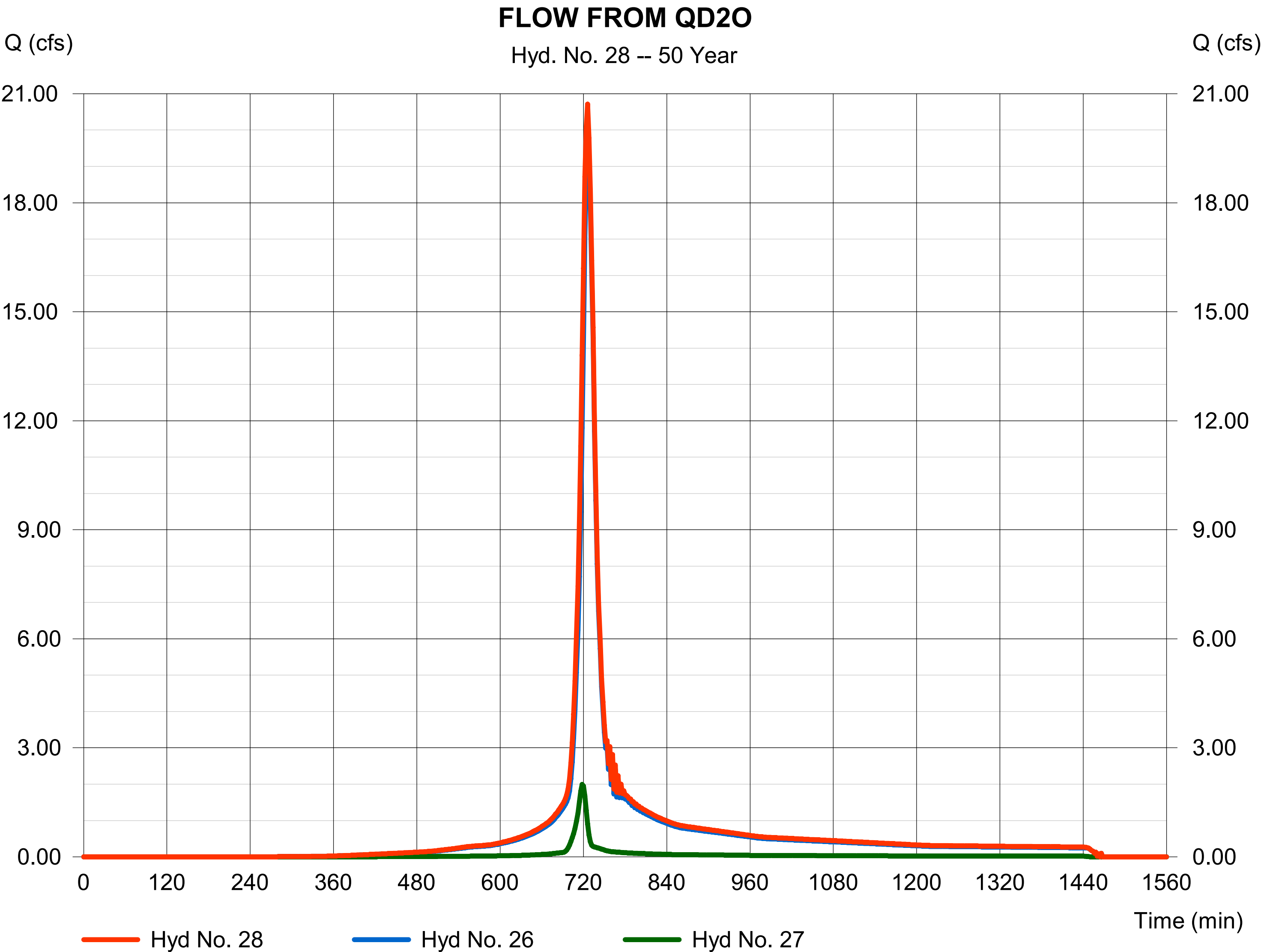
Hydrograph type	= SCS Runoff	Peak discharge	= 1.992 cfs
Storm frequency	= 50 yrs	Time to peak	= 718 min
Time interval	= 2 min	Hyd. volume	= 4,625 cuft
Drainage area	= 0.670 ac	Curve number	= 90
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 7.40 min
Total precip.	= 2.91 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hyd. No. 28

FLOW FROM QD2O

Hydrograph type	= Combine	Peak discharge	= 20.72 cfs
Storm frequency	= 50 yrs	Time to peak	= 726 min
Time interval	= 2 min	Hyd. volume	= 64,126 cuft
Inflow hyds.	= 26, 27	Contrib. drain. area	= 0.670 ac

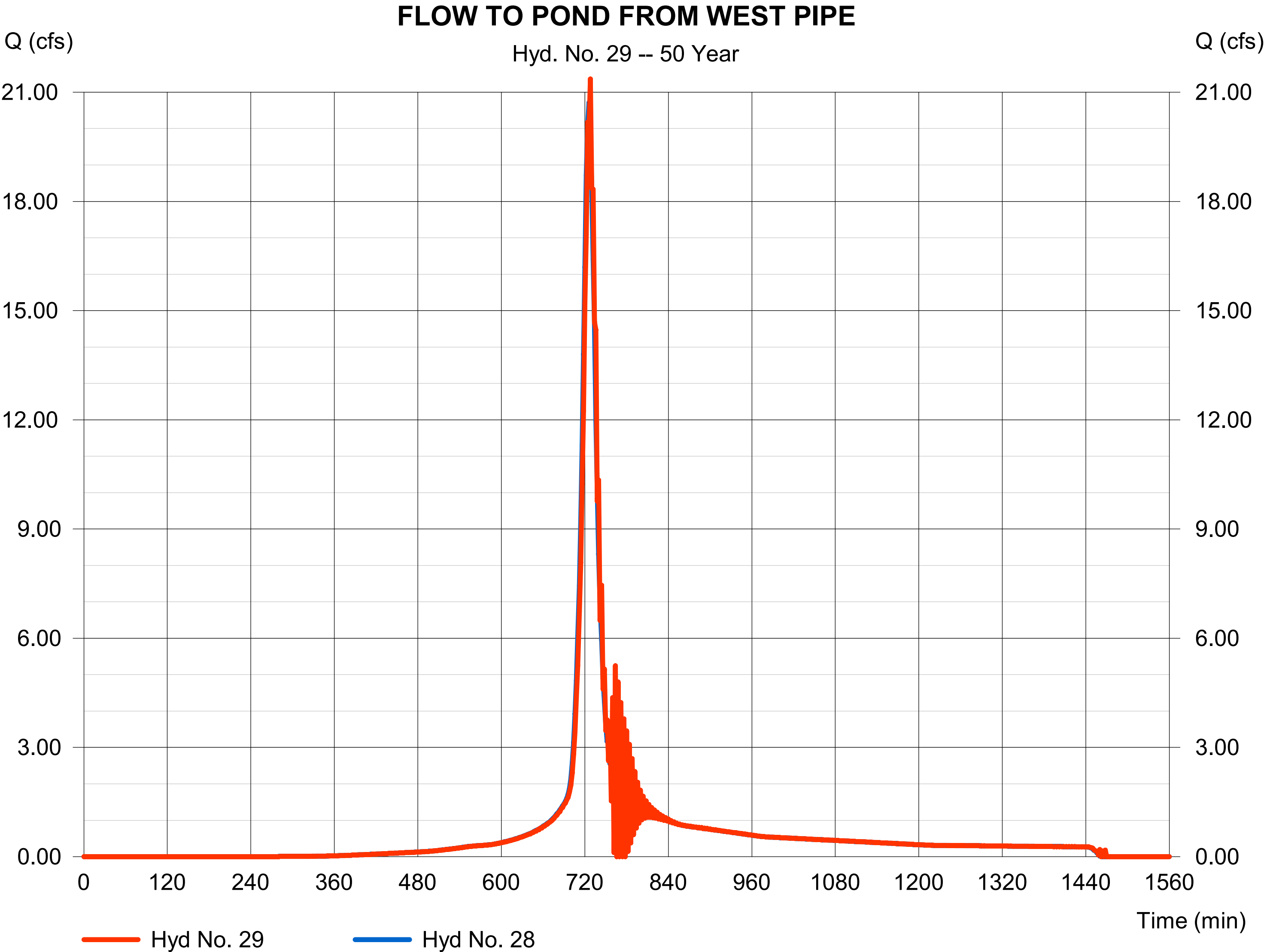


Hyd. No. 29

FLOW TO POND FROM WEST PIPE

Hydrograph type	= Reach	Peak discharge	= 21.36 cfs
Storm frequency	= 50 yrs	Time to peak	= 728 min
Time interval	= 2 min	Hyd. volume	= 64,321 cuft
Inflow hyd. No.	= 28 - FLOW FROM QD2O	Section type	= Circular
Reach length	= 27.1 ft	Channel slope	= 0.3 %
Manning's n	= 0.024	Bottom width	= 2.0 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.060	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.8959

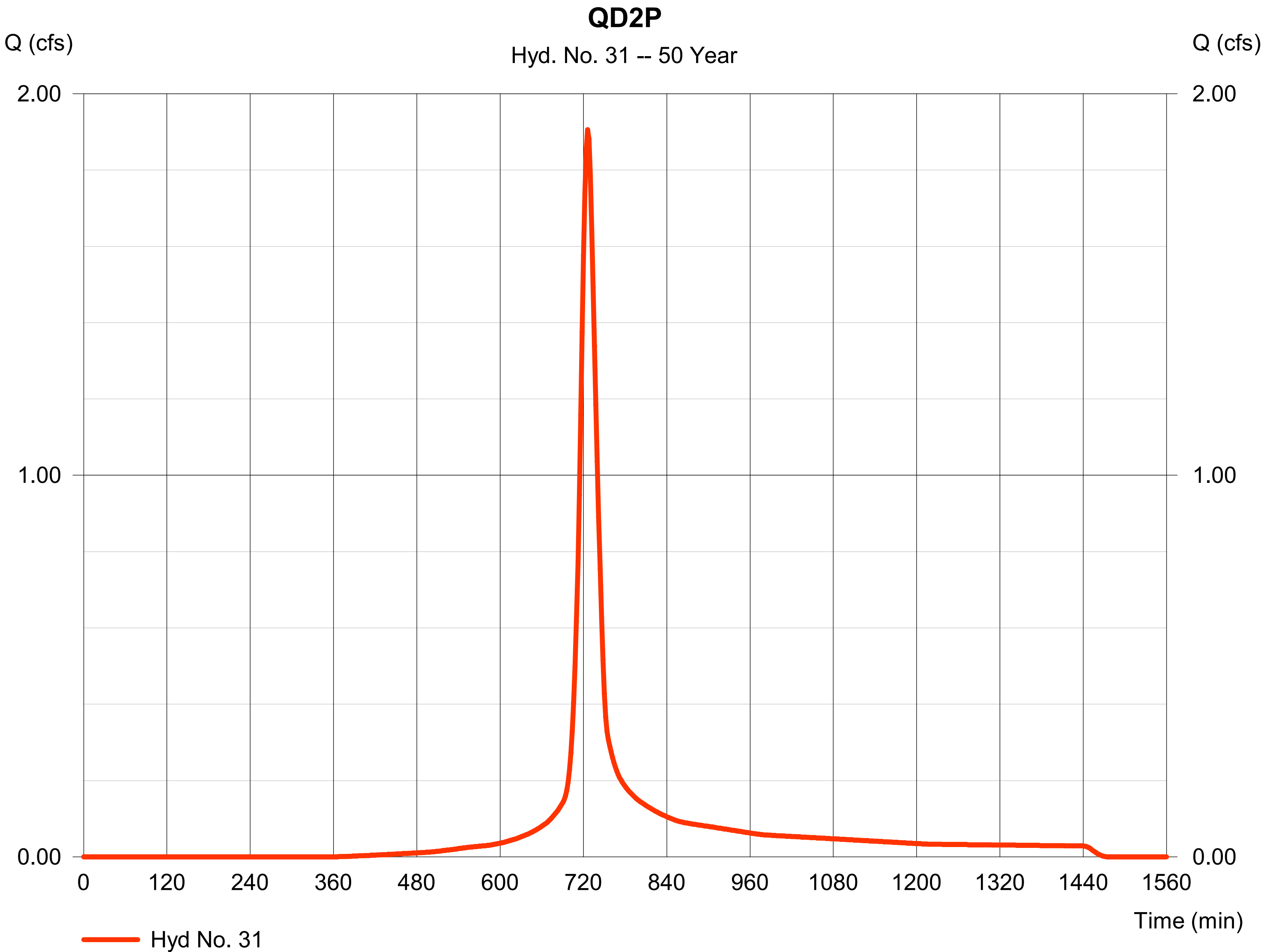
Modified Att-Kin routing method used.



Hyd. No. 31

QD2P

Hydrograph type	= SCS Runoff	Peak discharge	= 1.906 cfs
Storm frequency	= 50 yrs	Time to peak	= 726 min
Time interval	= 2 min	Hyd. volume	= 6,605 cuft
Drainage area	= 0.940 ac	Curve number	= 90
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 22.40 min
Total precip.	= 2.91 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

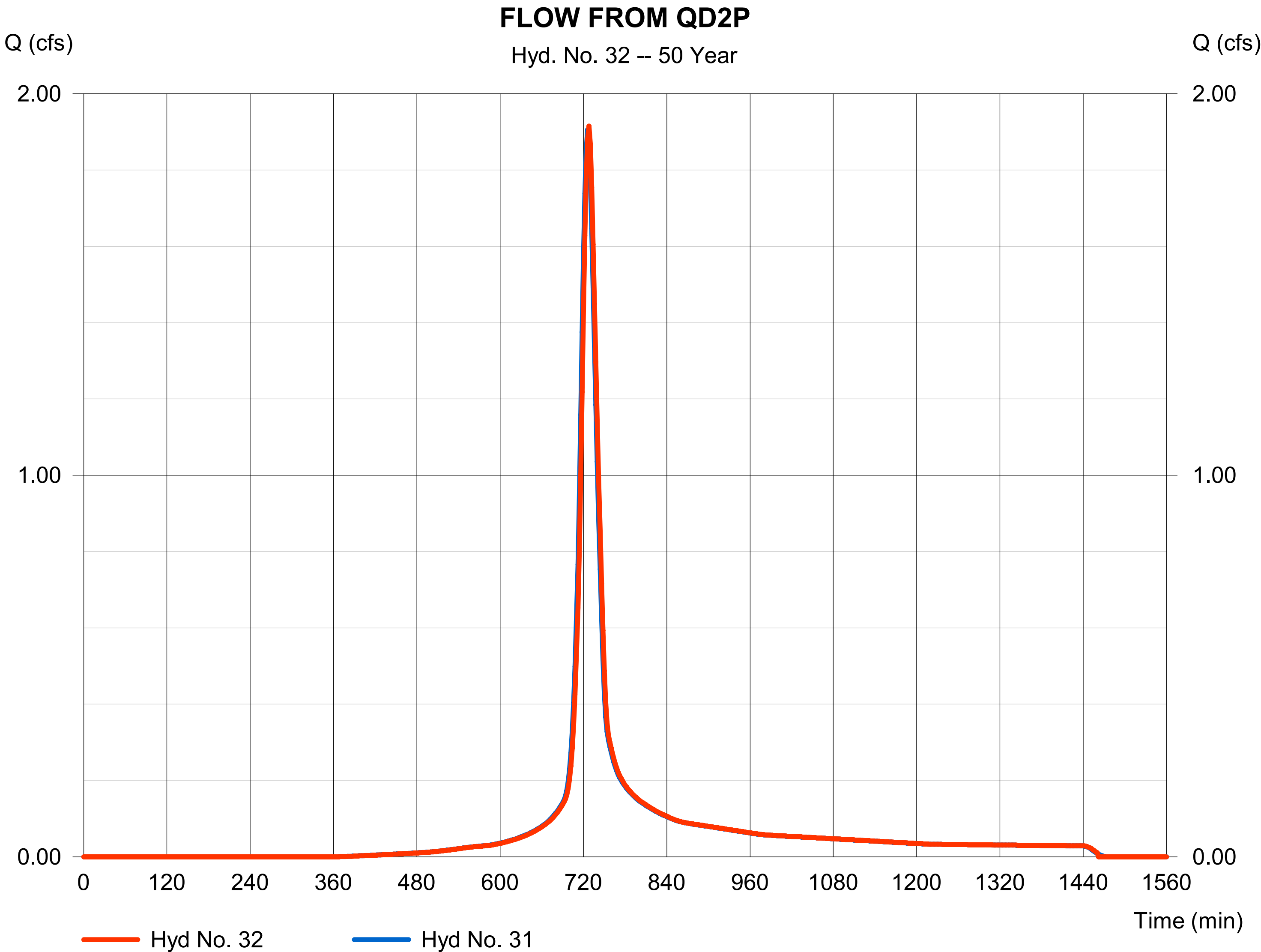


Hyd. No. 32

FLOW FROM QD2P

Hydrograph type	= Reach	Peak discharge	= 1.915 cfs
Storm frequency	= 50 yrs	Time to peak	= 728 min
Time interval	= 2 min	Hyd. volume	= 6,601 cuft
Inflow hyd. No.	= 31 - QD2P	Section type	= Circular
Reach length	= 36.3 ft	Channel slope	= 1.0 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 3.584	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.3811

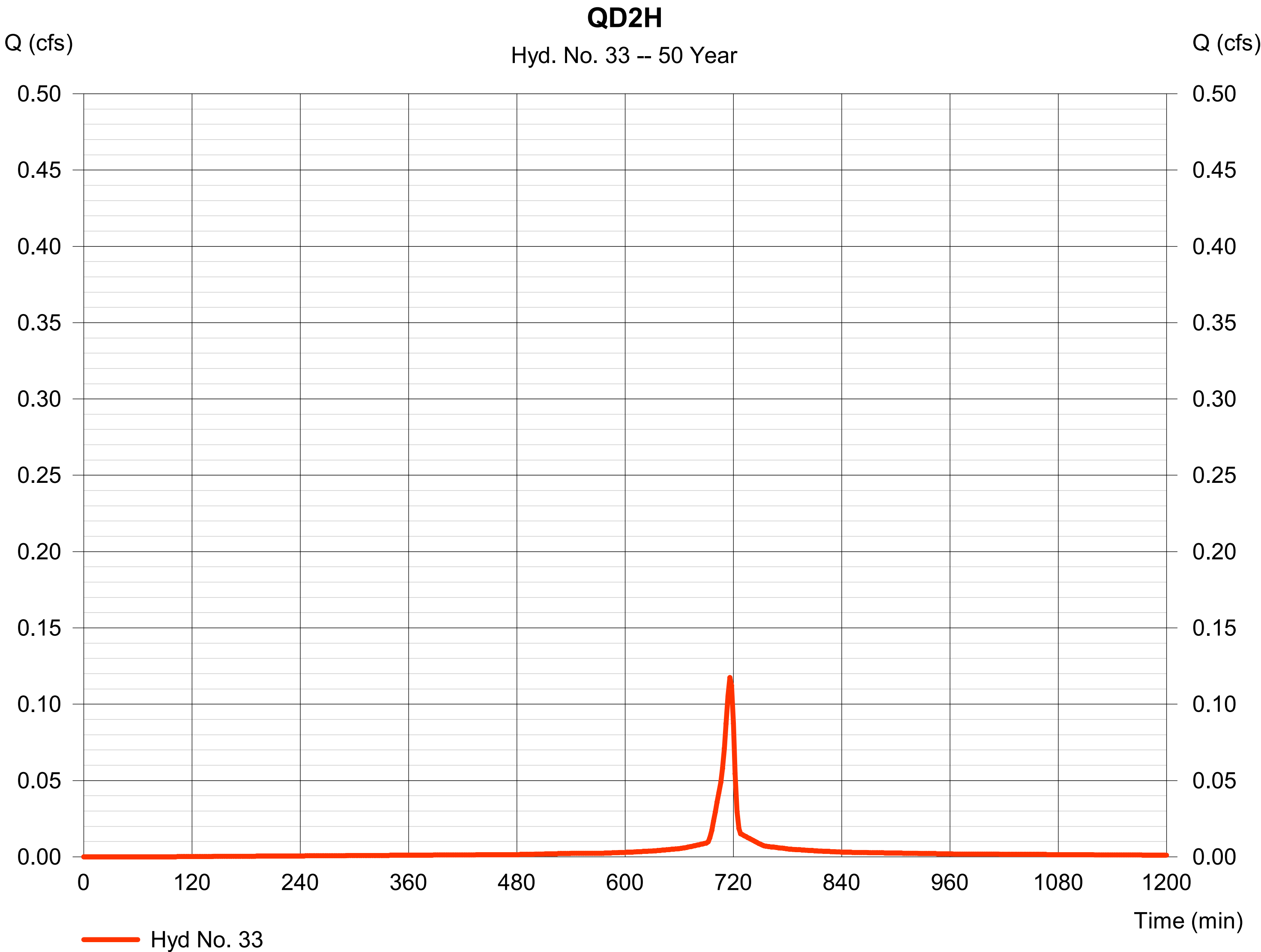
Modified Att-Kin routing method used.



Hyd. No. 33

QD2H

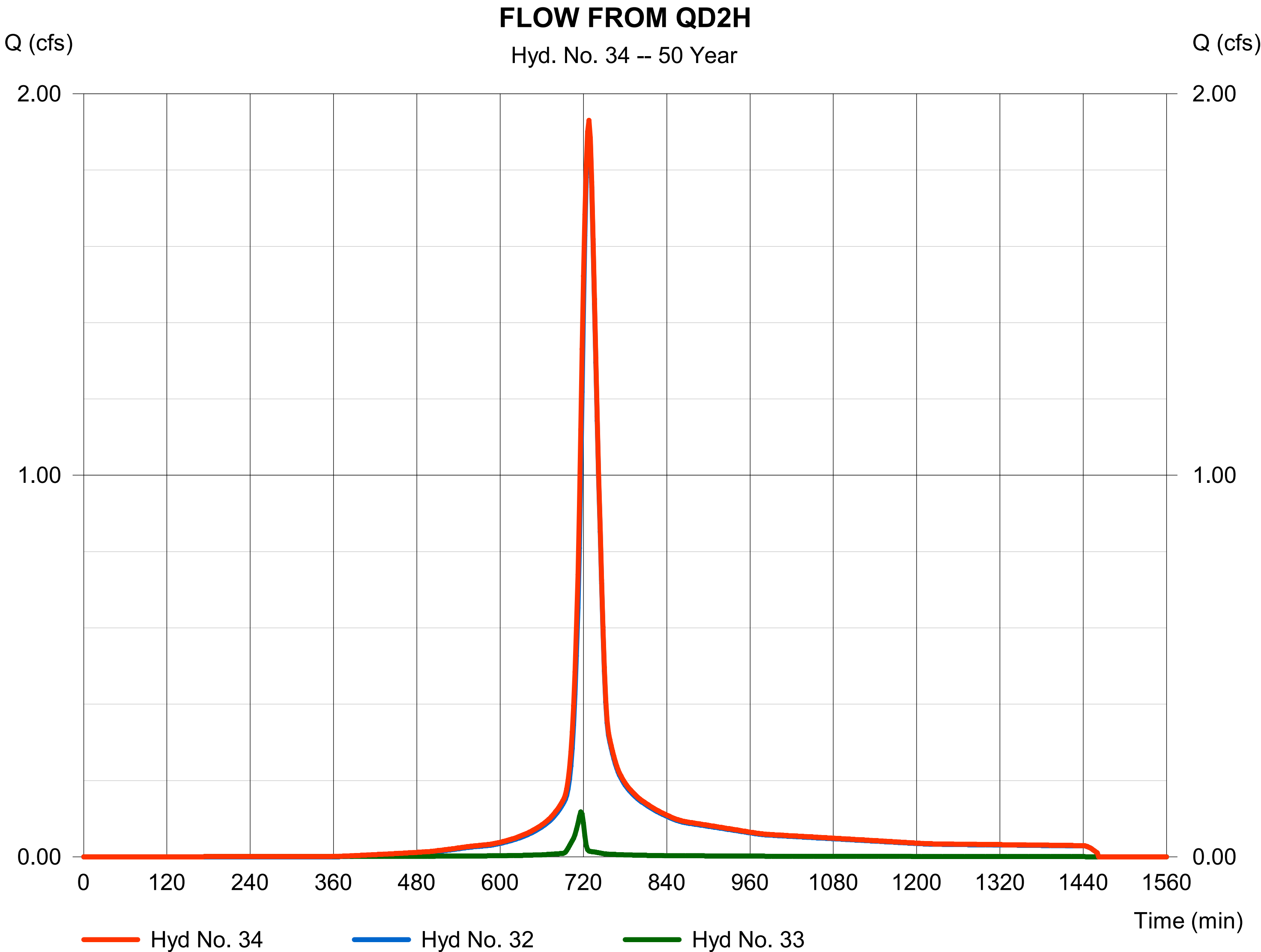
Hydrograph type	=	SCS Runoff	Peak discharge	=	0.118 cfs
Storm frequency	=	50 yrs	Time to peak	=	716 min
Time interval	=	2 min	Hyd. volume	=	273 cuft
Drainage area	=	0.030 ac	Curve number	=	98
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	6.00 min
Total precip.	=	2.91 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hyd. No. 34

FLOW FROM QD2H

Hydrograph type	= Combine	Peak discharge	= 1.930 cfs
Storm frequency	= 50 yrs	Time to peak	= 728 min
Time interval	= 2 min	Hyd. volume	= 6,875 cuft
Inflow hyds.	= 32, 33	Contrib. drain. area	= 0.030 ac

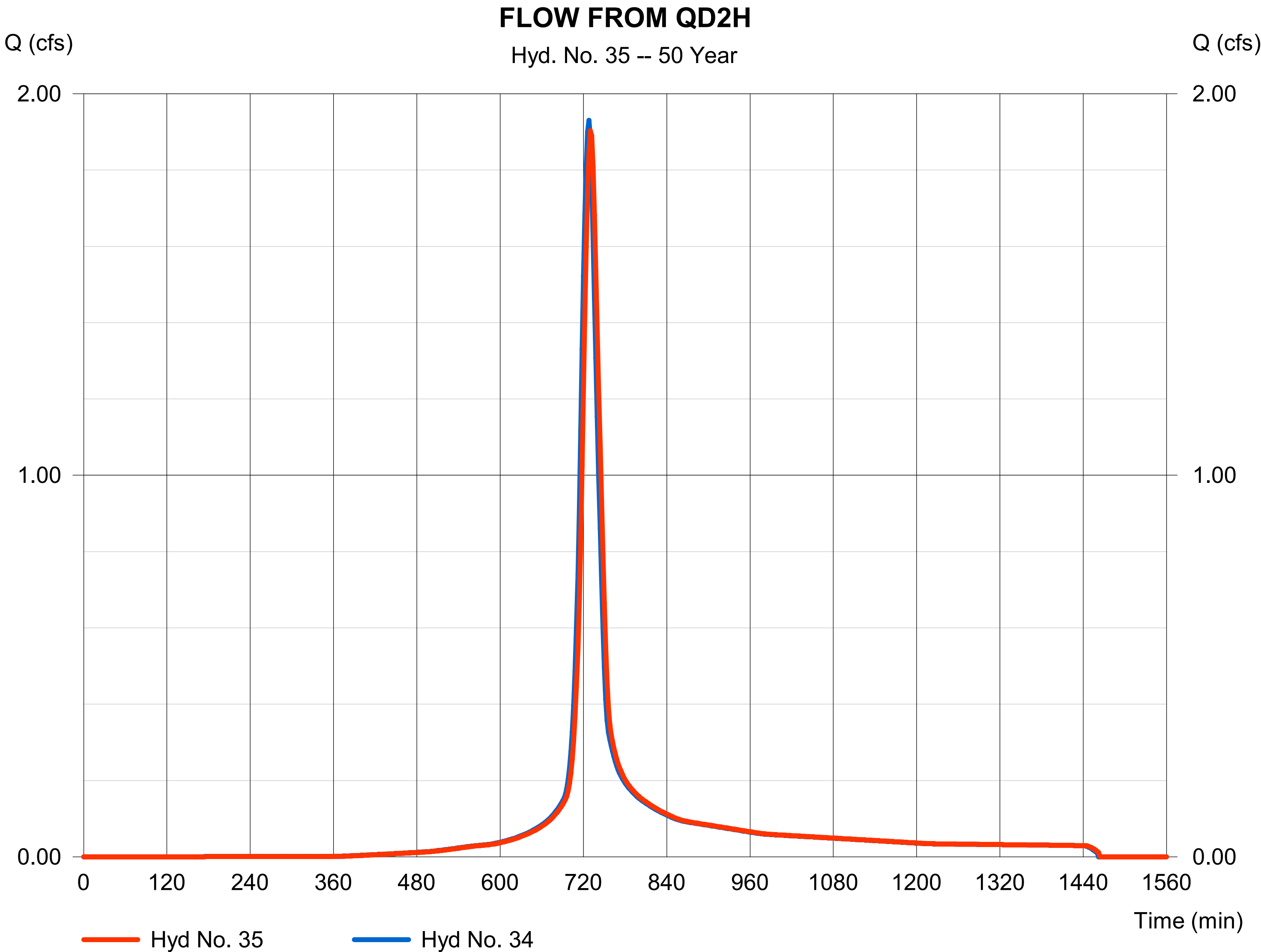


Hyd. No. 35

FLOW FROM QD2H

Hydrograph type	= Reach	Peak discharge	= 1.903 cfs
Storm frequency	= 50 yrs	Time to peak	= 730 min
Time interval	= 2 min	Hyd. volume	= 6,874 cuft
Inflow hyd. No.	= 34 - FLOW FROM QD2H	Section type	= Circular
Reach length	= 162.5 ft	Channel slope	= 1.0 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 3.584	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 0.6703

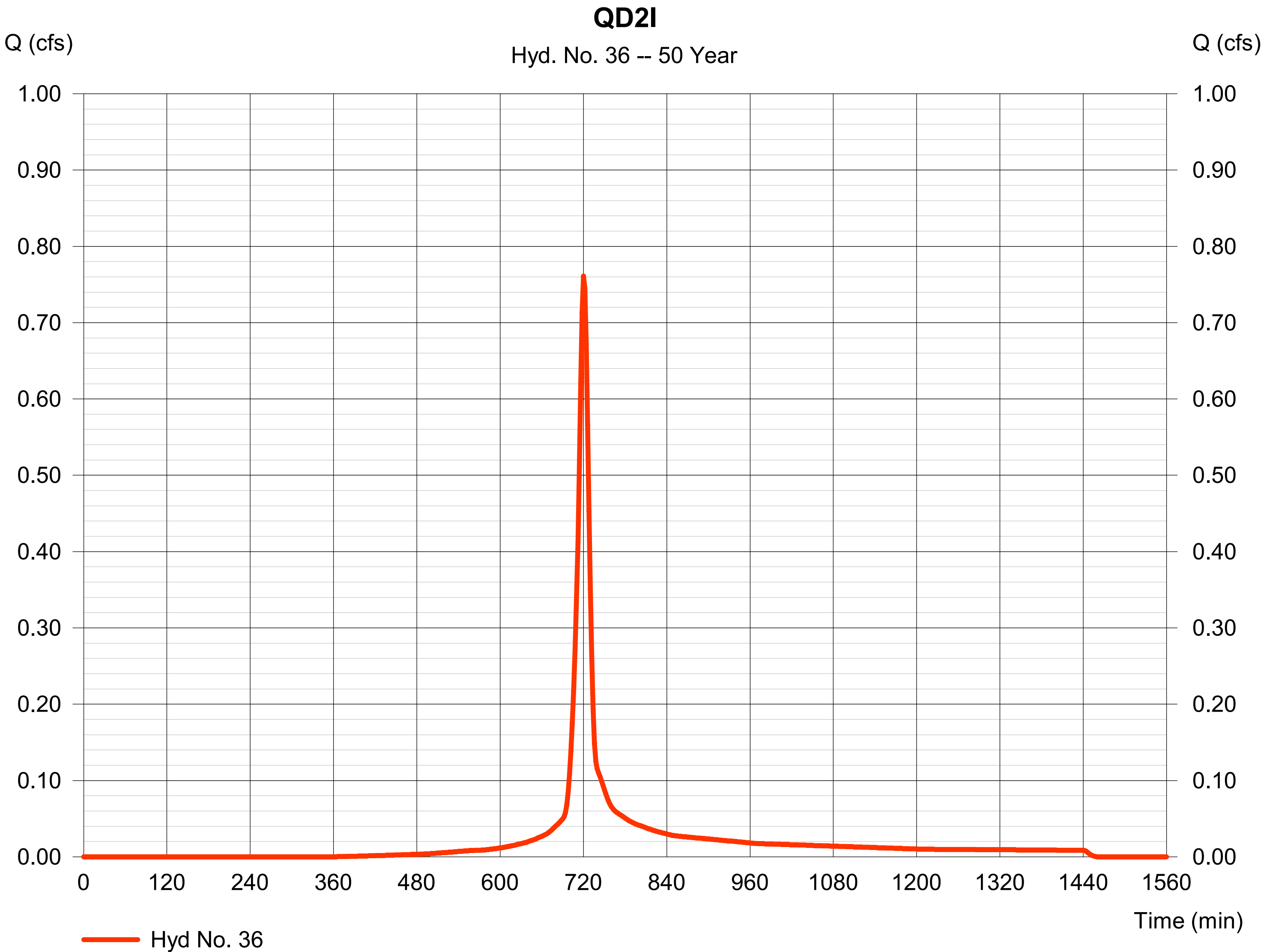
Modified Att-Kin routing method used.



Hyd. No. 36

QD2I

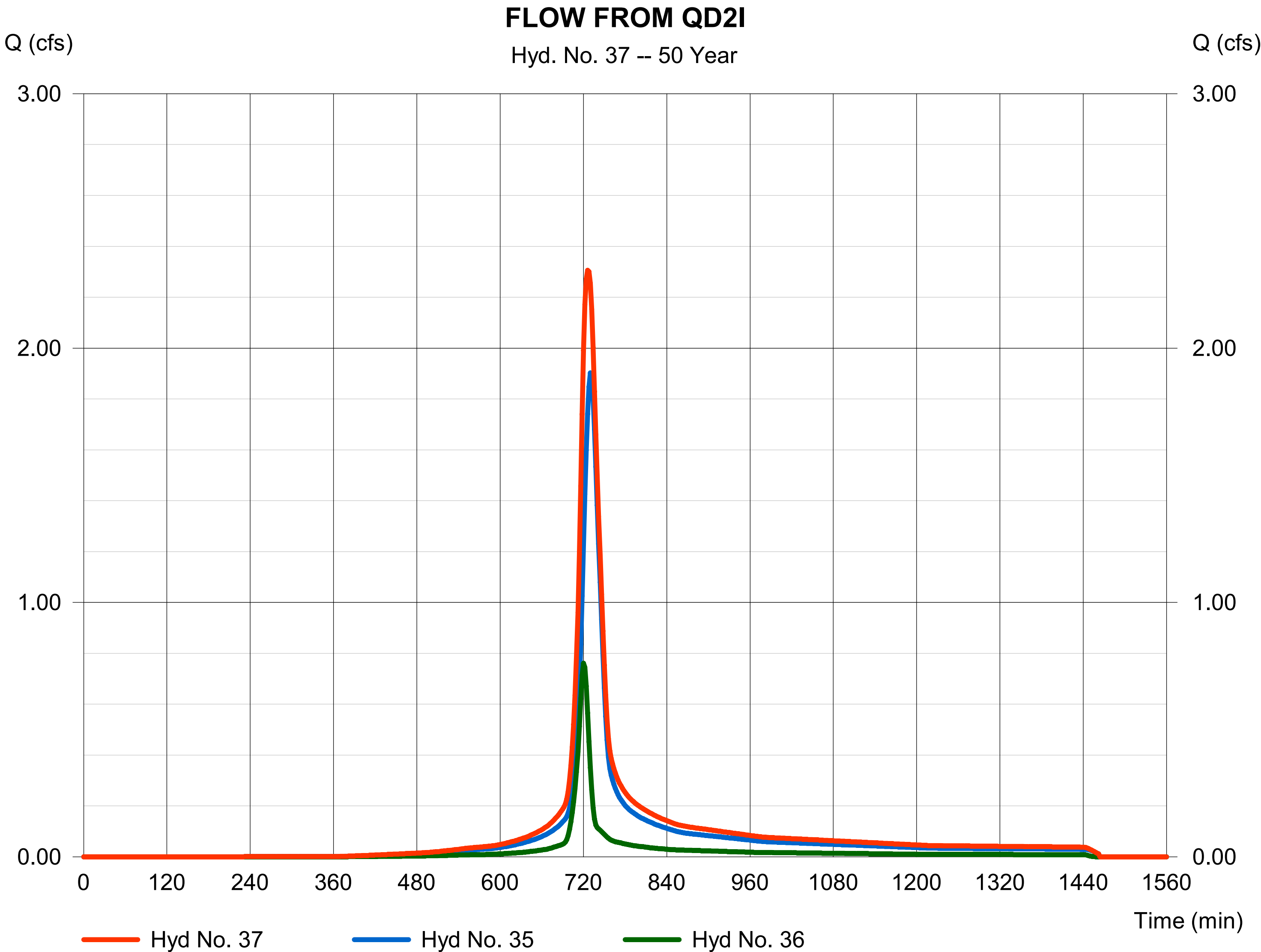
Hydrograph type	=	SCS Runoff	Peak discharge	=	0.761 cfs
Storm frequency	=	50 yrs	Time to peak	=	720 min
Time interval	=	2 min	Hyd. volume	=	1,993 cuft
Drainage area	=	0.280 ac	Curve number	=	90
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	12.30 min
Total precip.	=	2.91 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hyd. No. 37

FLOW FROM QD2I

Hydrograph type	= Combine	Peak discharge	= 2.306 cfs
Storm frequency	= 50 yrs	Time to peak	= 726 min
Time interval	= 2 min	Hyd. volume	= 8,867 cuft
Inflow hyds.	= 35, 36	Contrib. drain. area	= 0.280 ac

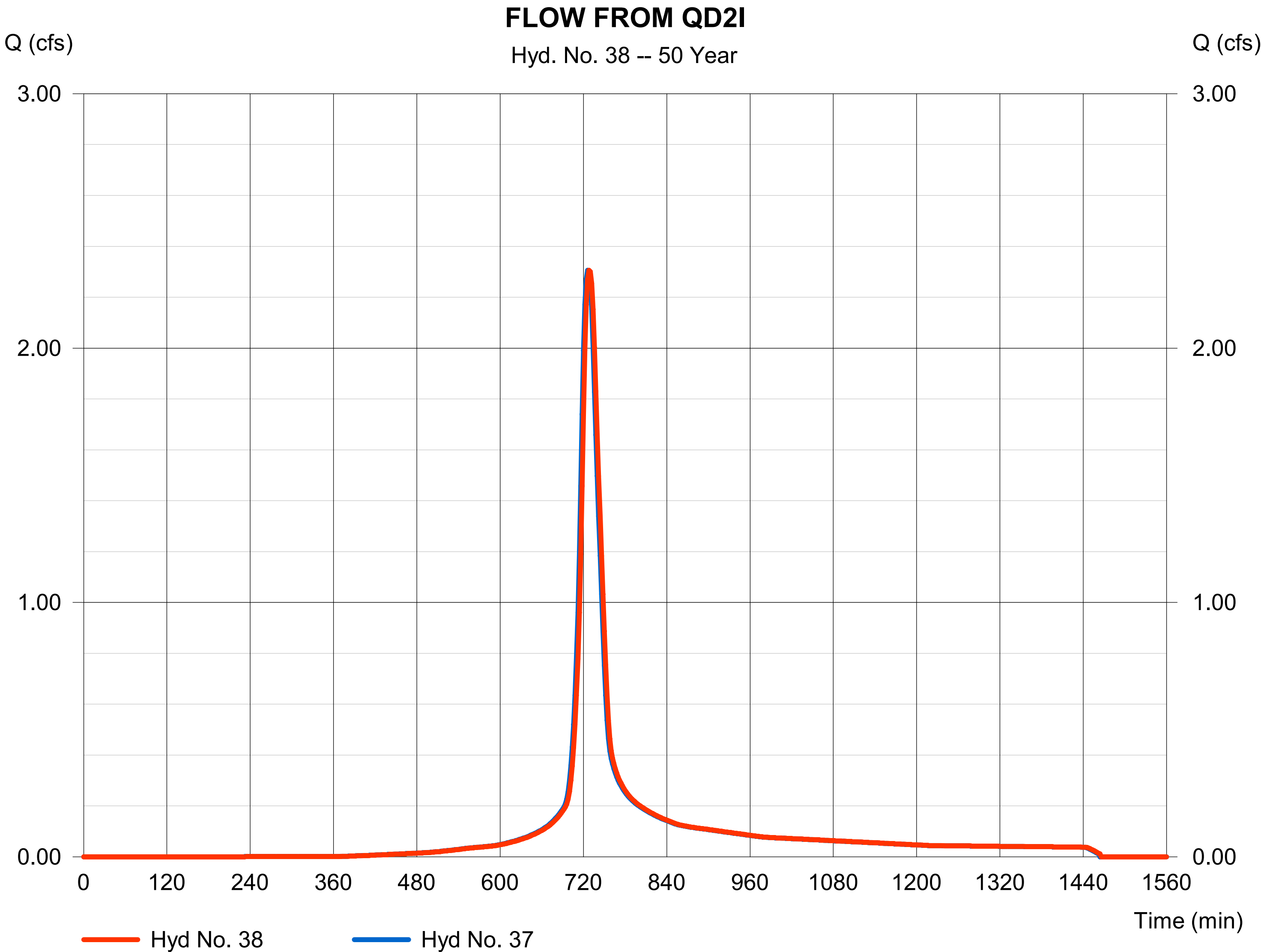


Hyd. No. 38

FLOW FROM QD2I

Hydrograph type	= Reach	Peak discharge	= 2.306 cfs
Storm frequency	= 50 yrs	Time to peak	= 728 min
Time interval	= 2 min	Hyd. volume	= 8,867 cuft
Inflow hyd. No.	= 37 - FLOW FROM QD2I	Section type	= Circular
Reach length	= 97.5 ft	Channel slope	= 0.7 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.934	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.0020

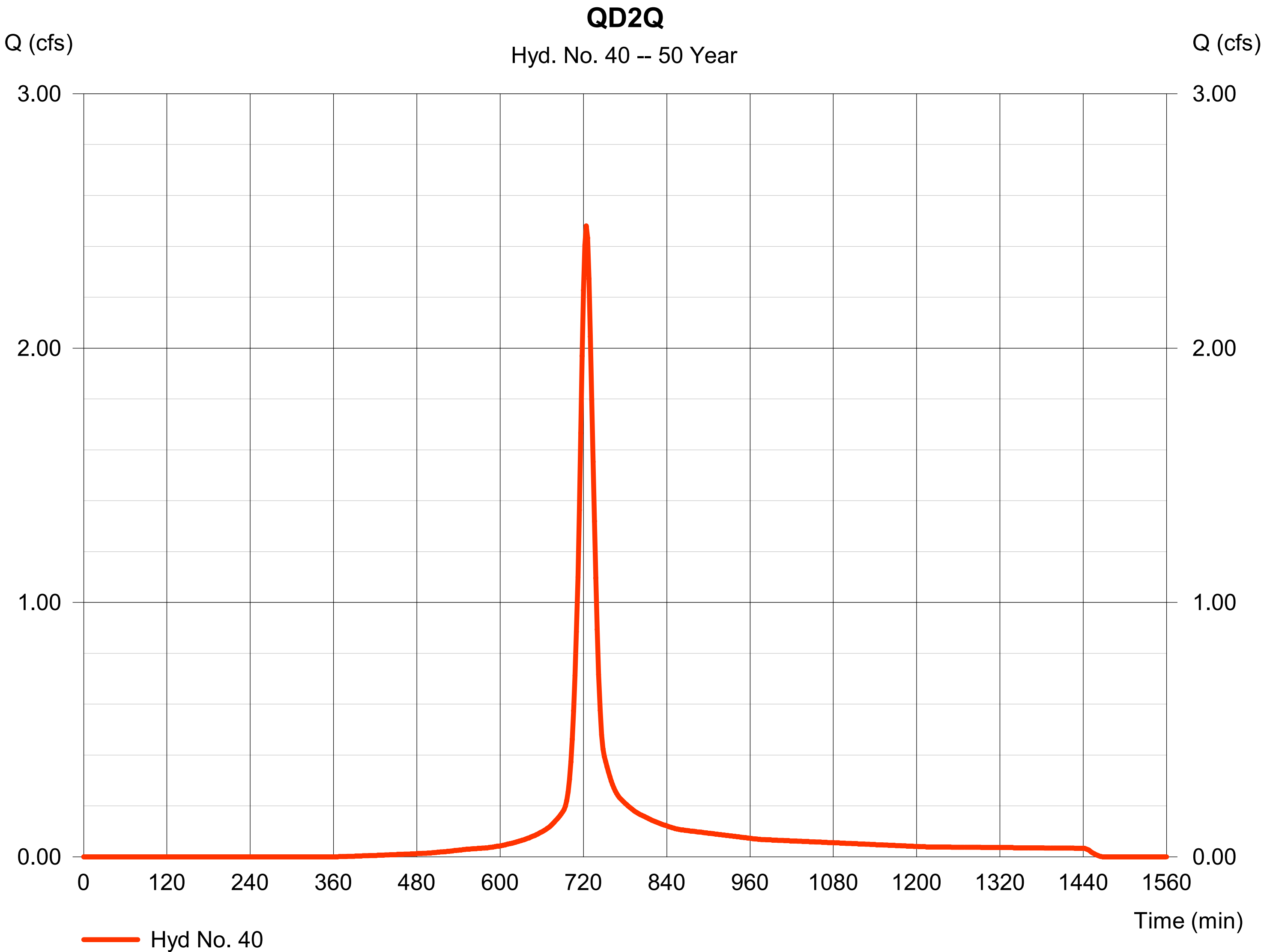
Modified Att-Kin routing method used.



Hyd. No. 40

QD2Q

Hydrograph type	=	SCS Runoff	Peak discharge	=	2.481 cfs
Storm frequency	=	50 yrs	Time to peak	=	724 min
Time interval	=	2 min	Hyd. volume	=	7,800 cuft
Drainage area	=	1.130 ac	Curve number	=	90
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	16.90 min
Total precip.	=	2.91 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484

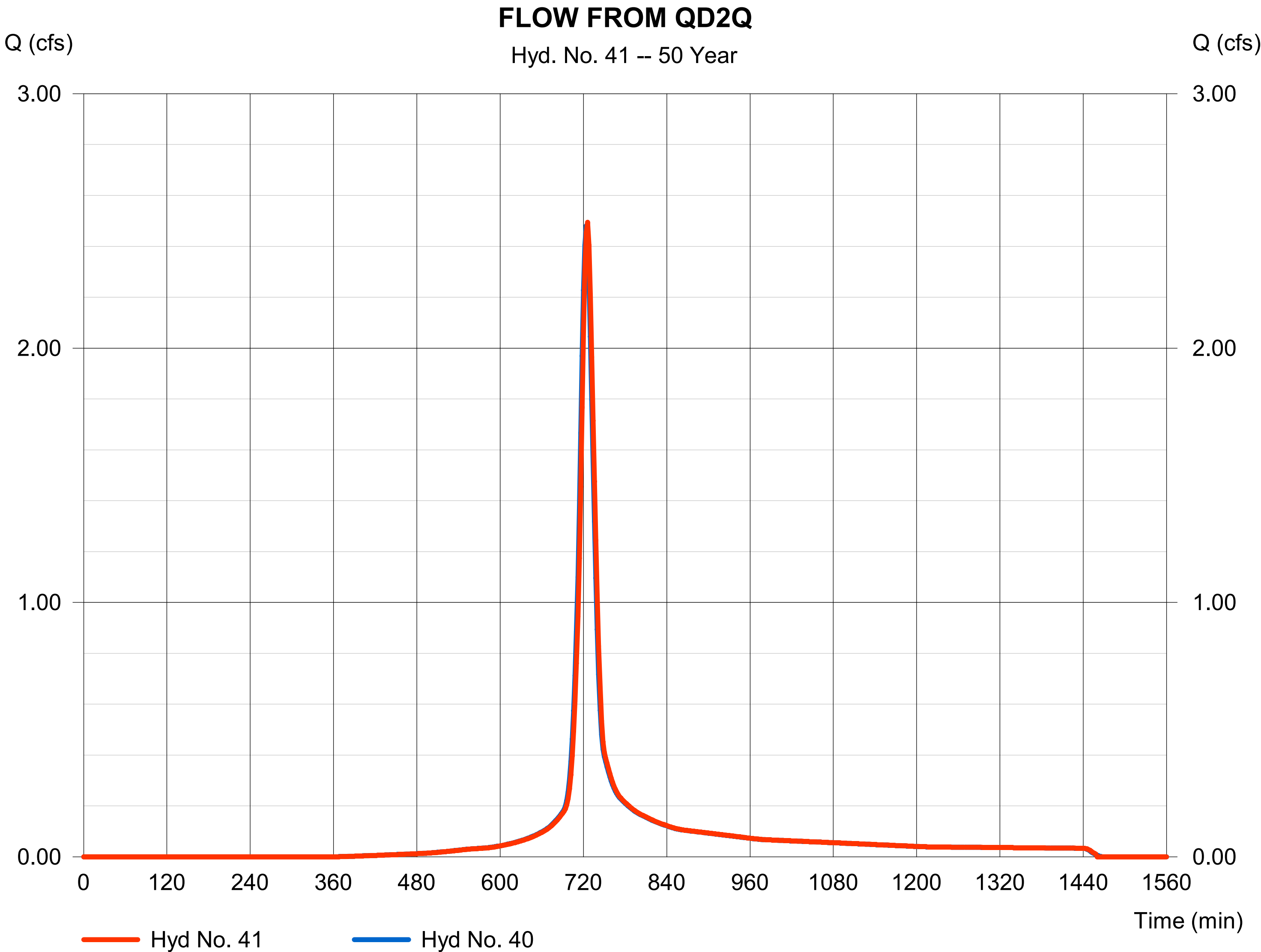


Hyd. No. 41

FLOW FROM QD2Q

Hydrograph type	= Reach	Peak discharge	= 2.494 cfs
Storm frequency	= 50 yrs	Time to peak	= 726 min
Time interval	= 2 min	Hyd. volume	= 7,798 cuft
Inflow hyd. No.	= 40 - QD2Q	Section type	= Circular
Reach length	= 36.9 ft	Channel slope	= 0.7 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.999	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.4806

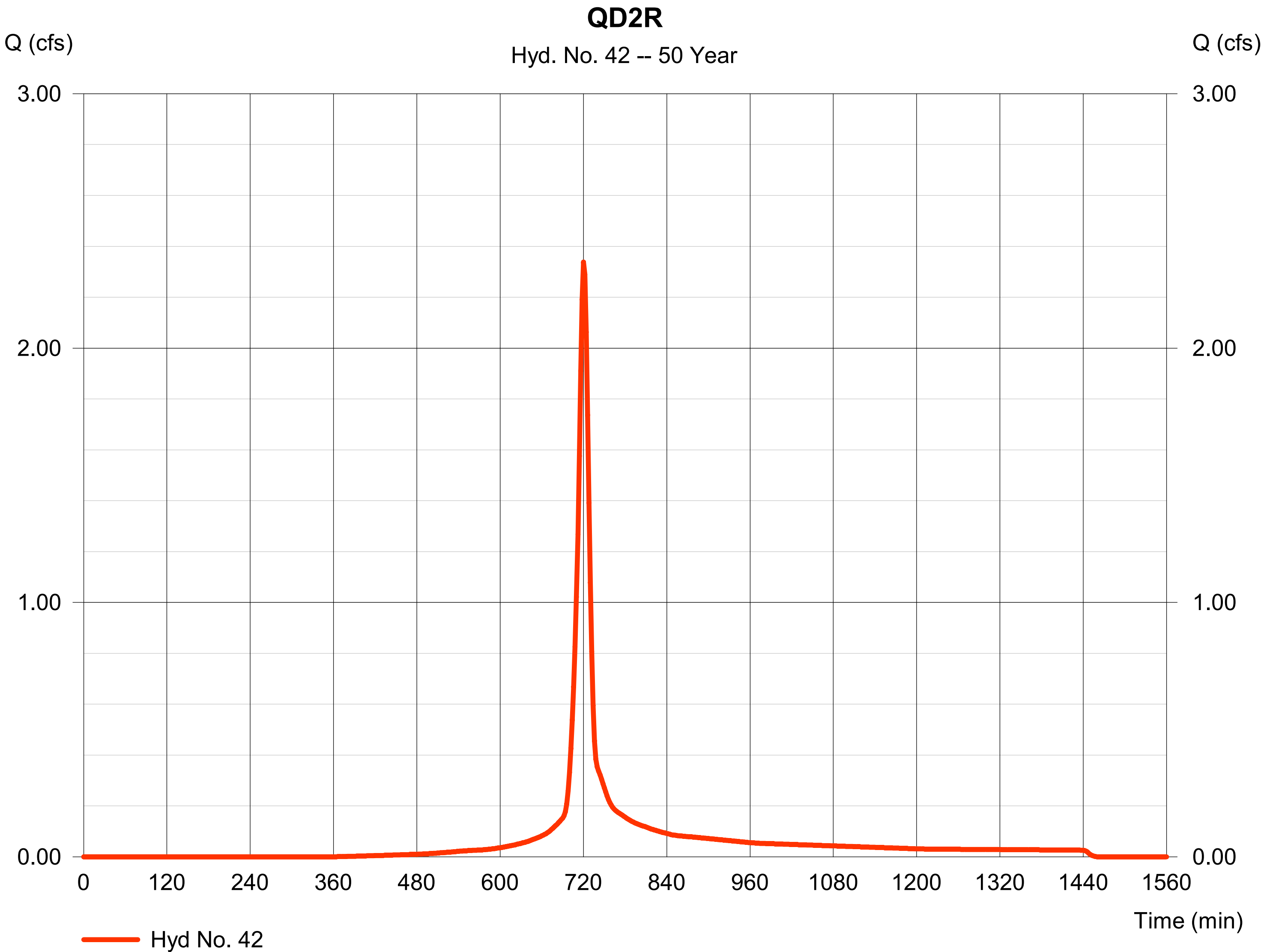
Modified Att-Kin routing method used.



Hyd. No. 42

QD2R

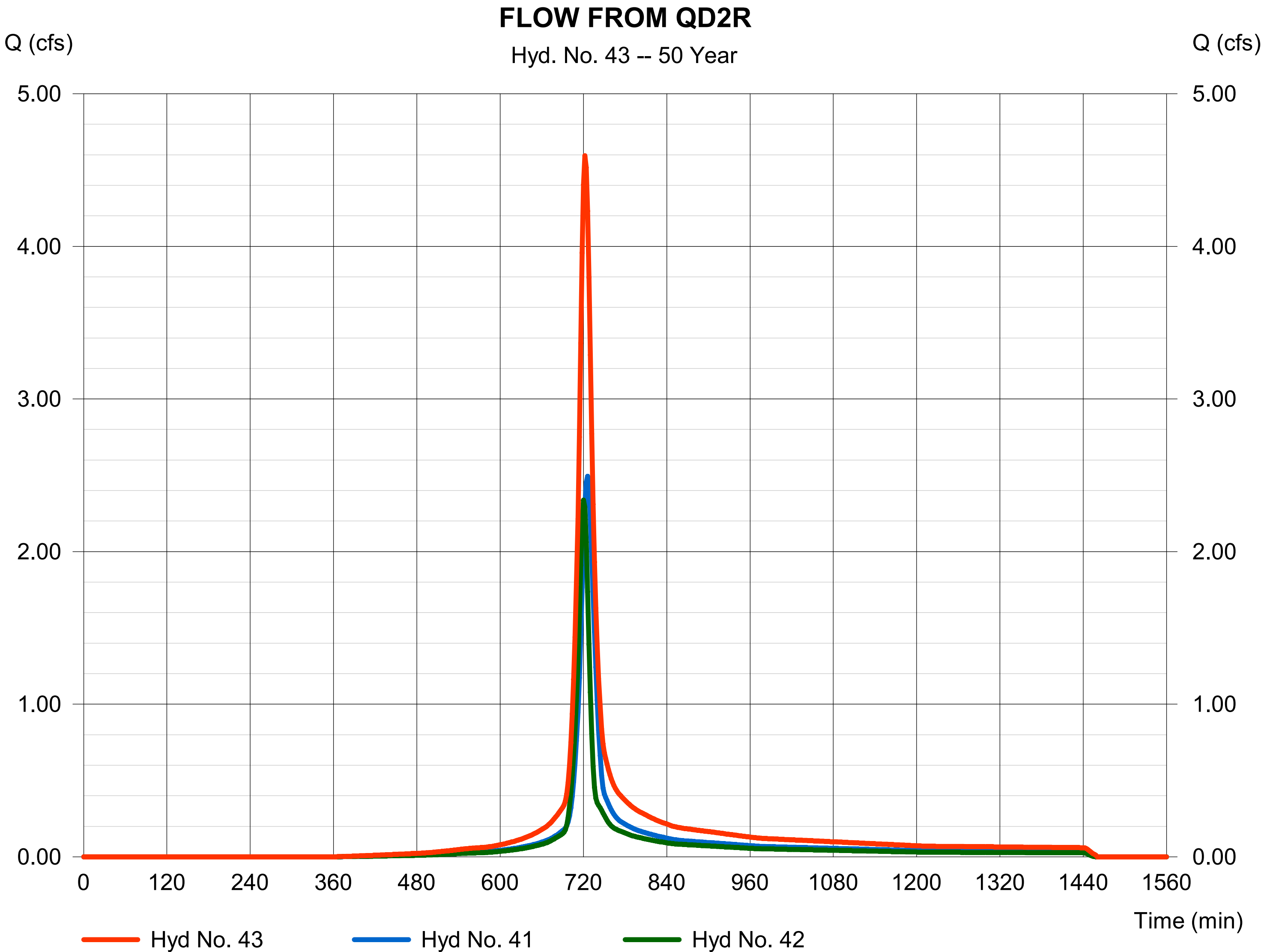
Hydrograph type	=	SCS Runoff	Peak discharge	=	2.338 cfs
Storm frequency	=	50 yrs	Time to peak	=	720 min
Time interval	=	2 min	Hyd. volume	=	6,122 cuft
Drainage area	=	0.860 ac	Curve number	=	90
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	13.10 min
Total precip.	=	2.91 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hyd. No. 43

FLOW FROM QD2R

Hydrograph type	= Combine	Peak discharge	= 4.593 cfs
Storm frequency	= 50 yrs	Time to peak	= 722 min
Time interval	= 2 min	Hyd. volume	= 13,920 cuft
Inflow hyds.	= 41, 42	Contrib. drain. area	= 0.860 ac

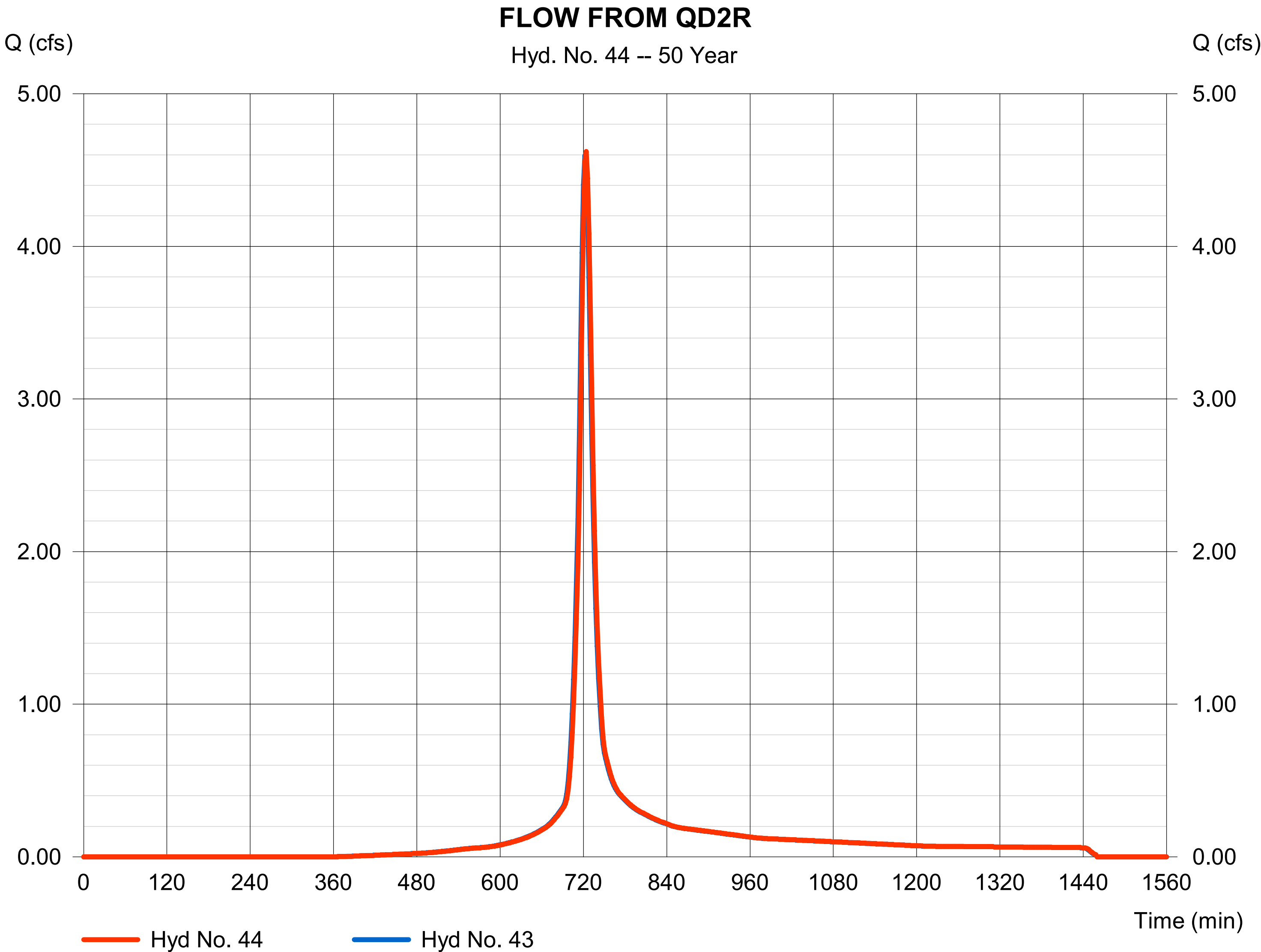


Hyd. No. 44

FLOW FROM QD2R

Hydrograph type	= Reach	Peak discharge	= 4.621 cfs
Storm frequency	= 50 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 13,919 cuft
Inflow hyd. No.	= 43 - FLOW FROM QD2R	Section type	= Circular
Reach length	= 36.3 ft	Channel slope	= 1.3 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 4.102	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.6864

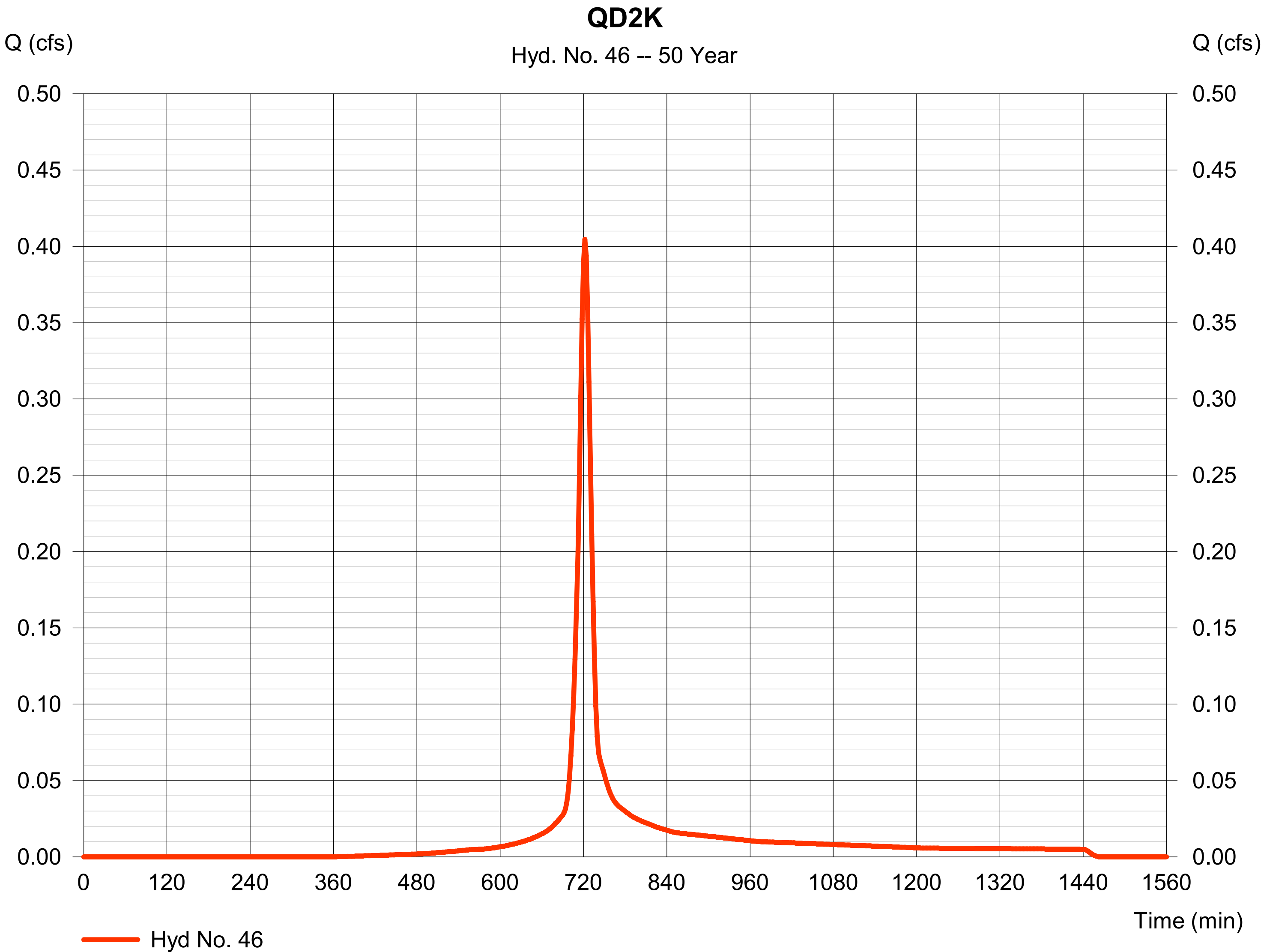
Modified Att-Kin routing method used.



Hyd. No. 46

QD2K

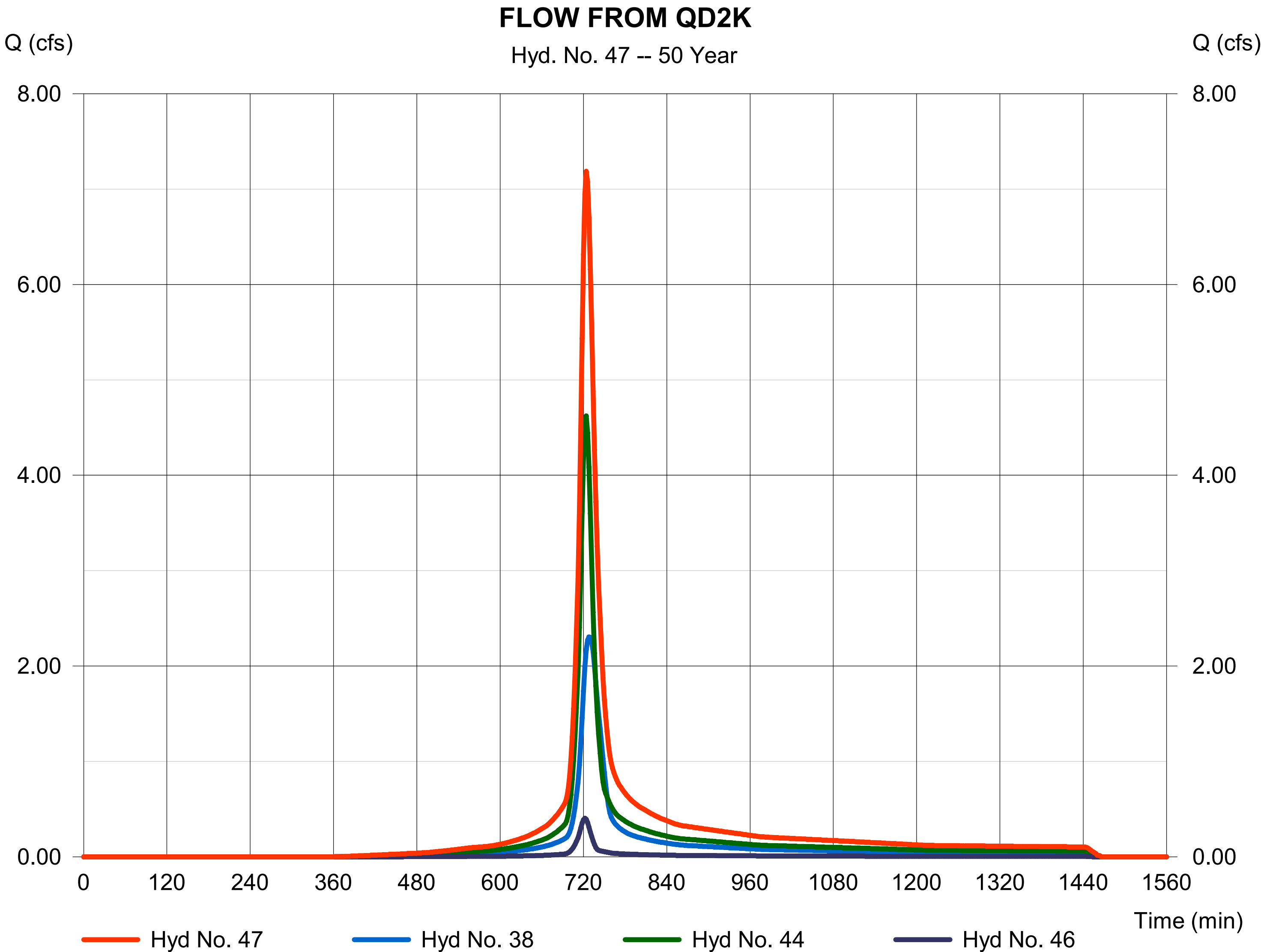
Hydrograph type	=	SCS Runoff	Peak discharge	=	0.405 cfs
Storm frequency	=	50 yrs	Time to peak	=	722 min
Time interval	=	2 min	Hyd. volume	=	1,144 cuft
Drainage area	=	0.170 ac	Curve number	=	90
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	13.80 min
Total precip.	=	2.91 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hyd. No. 47

FLOW FROM QD2K

Hydrograph type	= Combine	Peak discharge	= 7.186 cfs
Storm frequency	= 50 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 23,931 cuft
Inflow hyds.	= 38, 44, 46	Contrib. drain. area	= 0.170 ac

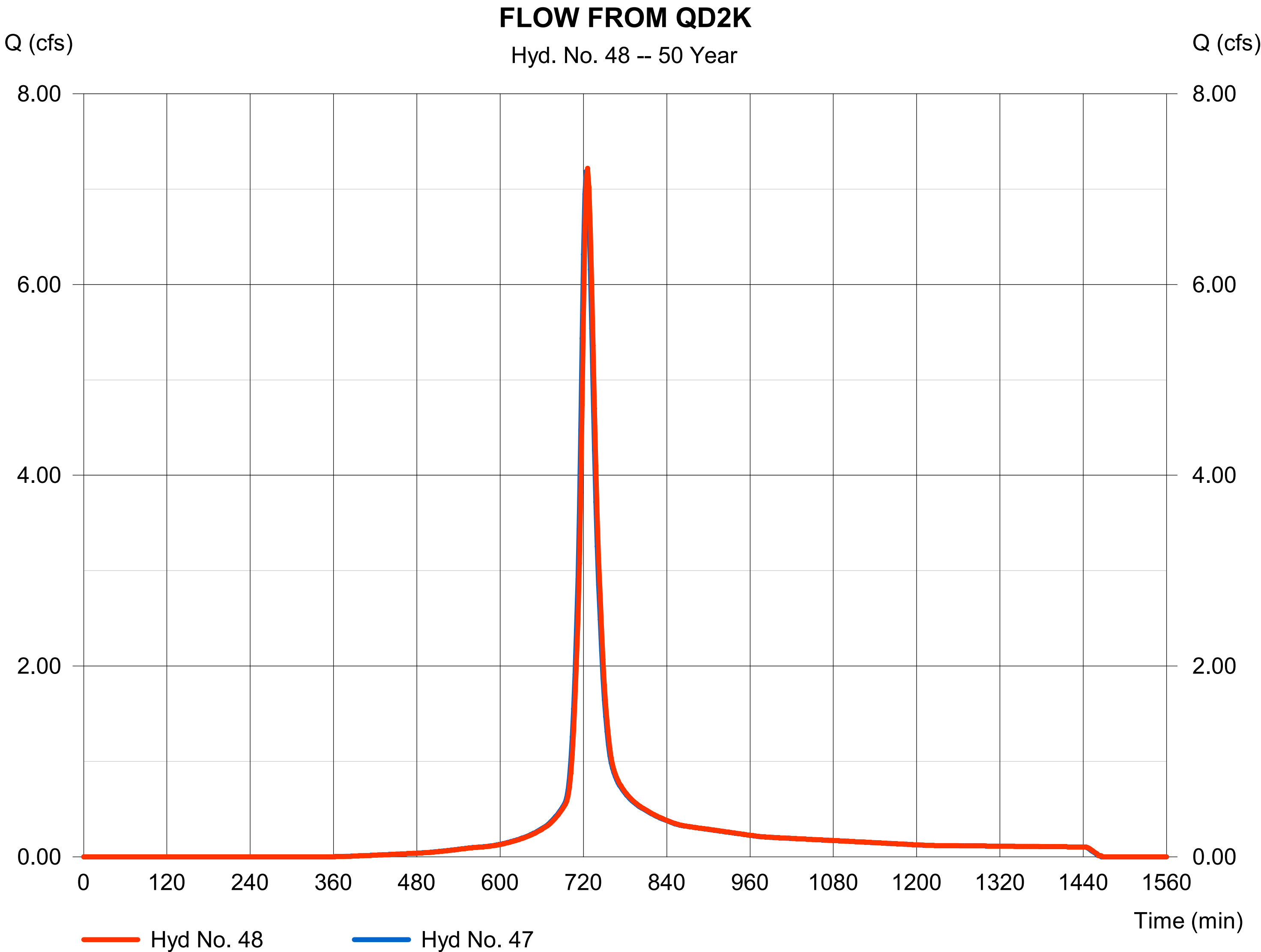


Hyd. No. 48

FLOW FROM QD2K

Hydrograph type	= Reach	Peak discharge	= 7.219 cfs
Storm frequency	= 50 yrs	Time to peak	= 726 min
Time interval	= 2 min	Hyd. volume	= 23,931 cuft
Inflow hyd. No.	= 47 - FLOW FROM QD2K	Section type	= Circular
Reach length	= 75.6 ft	Channel slope	= 0.3 %
Manning's n	= 0.024	Bottom width	= 2.0 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.060	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.3885

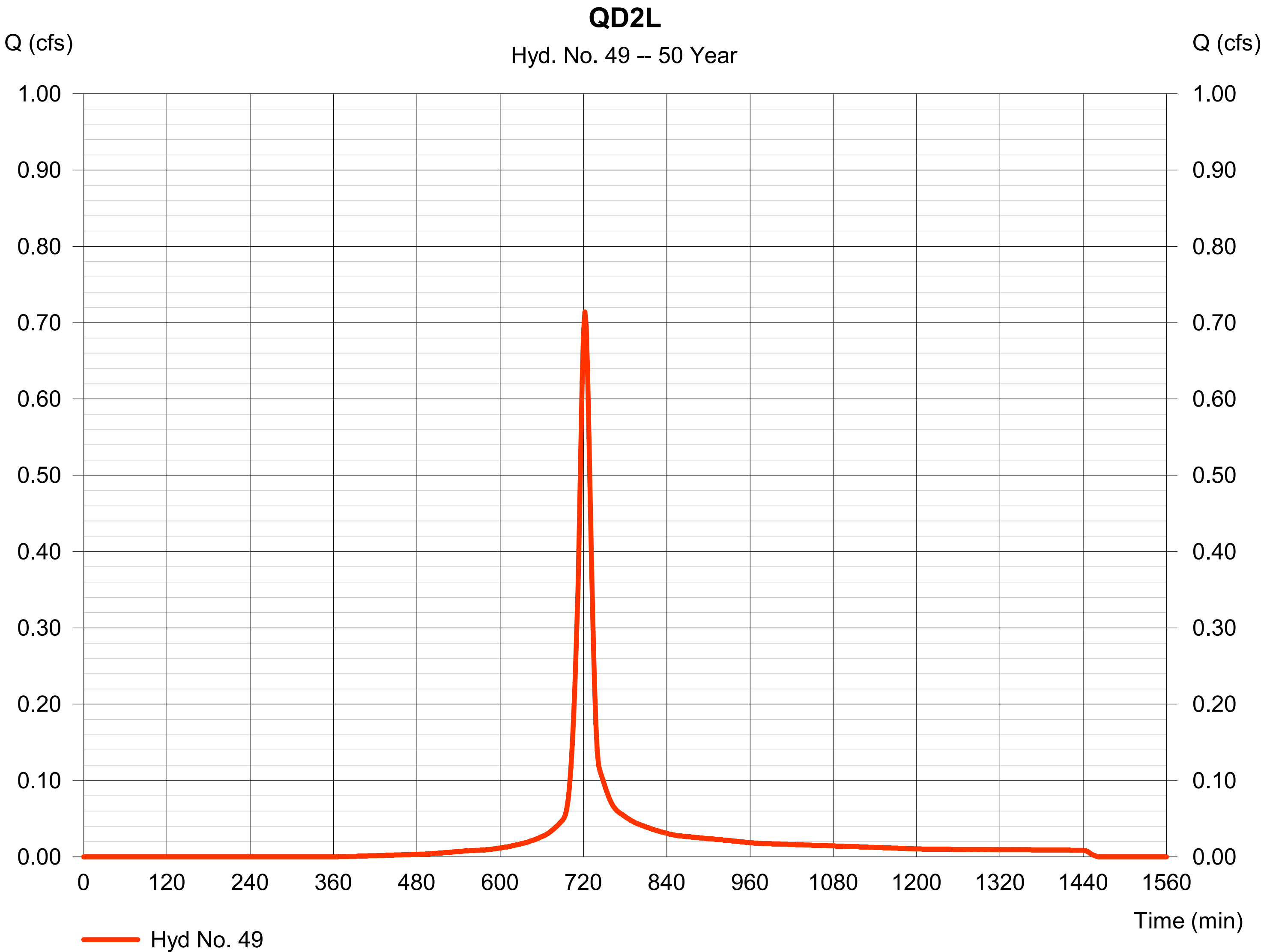
Modified Att-Kin routing method used.



Hyd. No. 49

QD2L

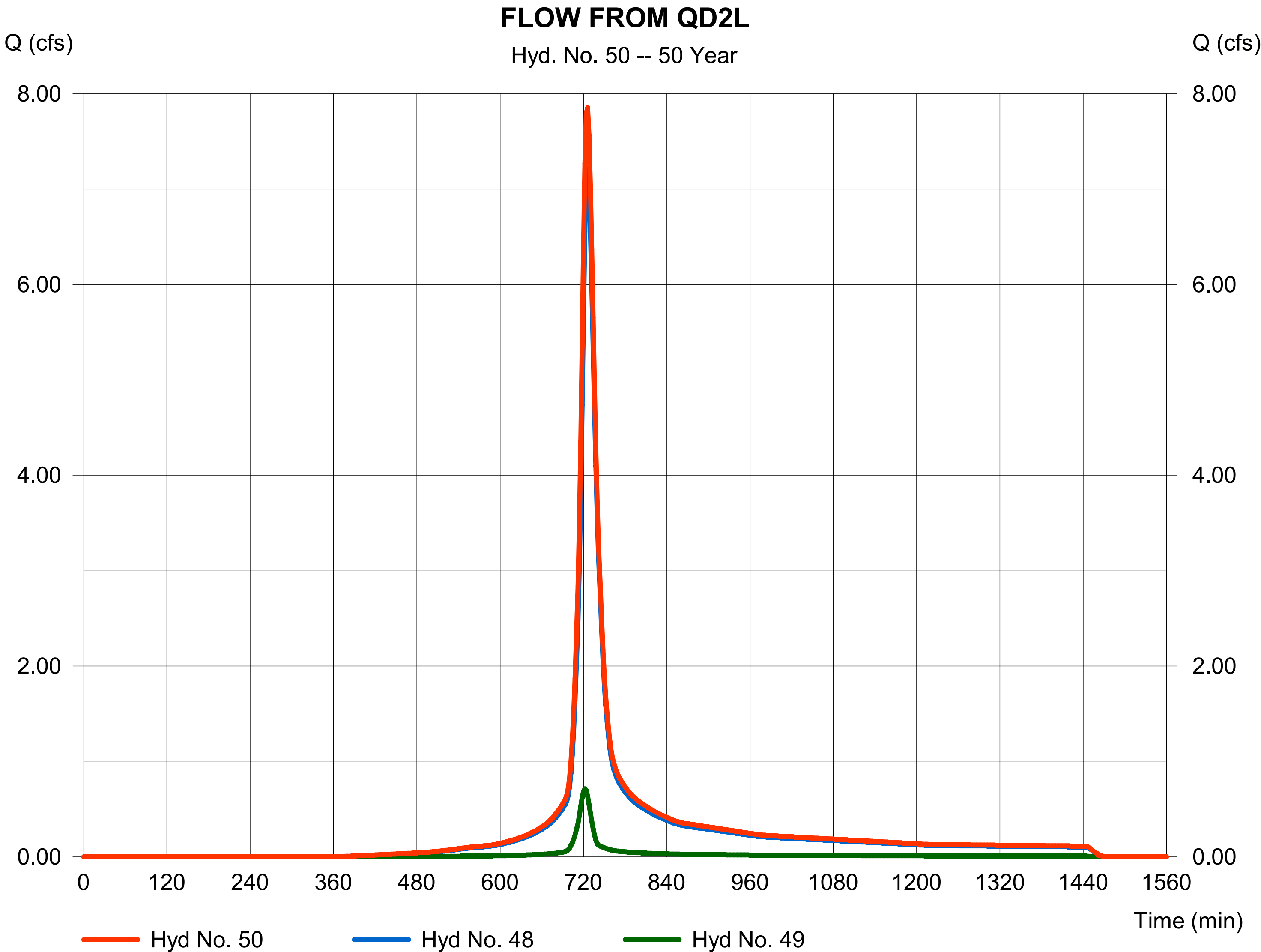
Hydrograph type	=	SCS Runoff	Peak discharge	=	0.714 cfs
Storm frequency	=	50 yrs	Time to peak	=	722 min
Time interval	=	2 min	Hyd. volume	=	2,019 cuft
Drainage area	=	0.300 ac	Curve number	=	90
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	14.30 min
Total precip.	=	2.91 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hyd. No. 50

FLOW FROM QD2L

Hydrograph type	= Combine	Peak discharge	= 7.853 cfs
Storm frequency	= 50 yrs	Time to peak	= 726 min
Time interval	= 2 min	Hyd. volume	= 25,951 cuft
Inflow hyds.	= 48, 49	Contrib. drain. area	= 0.300 ac

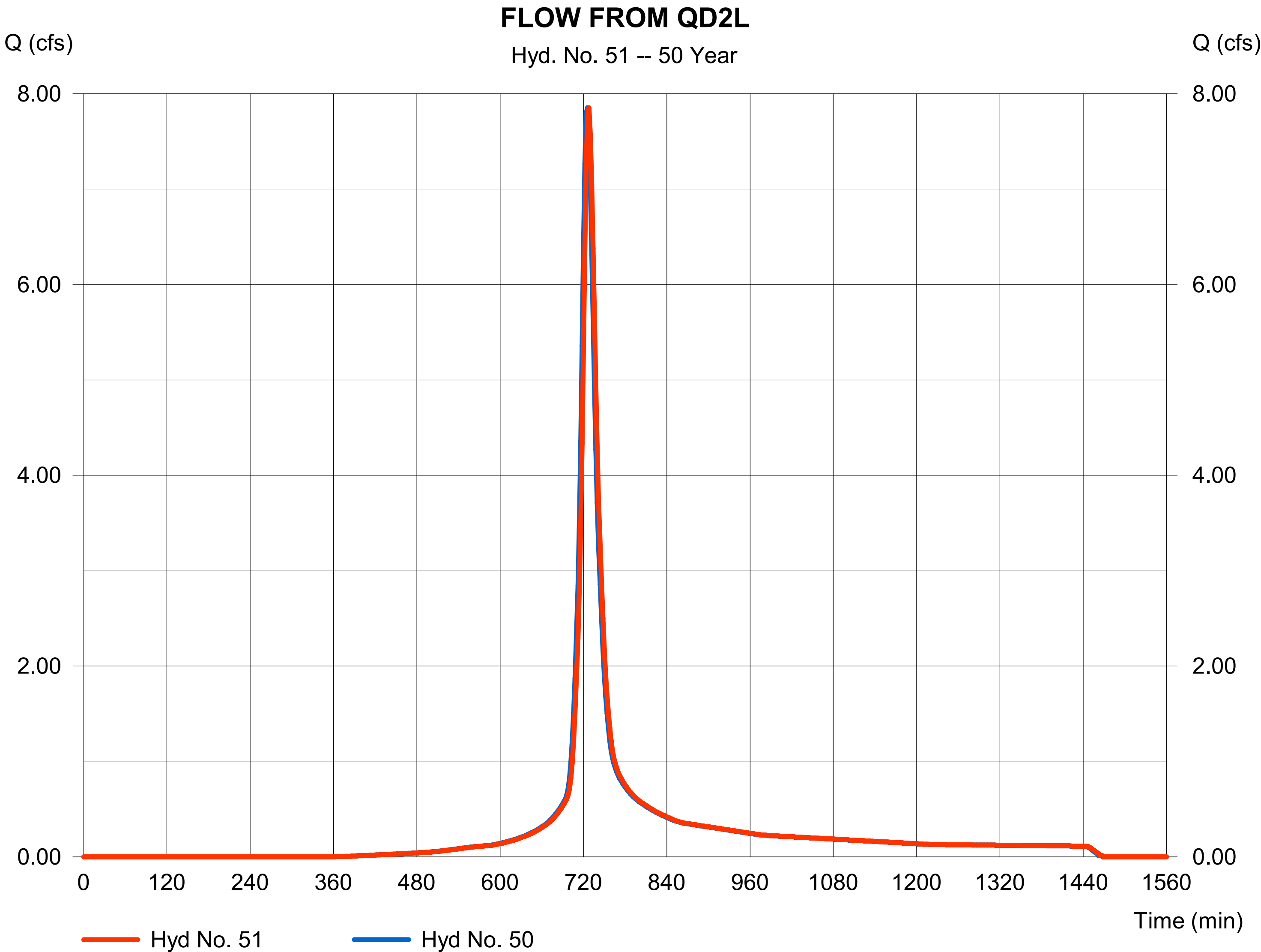


Hyd. No. 51

FLOW FROM QD2L

Hydrograph type	= Reach	Peak discharge	= 7.852 cfs
Storm frequency	= 50 yrs	Time to peak	= 728 min
Time interval	= 2 min	Hyd. volume	= 25,950 cuft
Inflow hyd. No.	= 50 - FLOW FROM QD2L	Section type	= Circular
Reach length	= 194.8 ft	Channel slope	= 0.3 %
Manning's n	= 0.024	Bottom width	= 2.0 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.060	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 0.9808

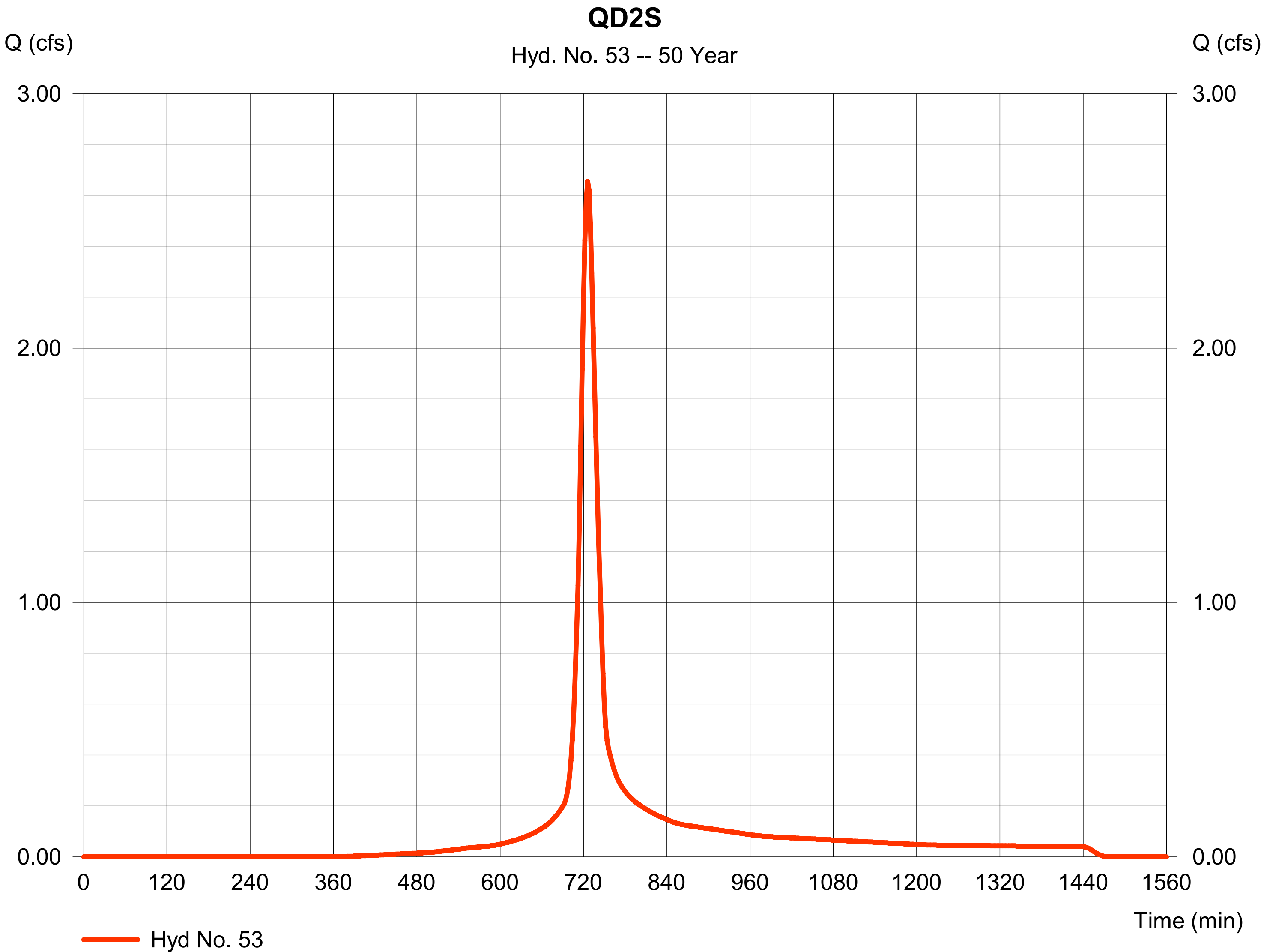
Modified Att-Kin routing method used.



Hyd. No. 53

QD2S

Hydrograph type	=	SCS Runoff	Peak discharge	=	2.656 cfs
Storm frequency	=	50 yrs	Time to peak	=	726 min
Time interval	=	2 min	Hyd. volume	=	9,204 cuft
Drainage area	=	1.310 ac	Curve number	=	90
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	23.20 min
Total precip.	=	2.91 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484

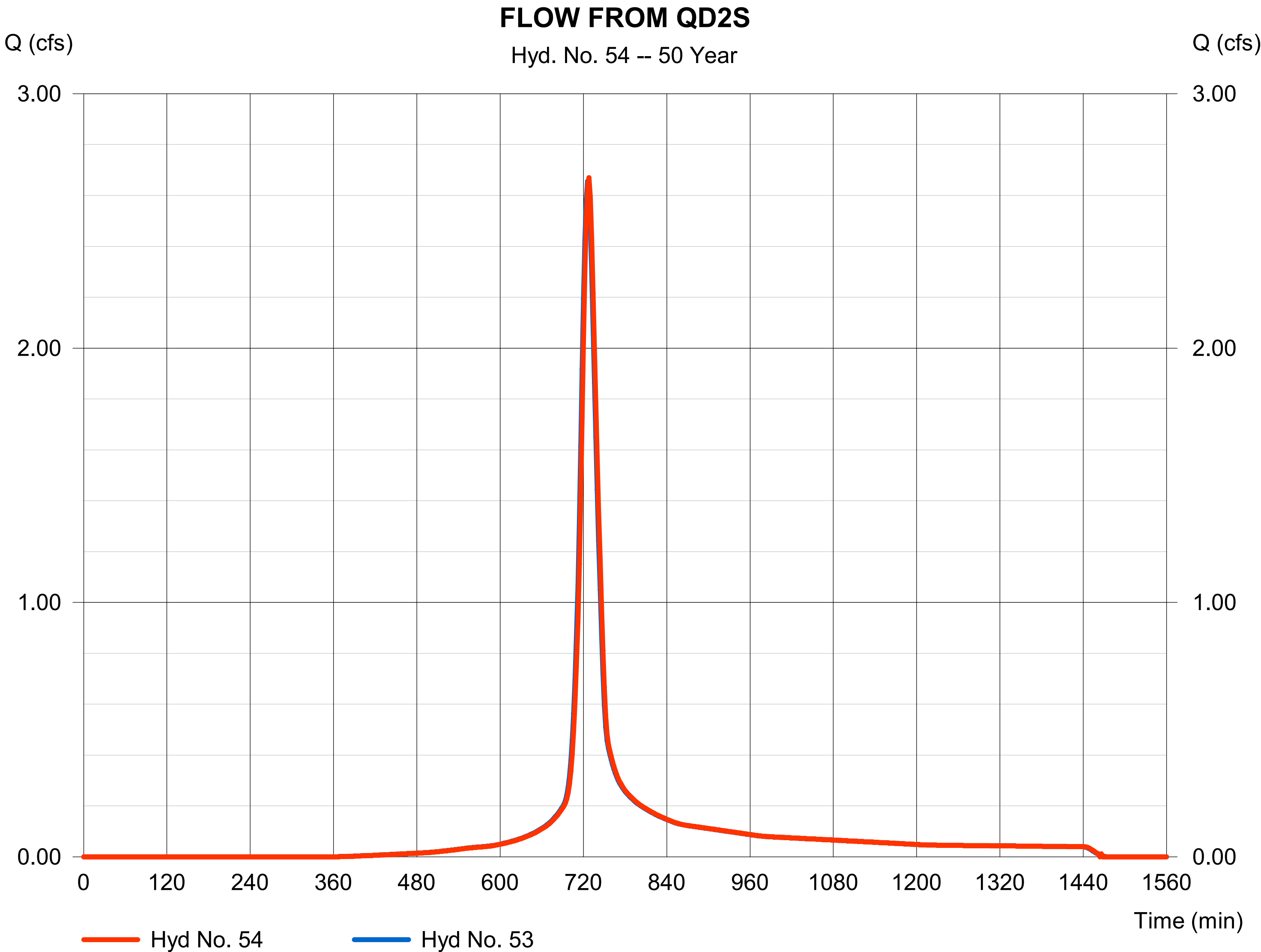


Hyd. No. 54

FLOW FROM QD2S

Hydrograph type	= Reach	Peak discharge	= 2.670 cfs
Storm frequency	= 50 yrs	Time to peak	= 728 min
Time interval	= 2 min	Hyd. volume	= 9,203 cuft
Inflow hyd. No.	= 53 - QD2S	Section type	= Circular
Reach length	= 28.7 ft	Channel slope	= 0.5 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.509	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.5945

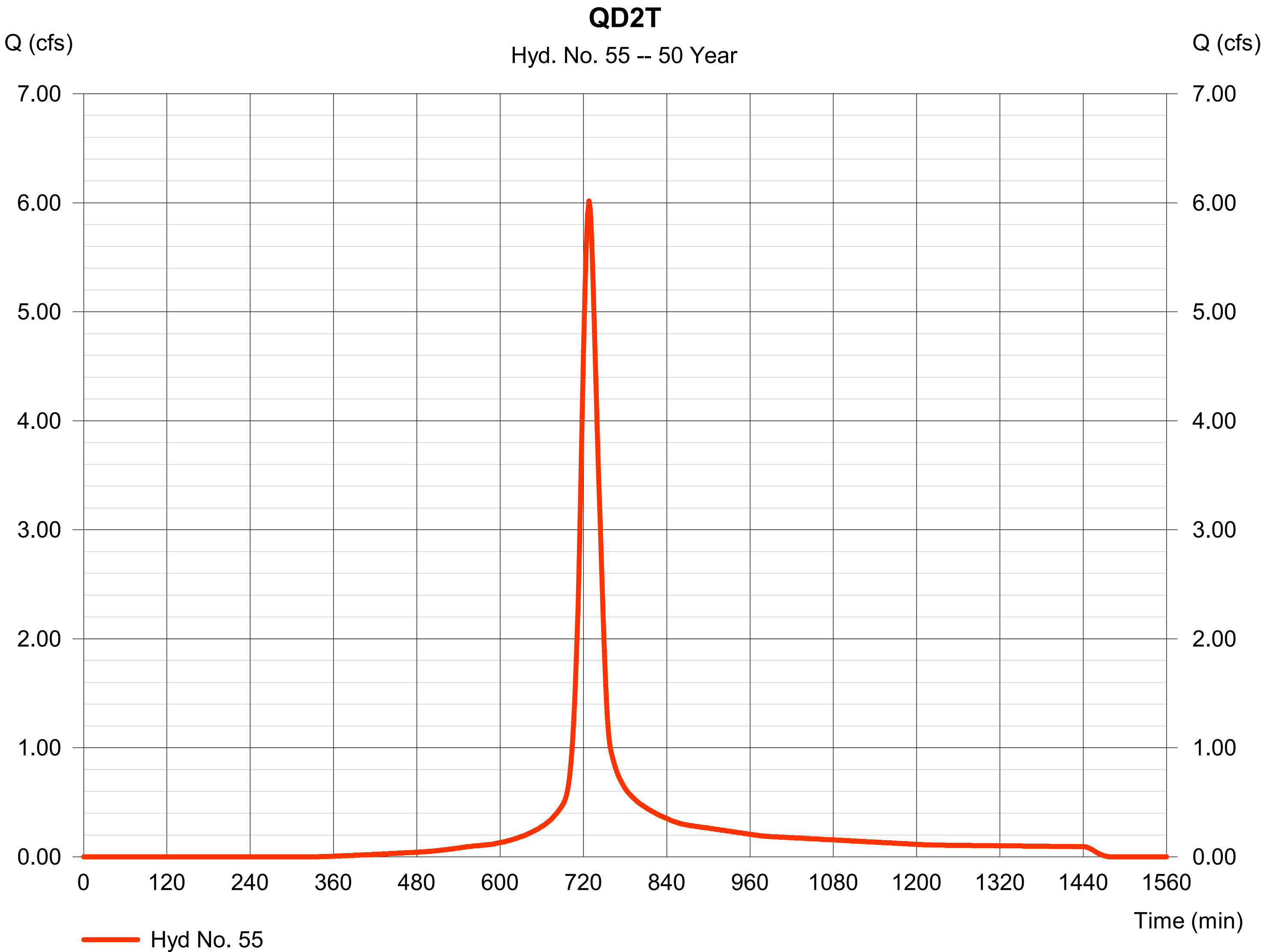
Modified Att-Kin routing method used.



Hyd. No. 55

QD2T

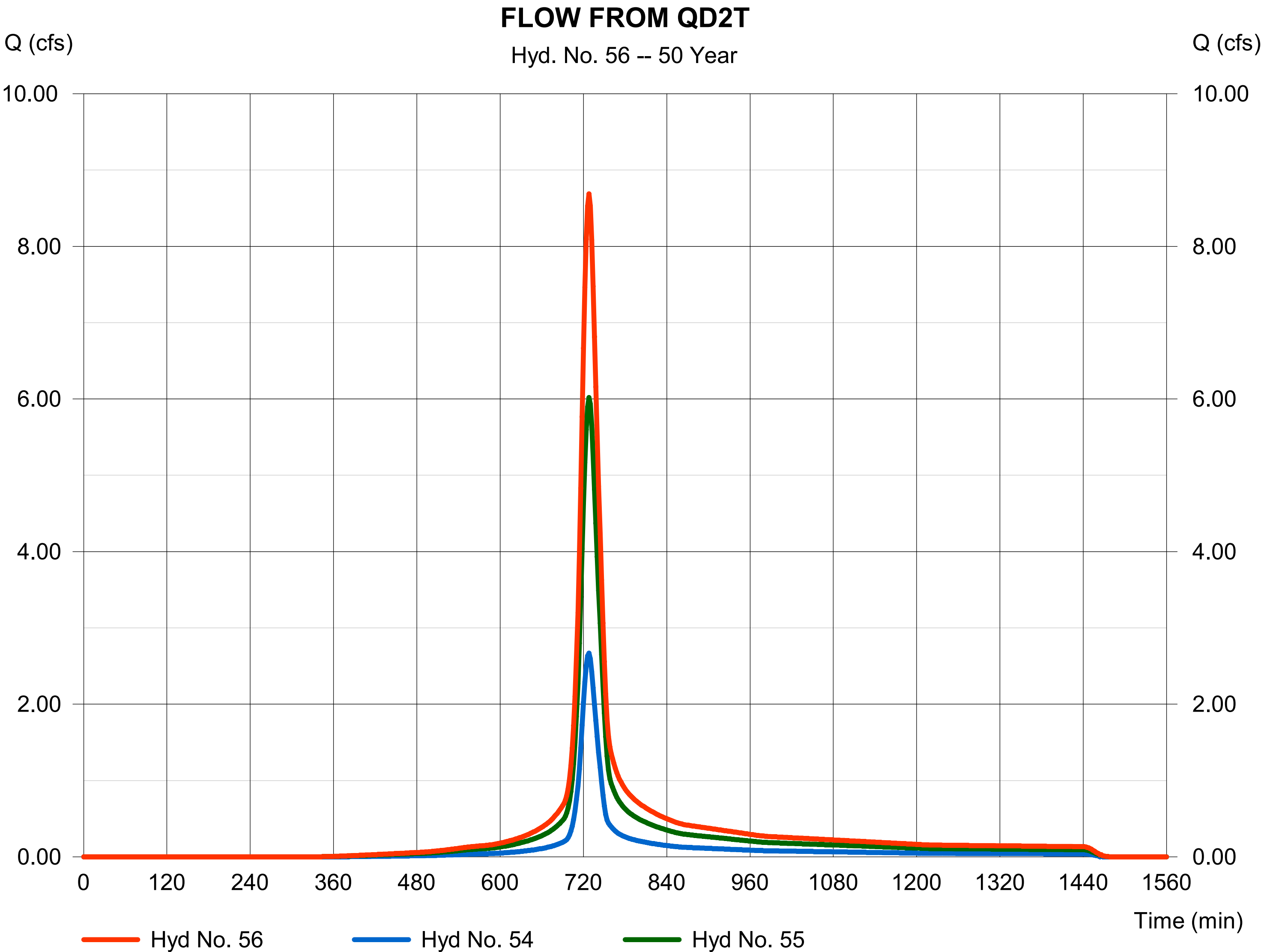
Hydrograph type	=	SCS Runoff	Peak discharge	=	6.017 cfs
Storm frequency	=	50 yrs	Time to peak	=	728 min
Time interval	=	2 min	Hyd. volume	=	22,229 cuft
Drainage area	=	3.130 ac	Curve number	=	91
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	24.20 min
Total precip.	=	2.91 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hyd. No. 56

FLOW FROM QD2T

Hydrograph type	= Combine	Peak discharge	= 8.687 cfs
Storm frequency	= 50 yrs	Time to peak	= 728 min
Time interval	= 2 min	Hyd. volume	= 31,431 cuft
Inflow hyds.	= 54, 55	Contrib. drain. area	= 3.130 ac

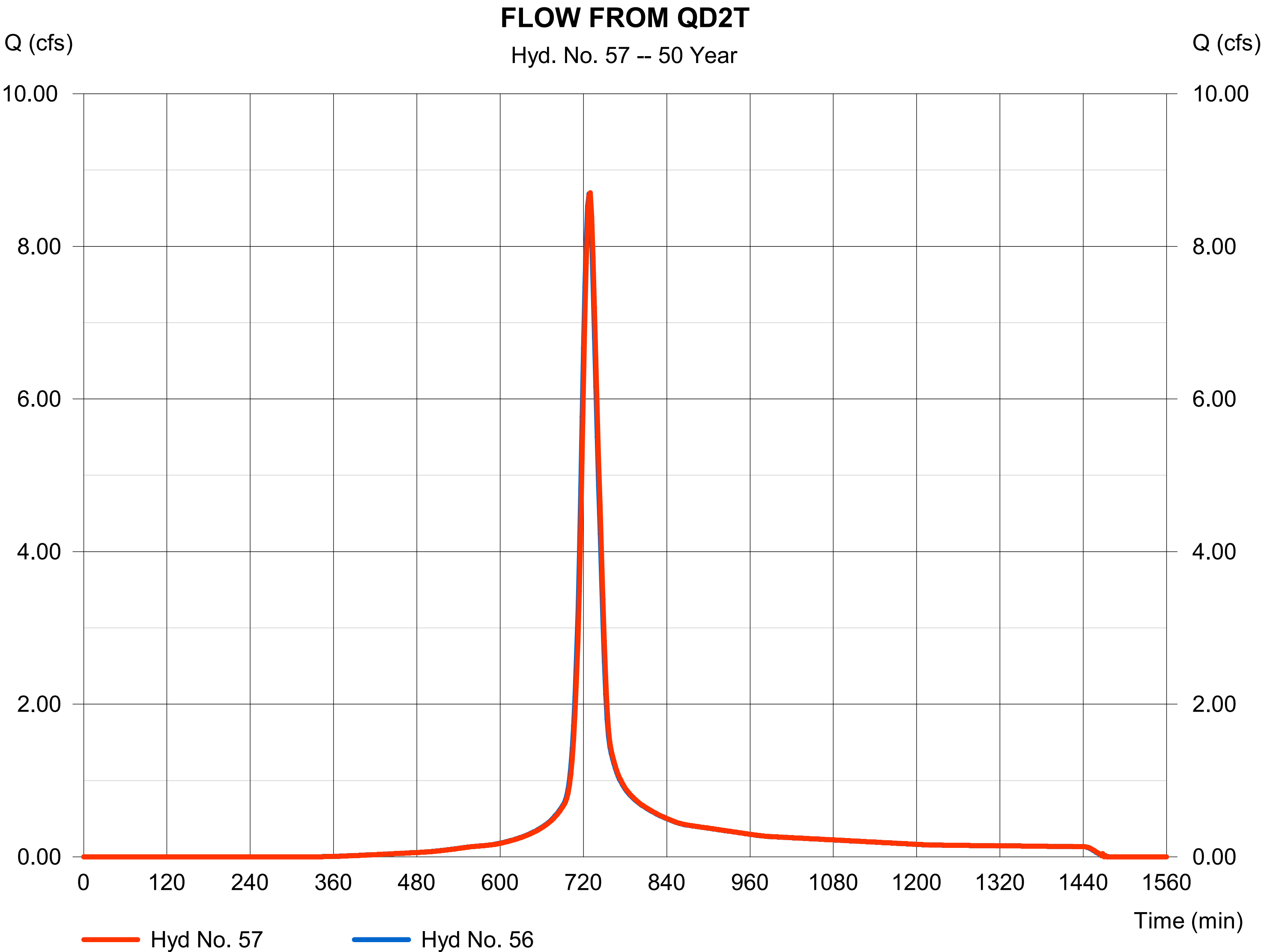


Hyd. No. 57

FLOW FROM QD2T

Hydrograph type	= Reach	Peak discharge	= 8.703 cfs
Storm frequency	= 50 yrs	Time to peak	= 730 min
Time interval	= 2 min	Hyd. volume	= 31,431 cuft
Inflow hyd. No.	= 56 - FLOW FROM QD2T	Section type	= Circular
Reach length	= 55.6 ft	Channel slope	= 0.5 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.509	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.7378

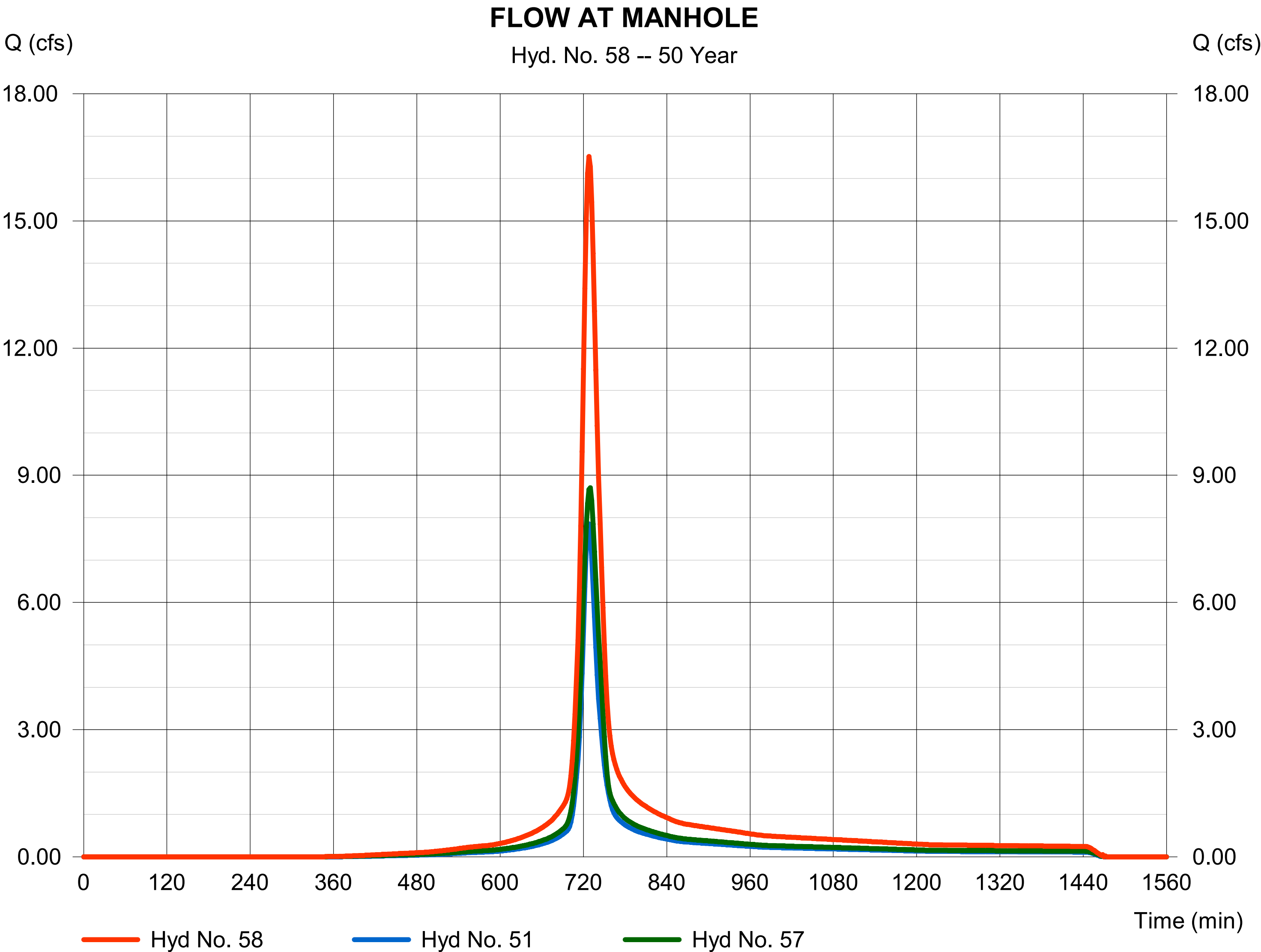
Modified Att-Kin routing method used.



Hyd. No. 58

FLOW AT MANHOLE

Hydrograph type	= Combine	Peak discharge	= 16.52 cfs
Storm frequency	= 50 yrs	Time to peak	= 728 min
Time interval	= 2 min	Hyd. volume	= 57,382 cuft
Inflow hyds.	= 51, 57	Contrib. drain. area	= 0.000 ac

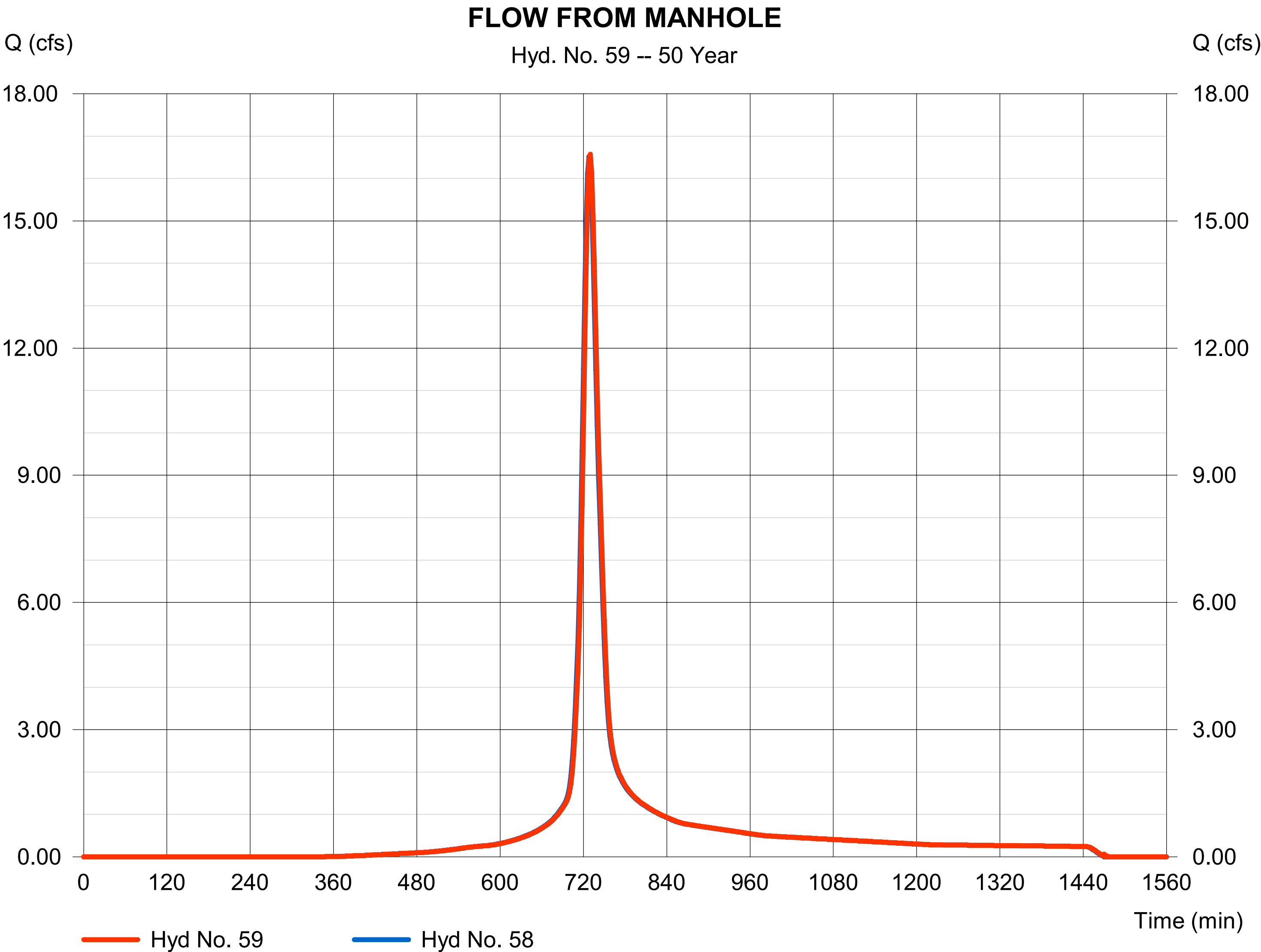


Hyd. No. 59

FLOW FROM MANHOLE

Hydrograph type	= Reach	Peak discharge	= 16.57 cfs
Storm frequency	= 50 yrs	Time to peak	= 730 min
Time interval	= 2 min	Hyd. volume	= 57,385 cuft
Inflow hyd. No.	= 58 - FLOW AT MANHOLE	Section type	= Circular
Reach length	= 141.5 ft	Channel slope	= 0.3 %
Manning's n	= 0.024	Bottom width	= 2.0 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.060	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.4718

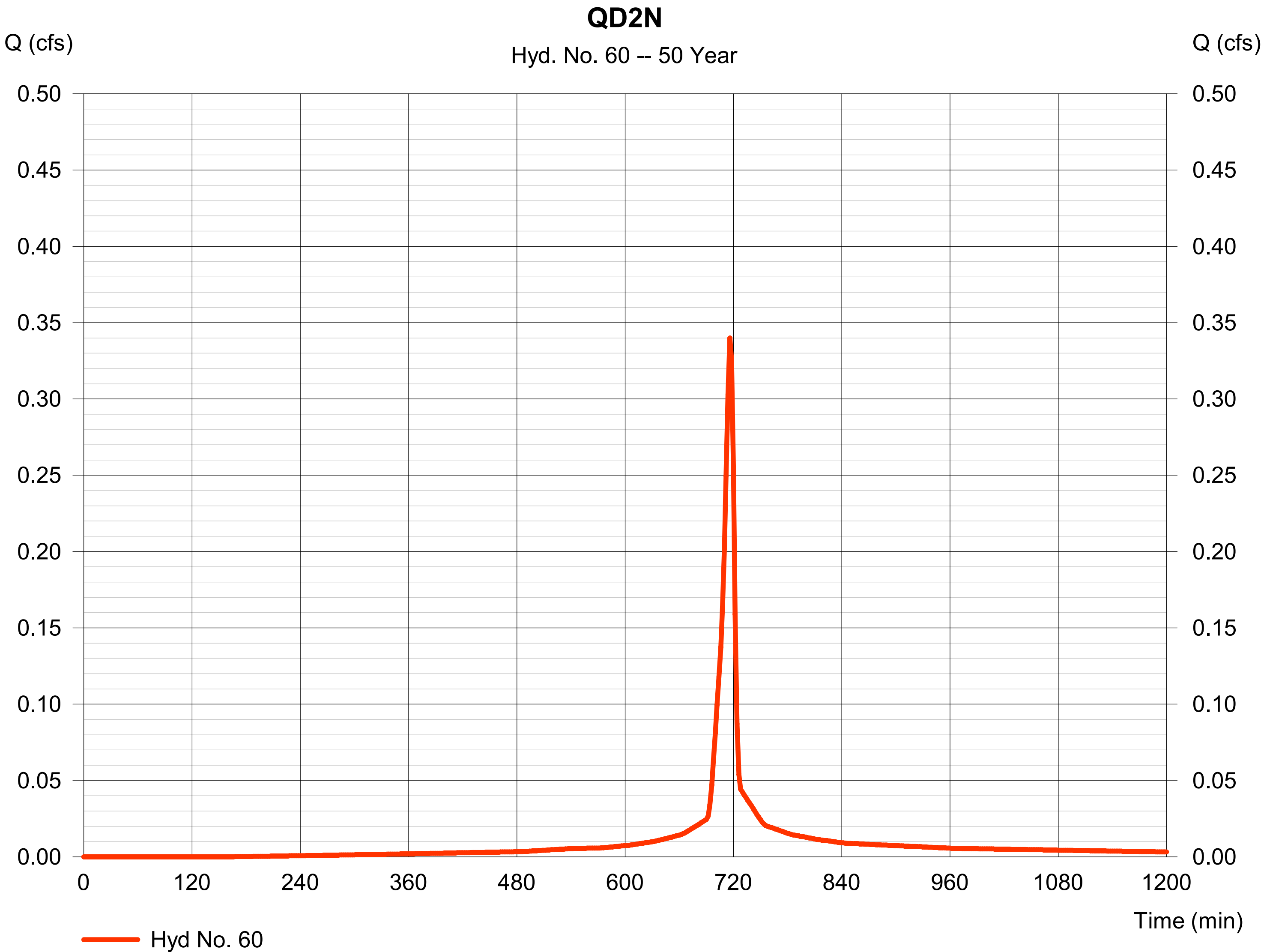
Modified Att-Kin routing method used.



Hyd. No. 60

QD2N

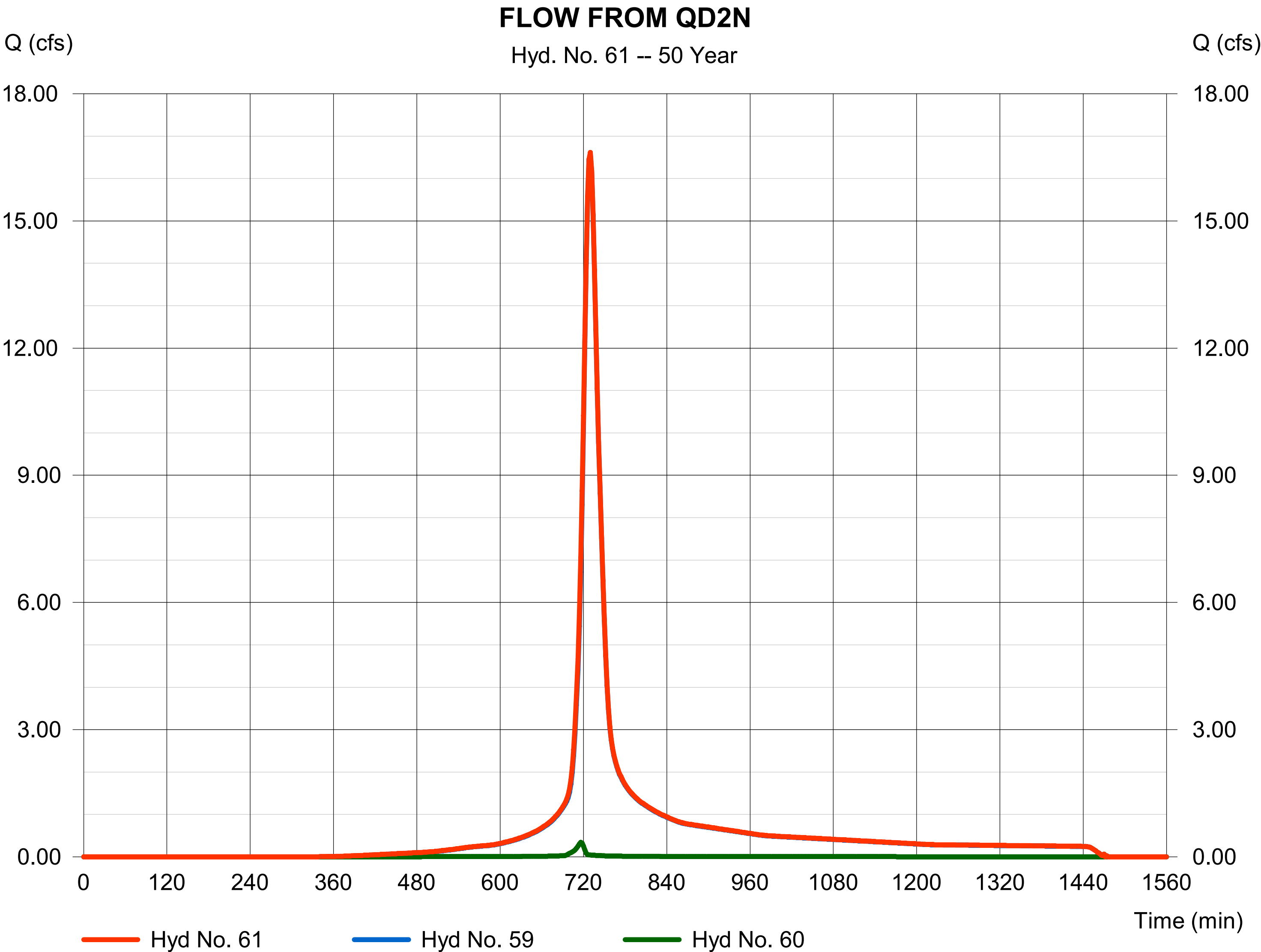
Hydrograph type	= SCS Runoff	Peak discharge	= 0.340 cfs
Storm frequency	= 50 yrs	Time to peak	= 716 min
Time interval	= 2 min	Hyd. volume	= 755 cuft
Drainage area	= 0.090 ac	Curve number	= 96
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 6.00 min
Total precip.	= 2.91 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hyd. No. 61

FLOW FROM QD2N

Hydrograph type	= Combine	Peak discharge	= 16.61 cfs
Storm frequency	= 50 yrs	Time to peak	= 730 min
Time interval	= 2 min	Hyd. volume	= 58,140 cuft
Inflow hyds.	= 59, 60	Contrib. drain. area	= 0.090 ac

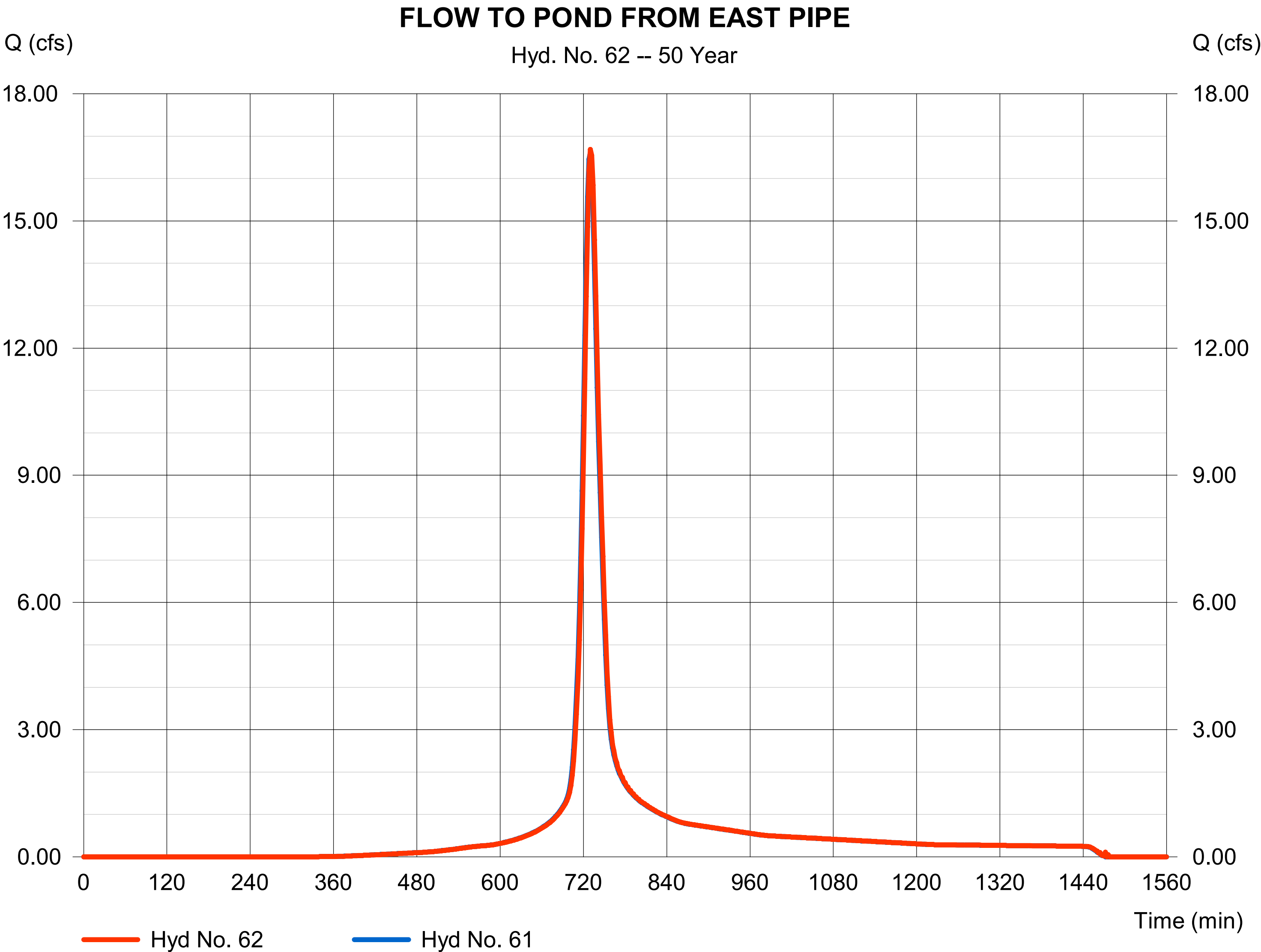


Hyd. No. 62

FLOW TO POND FROM EAST PIPE

Hydrograph type	= Reach	Peak discharge	= 16.69 cfs
Storm frequency	= 50 yrs	Time to peak	= 730 min
Time interval	= 2 min	Hyd. volume	= 58,149 cuft
Inflow hyd. No.	= 61 - FLOW FROM QD2N	Section type	= Circular
Reach length	= 23.1 ft	Channel slope	= 0.3 %
Manning's n	= 0.024	Bottom width	= 2.0 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.060	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.8900

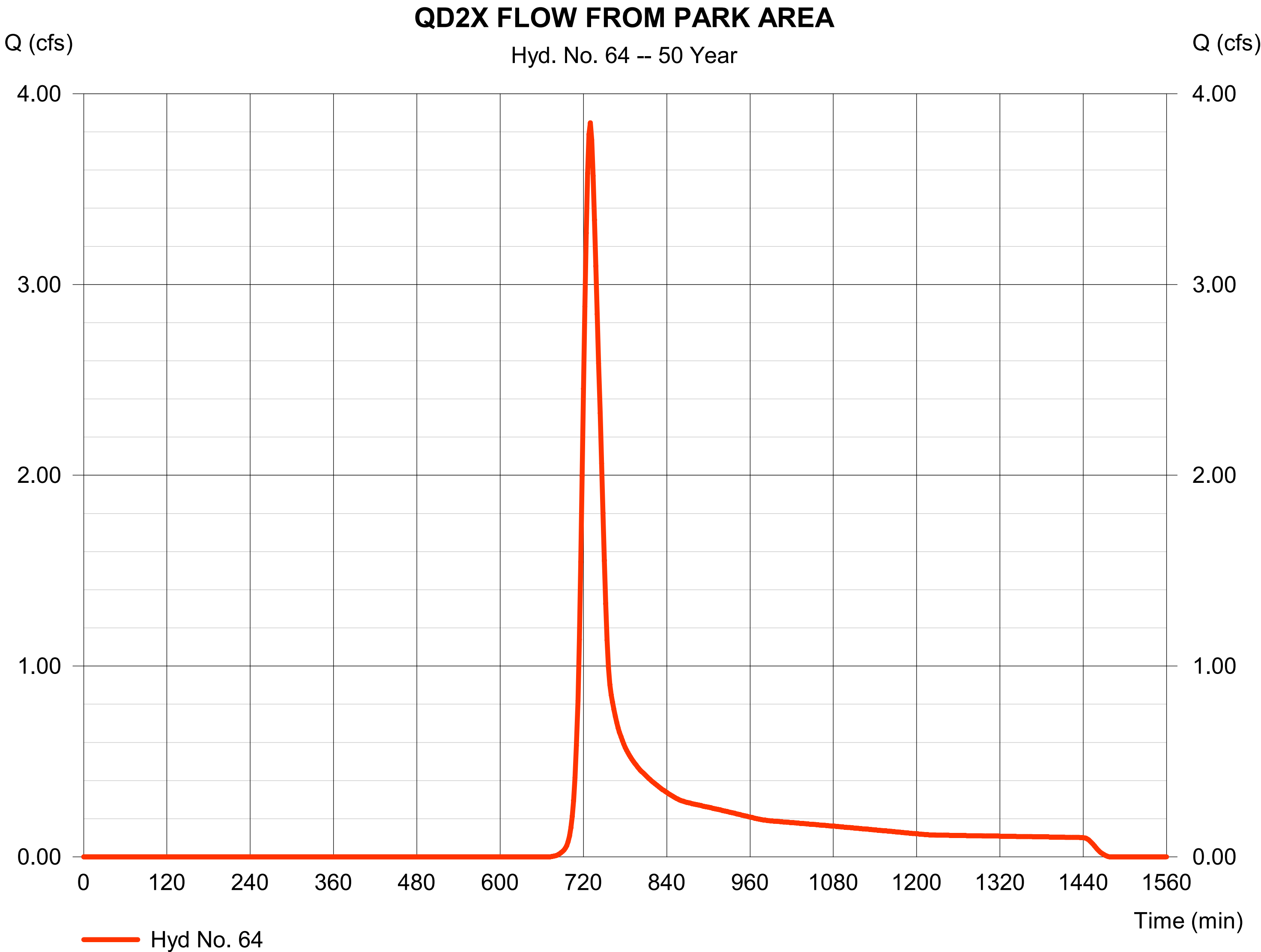
Modified Att-Kin routing method used.



Hyd. No. 64

QD2X FLOW FROM PARK AREA

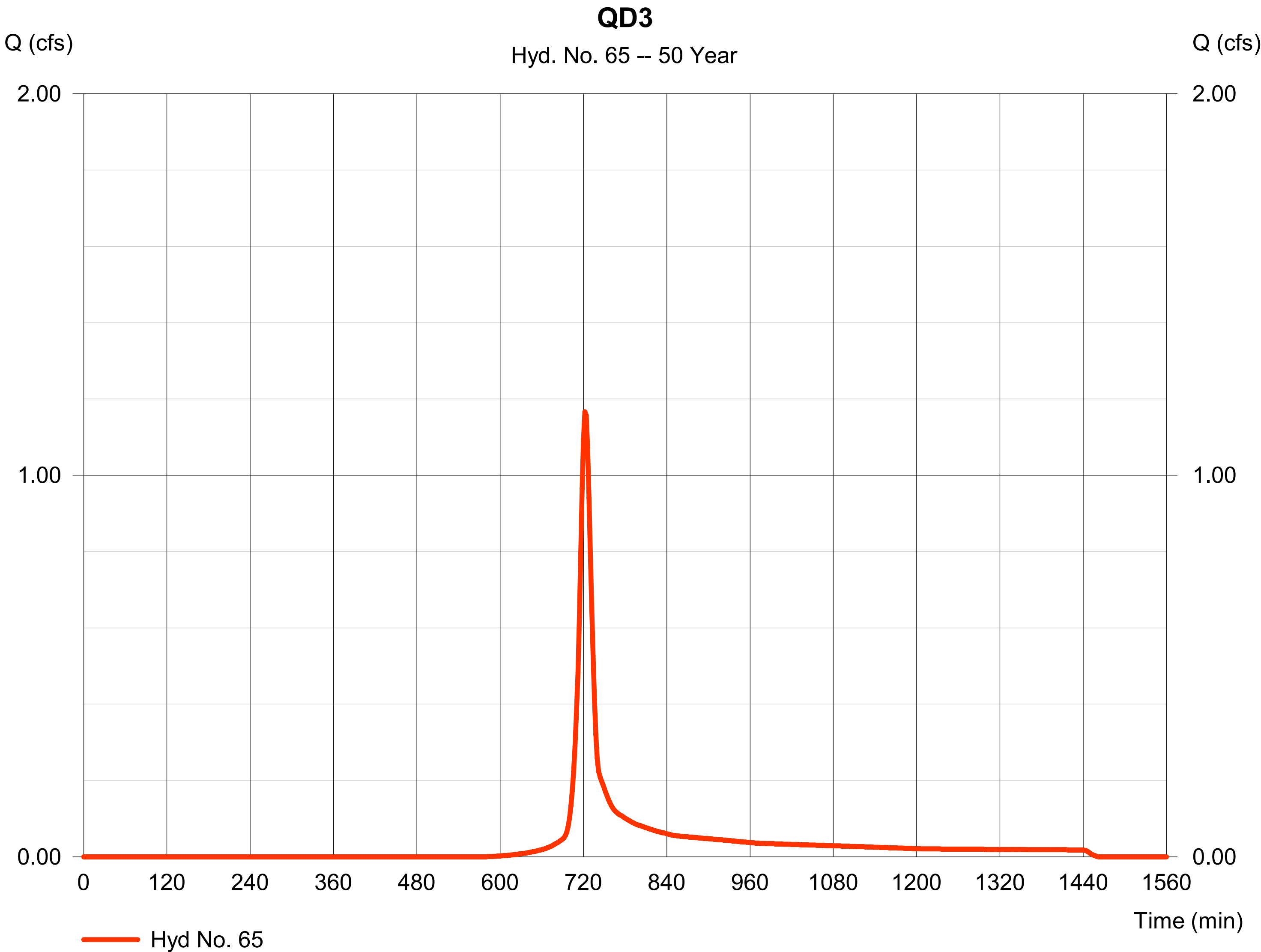
Hydrograph type	=	SCS Runoff	Peak discharge	=	3.847 cfs
Storm frequency	=	50 yrs	Time to peak	=	730 min
Time interval	=	2 min	Hyd. volume	=	15,368 cuft
Drainage area	=	5.050 ac	Curve number	=	74
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	25.00 min
Total precip.	=	2.91 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hyd. No. 65

QD3

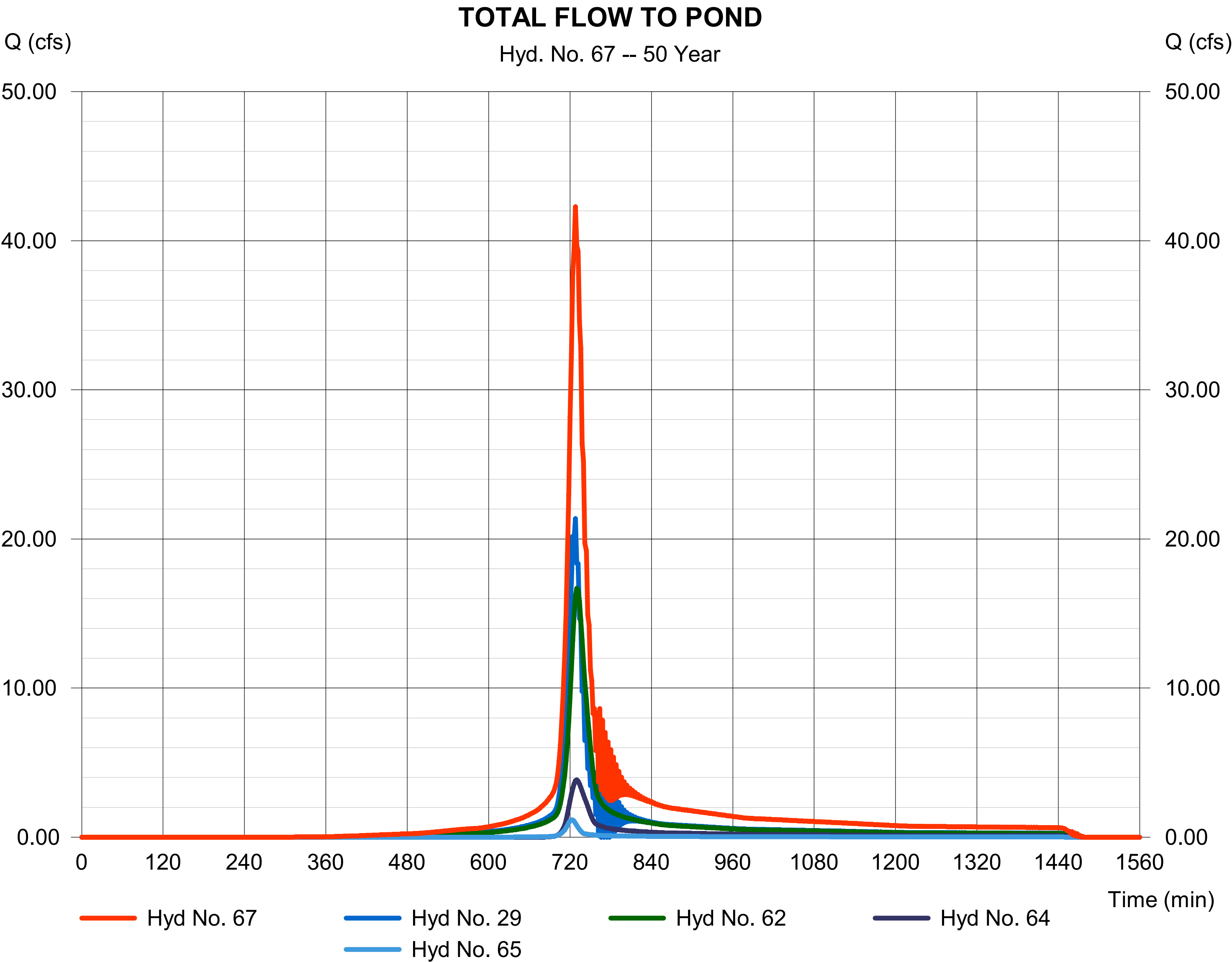
Hydrograph type	=	SCS Runoff	Peak discharge	=	1.166 cfs
Storm frequency	=	50 yrs	Time to peak	=	722 min
Time interval	=	2 min	Hyd. volume	=	3,304 cuft
Drainage area	=	0.750 ac	Curve number	=	81
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	14.30 min
Total precip.	=	2.91 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hyd. No. 67

TOTAL FLOW TO POND

Hydrograph type	= Combine	Peak discharge	= 42.27 cfs
Storm frequency	= 50 yrs	Time to peak	= 728 min
Time interval	= 2 min	Hyd. volume	= 141,142 cuft
Inflow hyds.	= 29, 62, 64, 65	Contrib. drain. area	= 5.800 ac

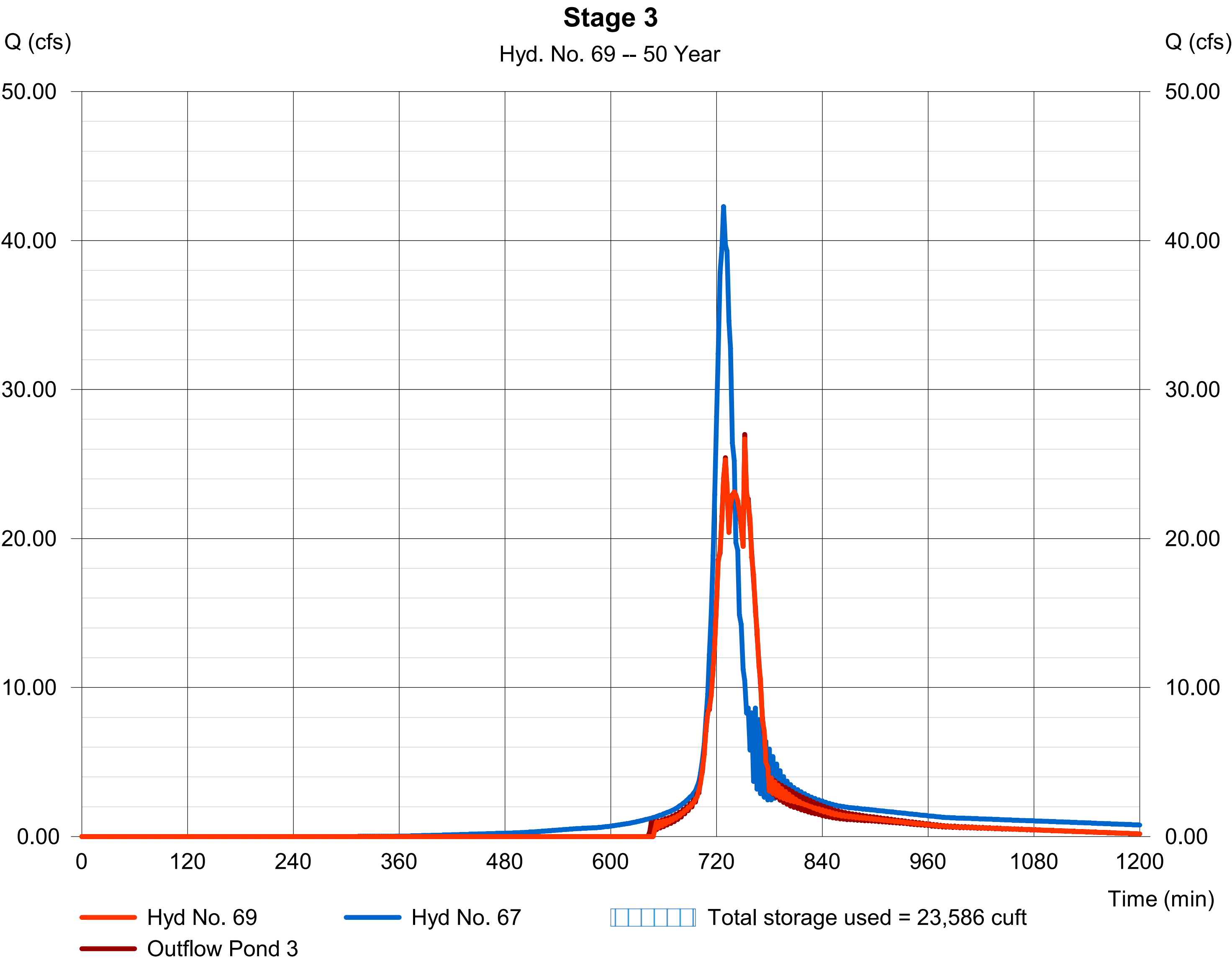


Hyd. No. 69

Stage 3

Hydrograph type	= Reservoir (Interconnected)	Peak discharge	= 26.68 cfs
Storm frequency	= 50 yrs	Time to peak	= 752 min
Time interval	= 2 min	Hyd. volume	= 104,632 cuft
Upper Pond	= Stage 3	Lower Pond	= outfall
Inflow hyd.	= 67 - TOTAL FLOW TO POND	Other Inflow hyd.	= None
Max. Elevation	= 6476.25 ft	Max. Elevation	= 6475.12 ft
Max. Storage	= 23,296 cuft	Max. Storage	= 289 cuft

Interconnected Pond Routing. Storage Indication method used.



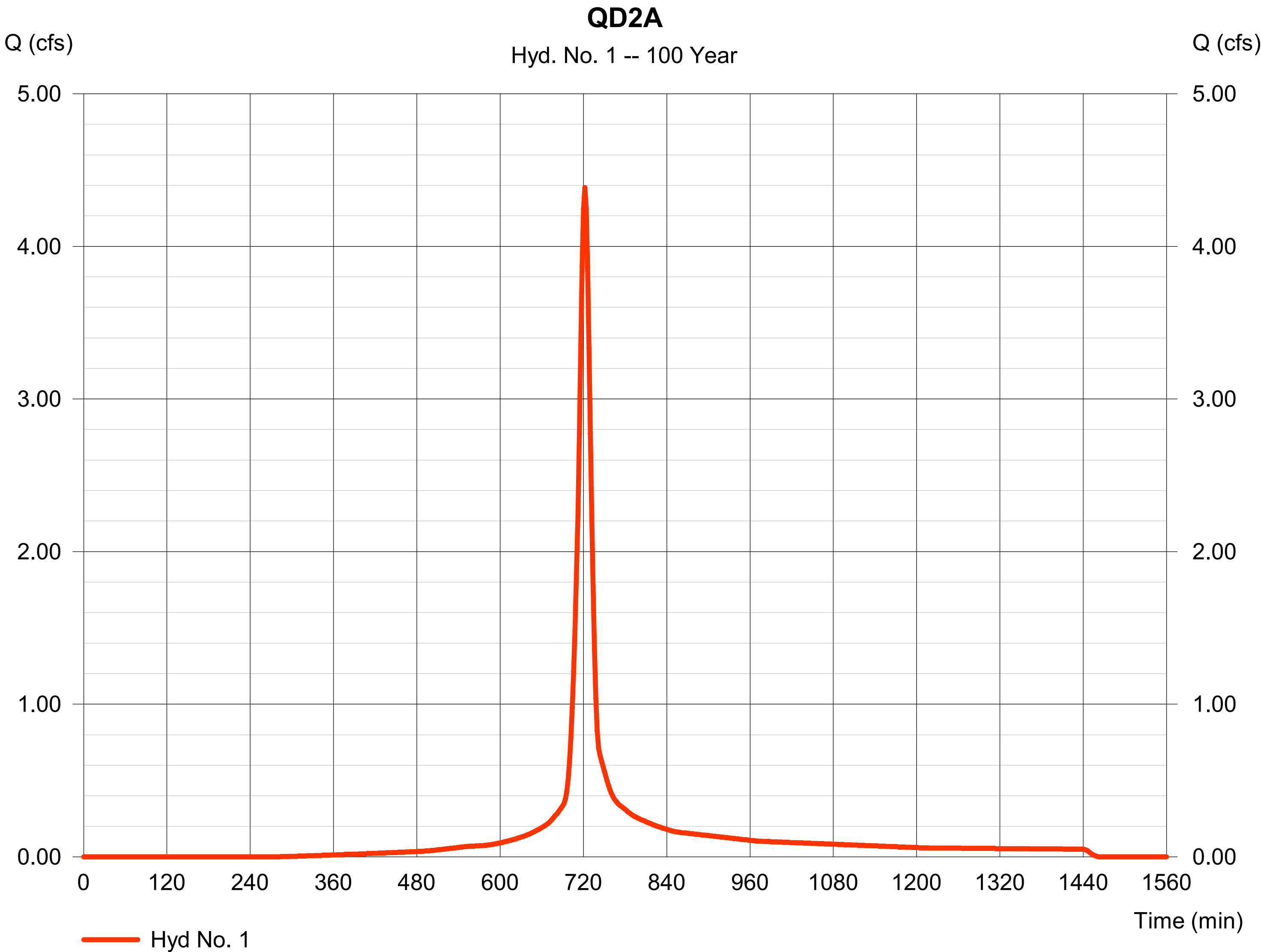
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	4.386	2	722	12,631	-----	-----	-----	QD2A
2	Reach	4.403	2	724	12,629	1	-----	-----	FLOW FROM QD2A
3	SCS Runoff	2.096	2	724	6,618	-----	-----	-----	QD2B
4	Combine	6.500	2	724	19,246	2, 3	-----	-----	FLOW FROM QD2B
5	Reach	6.533	2	724	19,245	4	-----	-----	FLOW FROM QD2B
6	SCS Runoff	2.131	2	718	5,242	-----	-----	-----	QD2C
7	Combine	7.965	2	722	24,487	5, 6	-----	-----	FLOW FROM QD2C
8	Reach	8.002	2	724	24,487	7	-----	-----	FLOW FROM QD2C
9	SCS Runoff	3.921	2	724	12,523	-----	-----	-----	QD2D
10	Combine	11.92	2	724	37,009	8, 9	-----	-----	FLOW FROM QD2D
11	Reach	11.93	2	724	37,008	10	-----	-----	FLOW FROM QD2D
12	SCS Runoff	3.280	2	720	8,633	-----	-----	-----	QD2E
13	Reach	3.311	2	722	8,631	12	-----	-----	FLOW FROM QD2E
14	SCS Runoff	2.015	2	716	4,186	-----	-----	-----	QD2F
15	Combine	4.748	2	718	12,817	13, 14	-----	-----	FLOW FROM QD2F
16	Reach	4.844	2	720	12,818	15	-----	-----	FLOW FROM QD2F
18	SCS Runoff	2.374	2	720	6,249	-----	-----	-----	QD2G
19	Combine	18.17	2	722	56,075	11, 16, 18	-----	-----	FLOW FROM QD2G
20	Reach	18.35	2	724	56,078	19	-----	-----	FLOW FROM QD2G
21	SCS Runoff	1.875	2	720	4,933	-----	-----	-----	QD2J
22	Combine	20.00	2	724	61,012	20, 21	-----	-----	FLOW FROM QD2J
23	Reach	20.24	2	724	61,021	22	-----	-----	FLOW FROM QD2J
24	SCS Runoff	2.812	2	720	7,400	-----	-----	-----	QD2M
25	Combine	22.72	2	724	68,421	23, 24	-----	-----	FLOW FROM QD2M
26	Reach	22.95	2	726	68,429	25	-----	-----	FLOW FROM QD2M
27	SCS Runoff	2.287	2	718	5,342	-----	-----	-----	QD2O
28	Combine	23.98	2	726	73,771	26, 27	-----	-----	FLOW FROM QD2O
29	Reach	25.37	2	728	74,693	28	-----	-----	FLOW TO POND FROM WEST PIPE
31	SCS Runoff	2.194	2	726	7,628	-----	-----	-----	QD2P
32	Reach	2.204	2	728	7,626	31	-----	-----	FLOW FROM QD2P
33	SCS Runoff	0.131	2	716	306	-----	-----	-----	QD2H
34	Combine	2.221	2	728	7,932	32, 33	-----	-----	FLOW FROM QD2H
35	Reach	2.202	2	730	7,930	34	-----	-----	FLOW FROM QD2H
2024-06-21 PROPOSED.gpw					Return Period: 100 Year			Monday, 06 / 24 / 2024	

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
36	SCS Runoff	0.875	2	720	2,302	-----	-----	-----	QD2I
37	Combine	2.683	2	726	10,232	35, 36	-----	-----	FLOW FROM QD2I
38	Reach	2.685	2	728	10,233	37	-----	-----	FLOW FROM QD2I
40	SCS Runoff	2.854	2	724	9,009	-----	-----	-----	QD2Q
41	Reach	2.868	2	726	9,008	40	-----	-----	FLOW FROM QD2Q
42	SCS Runoff	2.687	2	720	7,071	-----	-----	-----	QD2R
43	Combine	5.291	2	722	16,079	41, 42	-----	-----	FLOW FROM QD2R
44	Reach	5.317	2	724	16,081	43	-----	-----	FLOW FROM QD2R
46	SCS Runoff	0.465	2	722	1,322	-----	-----	-----	QD2K
47	Combine	8.324	2	724	27,635	38, 44, 46	-----	-----	FLOW FROM QD2K
48	Reach	8.352	2	726	27,636	47	-----	-----	FLOW FROM QD2K
49	SCS Runoff	0.821	2	722	2,332	-----	-----	-----	QD2L
50	Combine	9.079	2	726	29,968	48, 49	-----	-----	FLOW FROM QD2L
51	Reach	9.088	2	726	29,967	50	-----	-----	FLOW FROM QD2L
53	SCS Runoff	3.058	2	726	10,631	-----	-----	-----	QD2S
54	Reach	3.072	2	728	10,629	53	-----	-----	FLOW FROM QD2S
55	SCS Runoff	6.894	2	728	25,572	-----	-----	-----	QD2T
56	Combine	9.967	2	728	36,202	54, 55	-----	-----	FLOW FROM QD2T
57	Reach	9.977	2	730	36,202	56	-----	-----	FLOW FROM QD2T
58	Combine	19.03	2	728	66,169	51, 57	-----	-----	FLOW AT MANHOLE
59	Reach	19.05	2	730	66,177	58	-----	-----	FLOW FROM MANHOLE
60	SCS Runoff	0.380	2	716	851	-----	-----	-----	QD2N
61	Combine	19.10	2	730	67,028	59, 60	-----	-----	FLOW FROM QD2N
62	Reach	19.23	2	730	67,051	61	-----	-----	FLOW TO POND FROM EAST PIPE
64	SCS Runoff	4.900	2	730	19,080	-----	-----	-----	QD2X FLOW FROM PARK AREA
65	SCS Runoff	1.407	2	722	3,962	-----	-----	-----	QD3
67	Combine	50.18	2	728	164,786	29, 62, 64, 65,	-----	-----	TOTAL FLOW TO POND
69	Reservoir(i)	27.60	2	738	126,853	67	6476.30	29,295	Stage 3
2024-06-21 PROPOSED.gpw					Return Period: 100 Year			Monday, 06 / 24 / 2024	

Hyd. No. 1

QD2A

Hydrograph type	= SCS Runoff	Peak discharge	= 4.386 cfs
Storm frequency	= 100 yrs	Time to peak	= 722 min
Time interval	= 2 min	Hyd. volume	= 12,631 cuft
Drainage area	= 1.500 ac	Curve number	= 92
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 14.00 min
Total precip.	= 3.23 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484

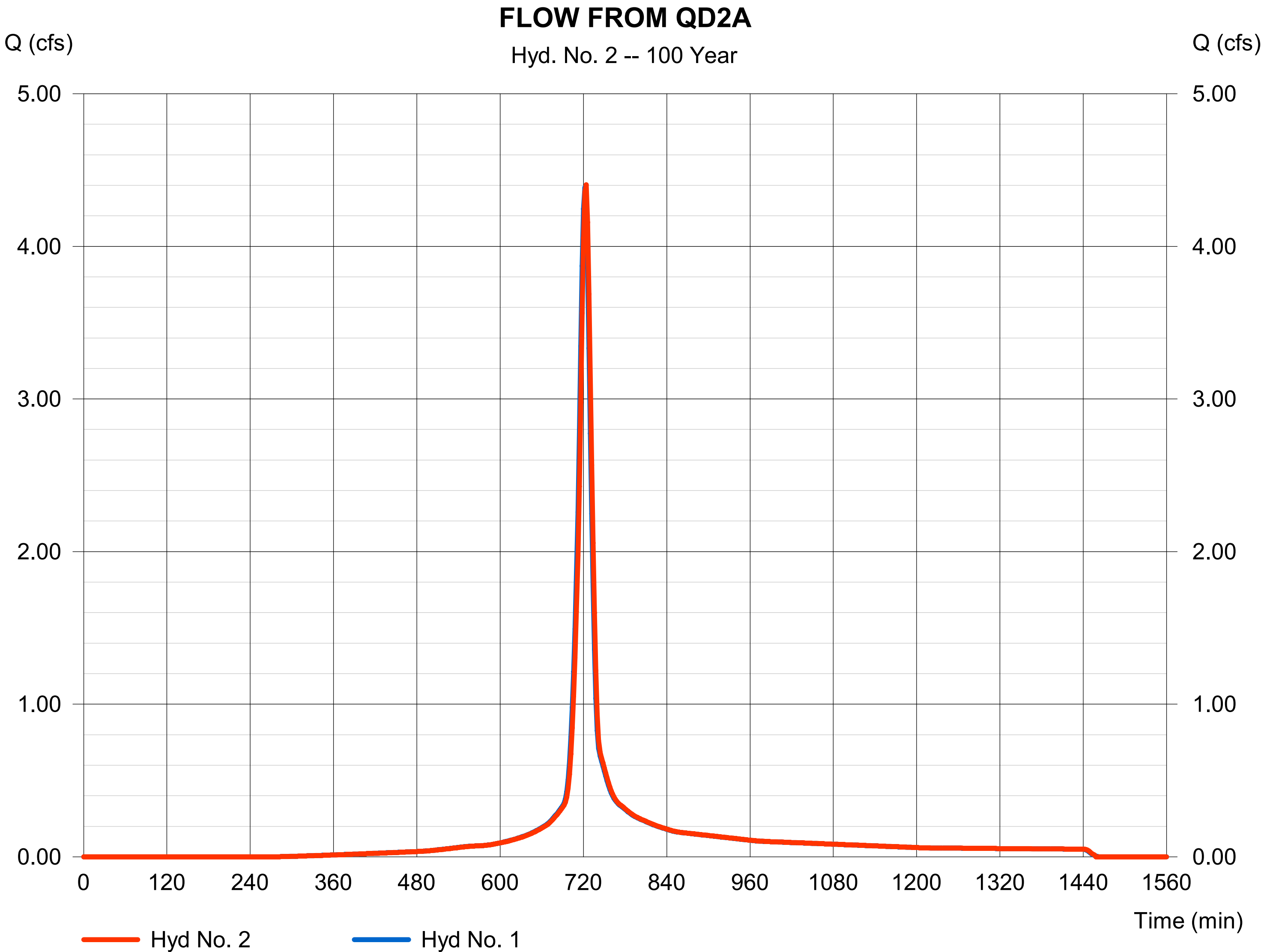


Hyd. No. 2

FLOW FROM QD2A

Hydrograph type	= Reach	Peak discharge	= 4.403 cfs
Storm frequency	= 100 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 12,629 cuft
Inflow hyd. No.	= 1 - QD2A	Section type	= Circular
Reach length	= 47.7 ft	Channel slope	= 0.9 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 3.438	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.5918

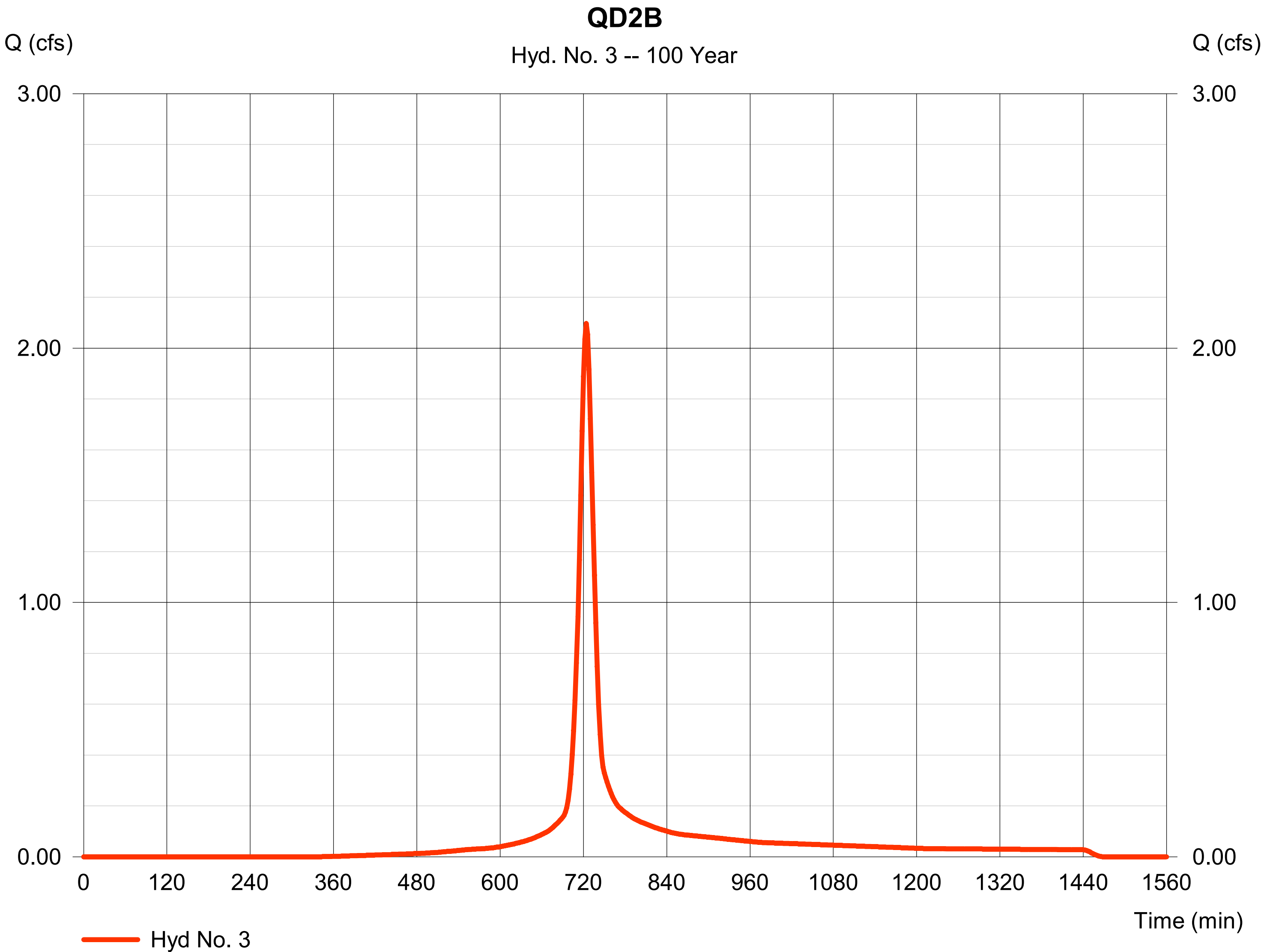
Modified Att-Kin routing method used.



Hyd. No. 3

QD2B

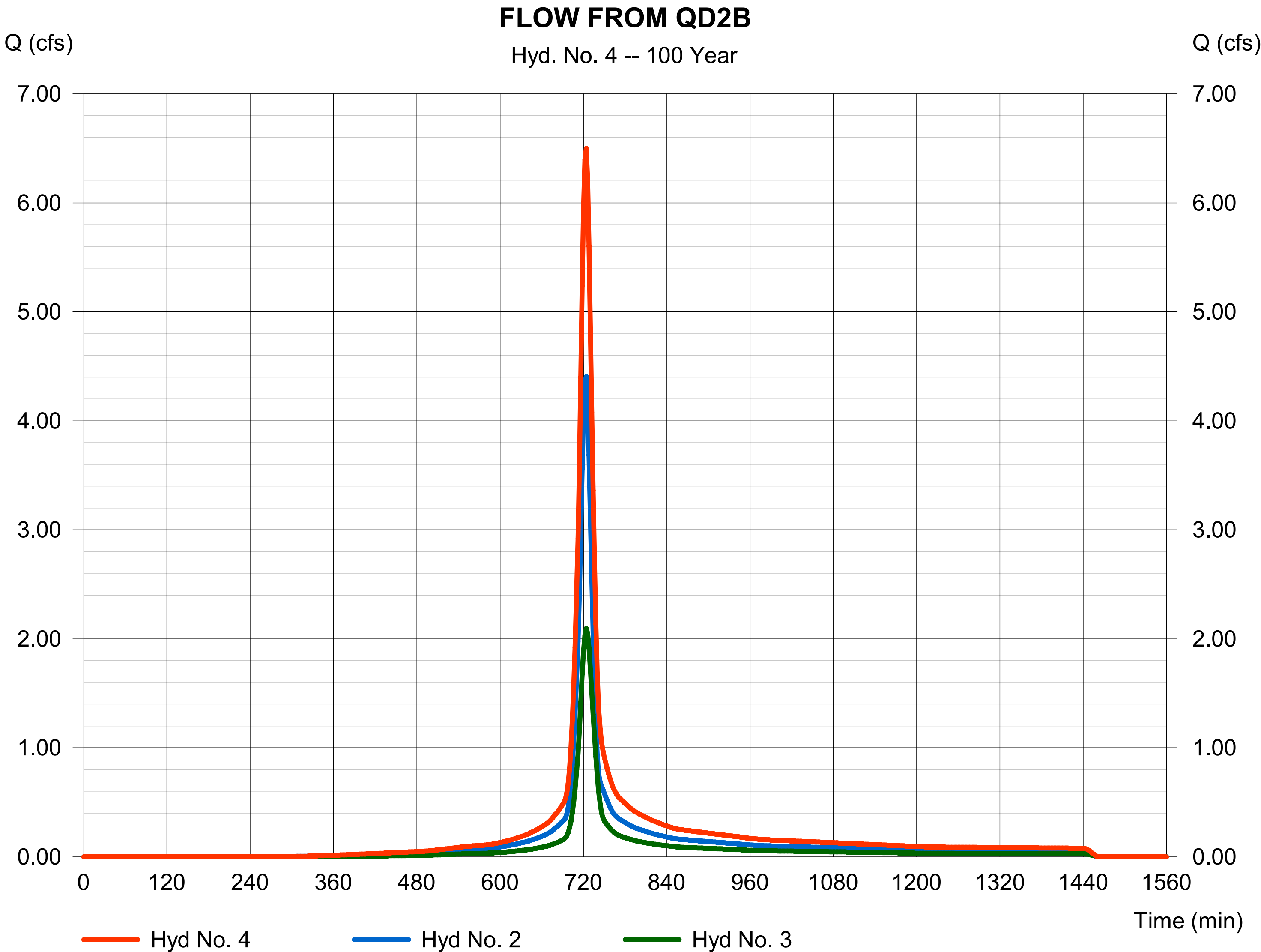
Hydrograph type	=	SCS Runoff	Peak discharge	=	2.096 cfs
Storm frequency	=	100 yrs	Time to peak	=	724 min
Time interval	=	2 min	Hyd. volume	=	6,618 cuft
Drainage area	=	0.830 ac	Curve number	=	90
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	16.90 min
Total precip.	=	3.23 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hyd. No. 4

FLOW FROM QD2B

Hydrograph type	= Combine	Peak discharge	= 6.500 cfs
Storm frequency	= 100 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 19,246 cuft
Inflow hyds.	= 2, 3	Contrib. drain. area	= 0.830 ac

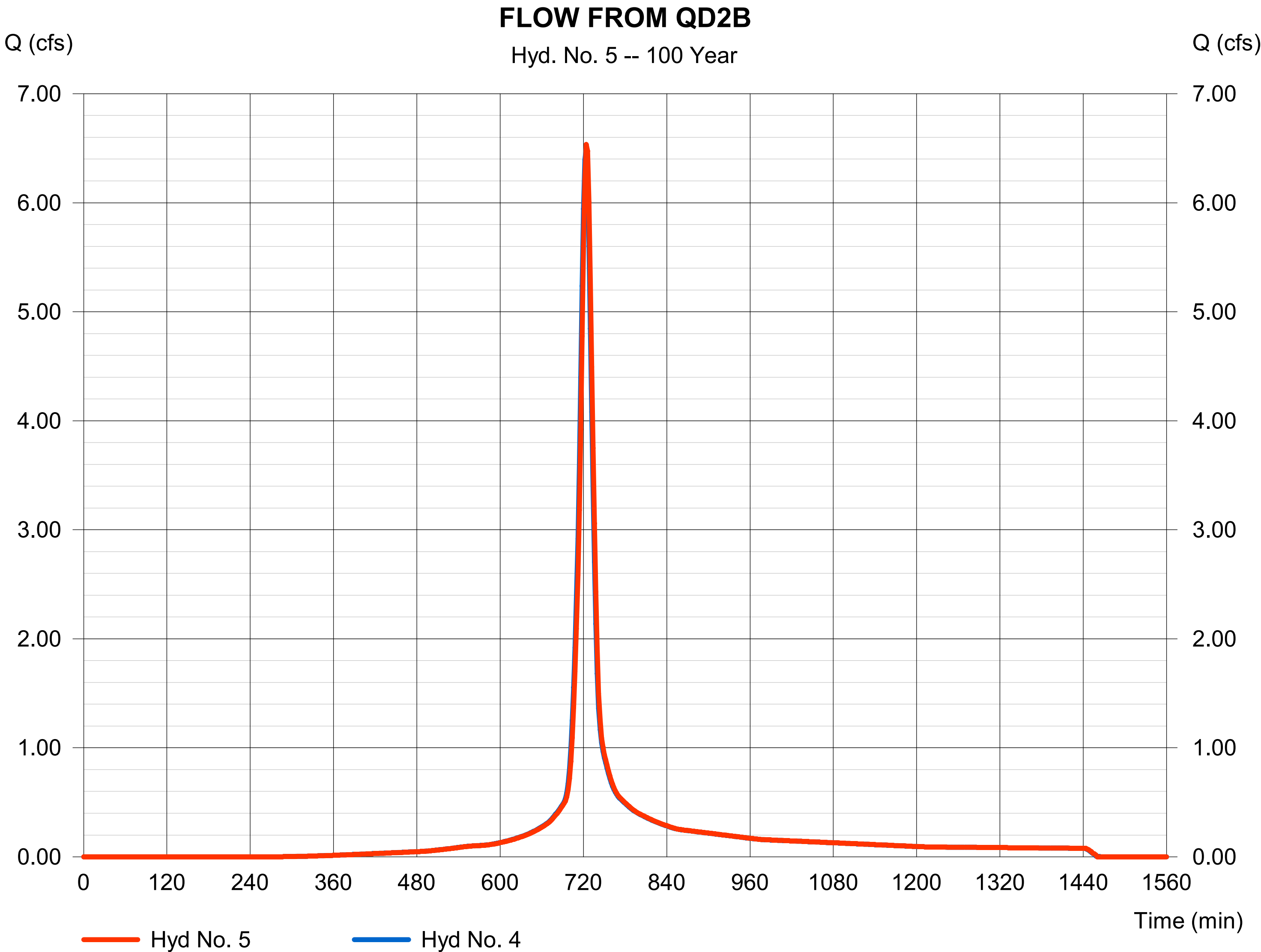


Hyd. No. 5

FLOW FROM QD2B

Hydrograph type	= Reach	Peak discharge	= 6.533 cfs
Storm frequency	= 100 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 19,245 cuft
Inflow hyd. No.	= 4 - FLOW FROM QD2B	Section type	= Circular
Reach length	= 32.2 ft	Channel slope	= 0.9 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 3.438	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.7911

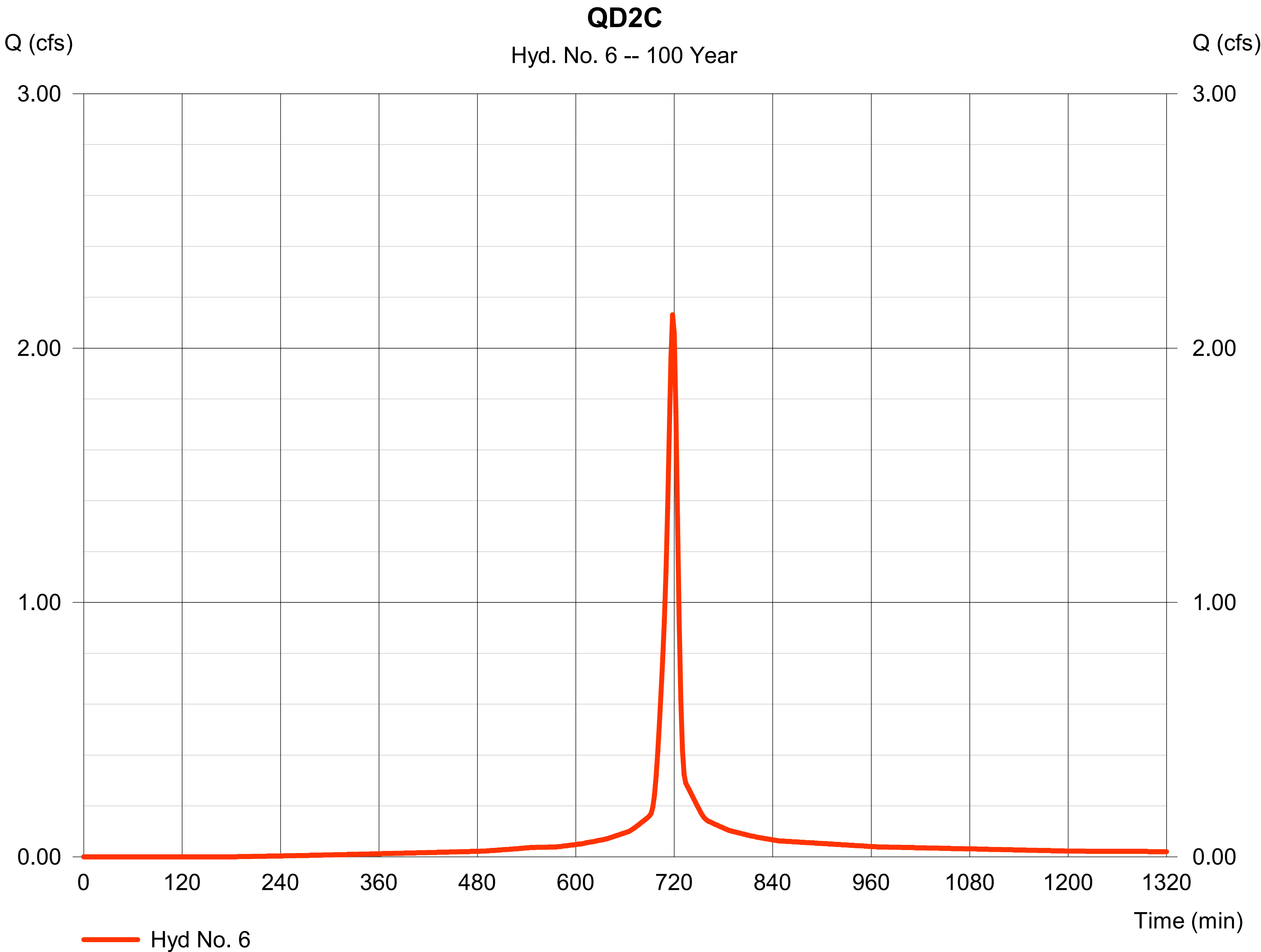
Modified Att-Kin routing method used.



Hyd. No. 6

QD2C

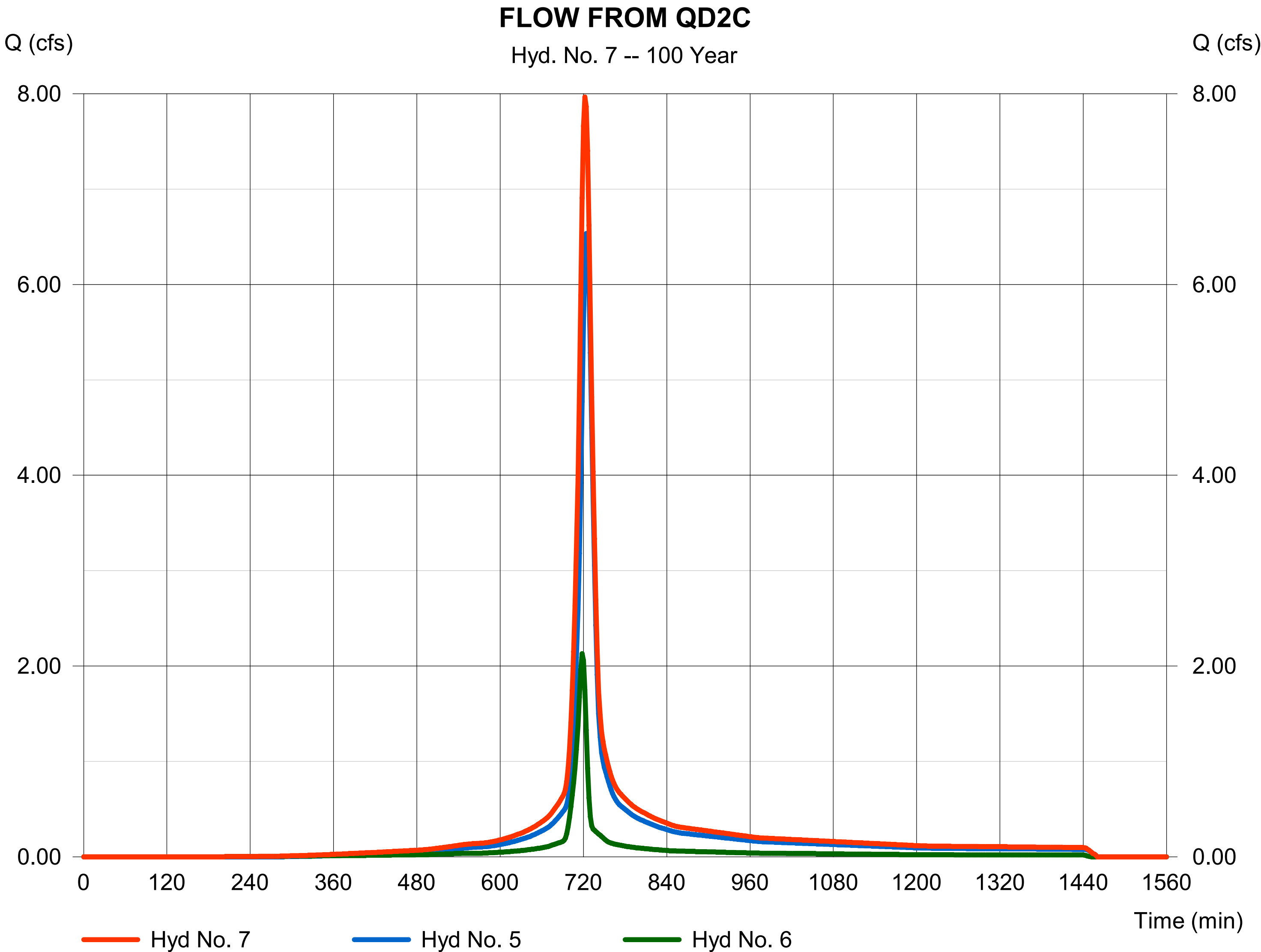
Hydrograph type	=	SCS Runoff	Peak discharge	=	2.131 cfs
Storm frequency	=	100 yrs	Time to peak	=	718 min
Time interval	=	2 min	Hyd. volume	=	5,242 cuft
Drainage area	=	0.540 ac	Curve number	=	95
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	9.50 min
Total precip.	=	3.23 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hyd. No. 7

FLOW FROM QD2C

Hydrograph type	= Combine	Peak discharge	= 7.965 cfs
Storm frequency	= 100 yrs	Time to peak	= 722 min
Time interval	= 2 min	Hyd. volume	= 24,487 cuft
Inflow hyds.	= 5, 6	Contrib. drain. area	= 0.540 ac

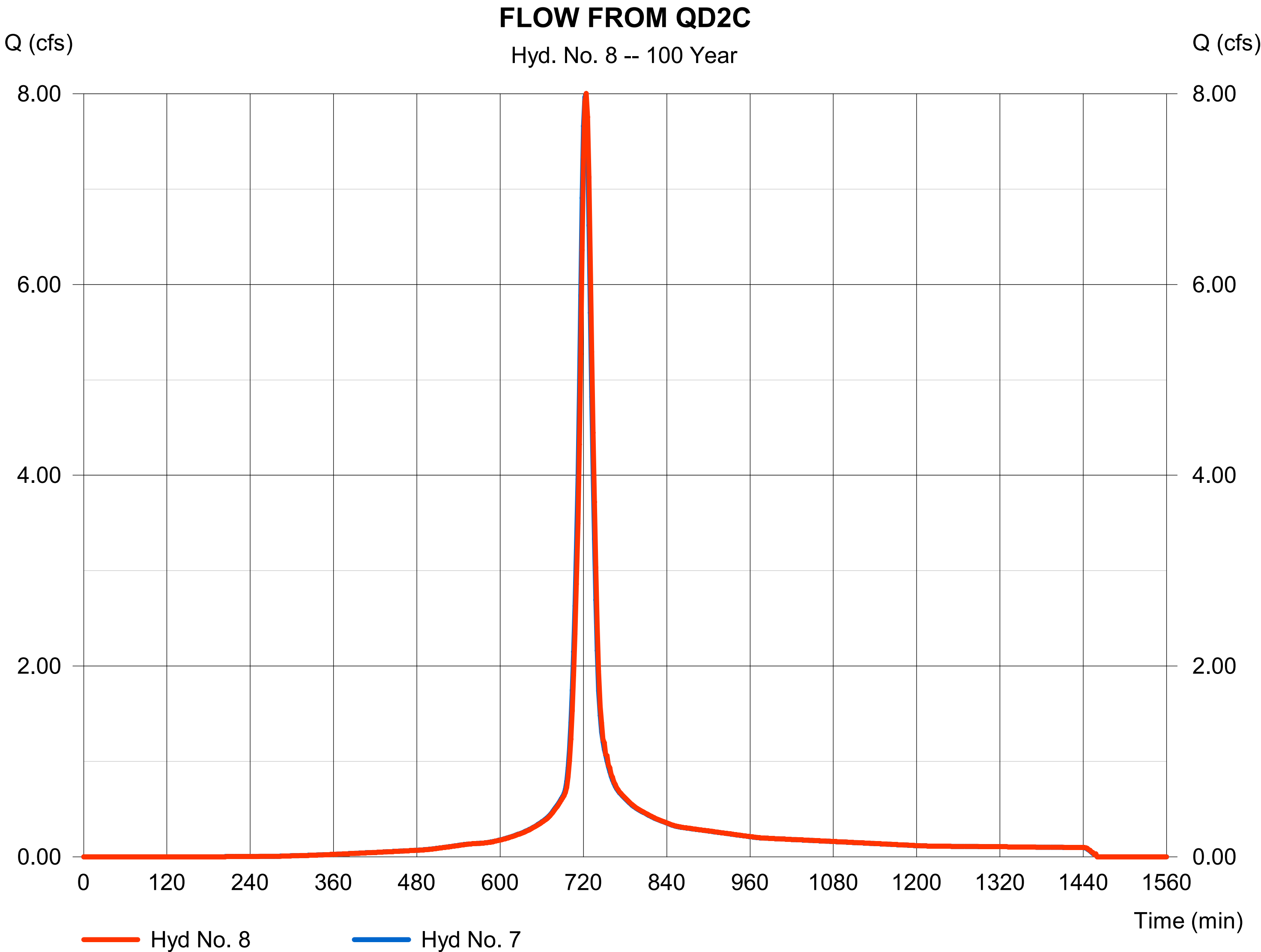


Hyd. No. 8

FLOW FROM QD2C

Hydrograph type	= Reach	Peak discharge	= 8.002 cfs
Storm frequency	= 100 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 24,487 cuft
Inflow hyd. No.	= 7 - FLOW FROM QD2C	Section type	= Circular
Reach length	= 41.0 ft	Channel slope	= 0.9 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 3.438	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.7839

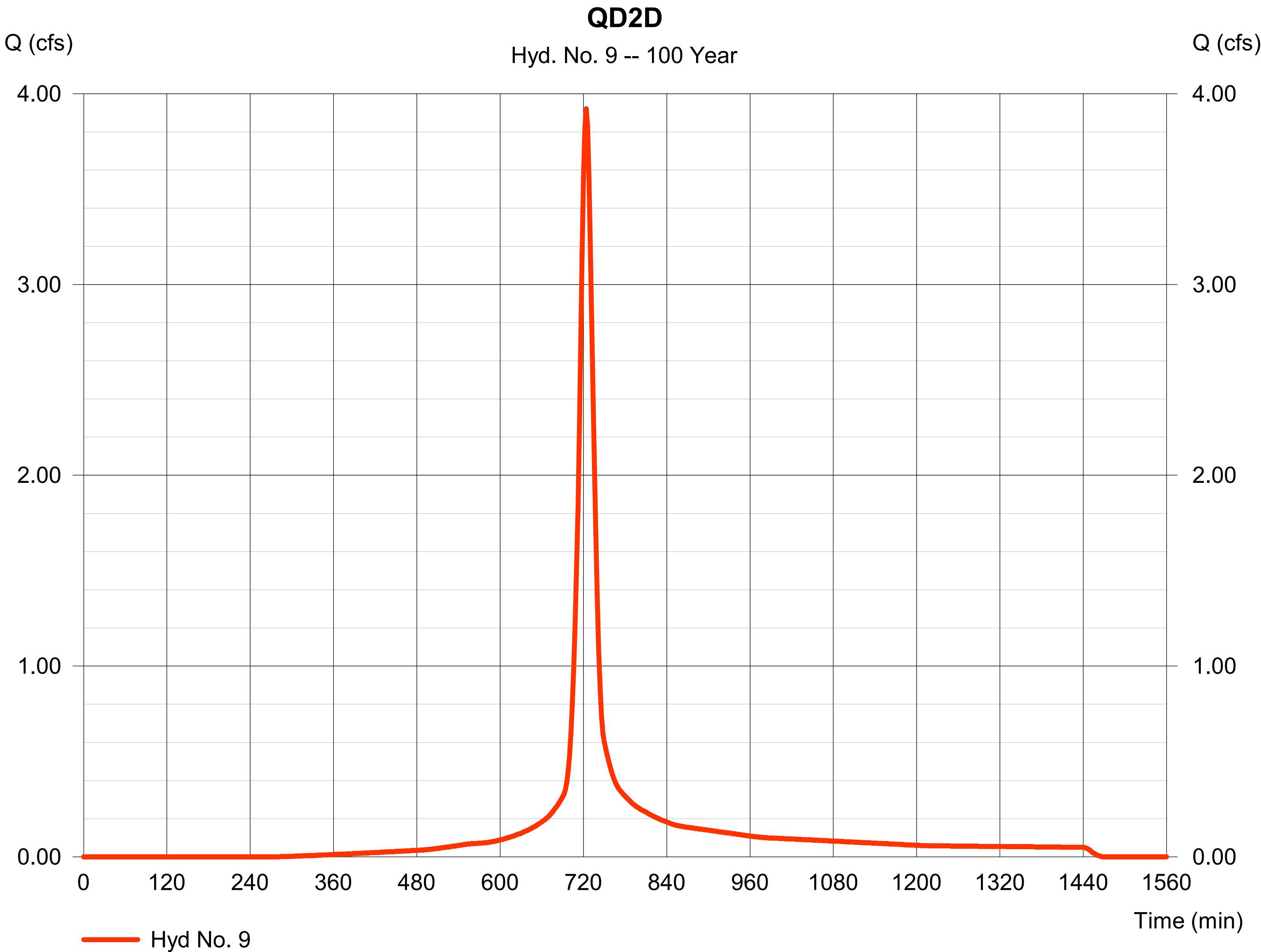
Modified Att-Kin routing method used.



Hyd. No. 9

QD2D

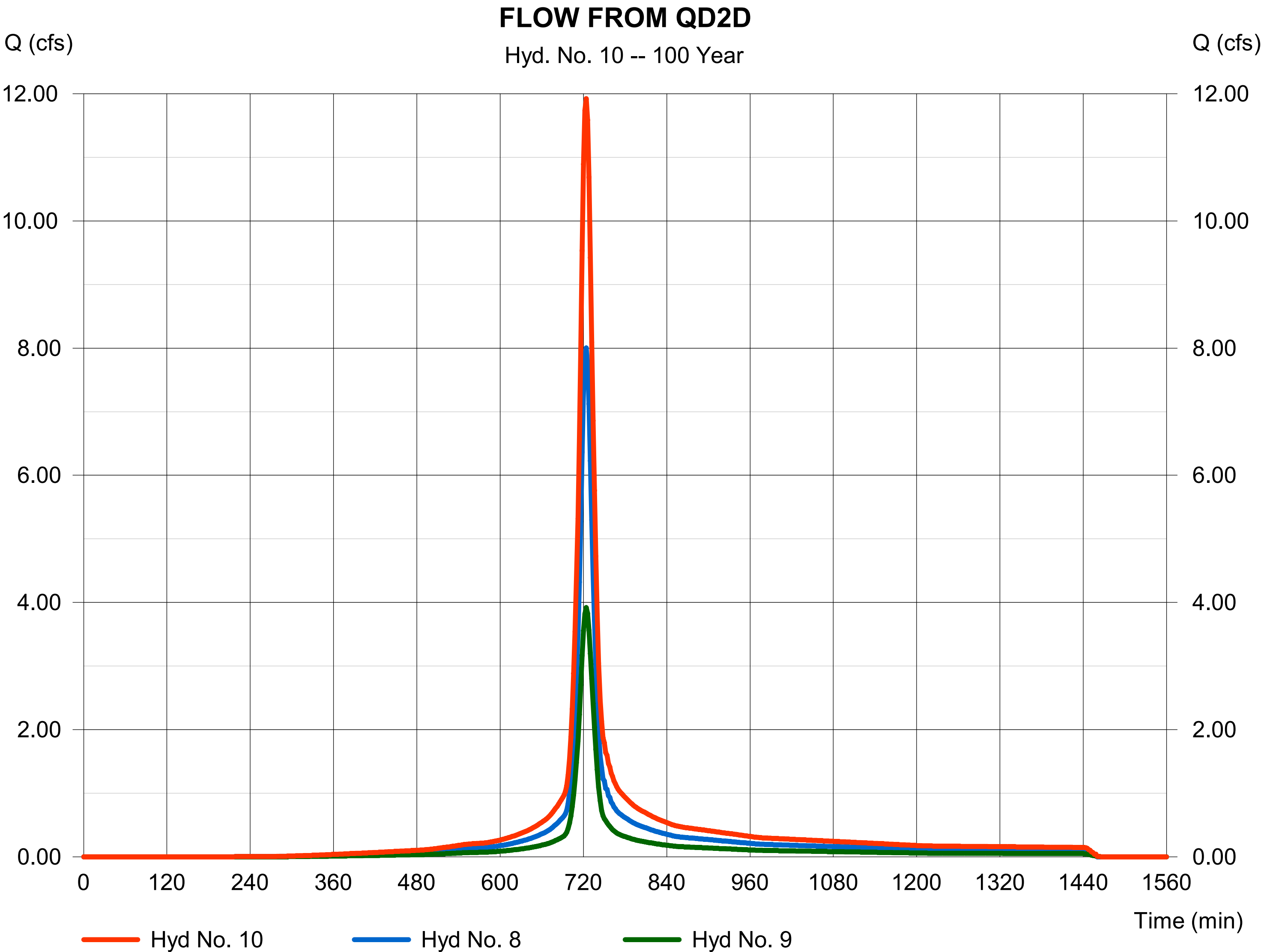
Hydrograph type	=	SCS Runoff	Peak discharge	=	3.921 cfs
Storm frequency	=	100 yrs	Time to peak	=	724 min
Time interval	=	2 min	Hyd. volume	=	12,523 cuft
Drainage area	=	1.450 ac	Curve number	=	92
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	18.60 min
Total precip.	=	3.23 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hyd. No. 10

FLOW FROM QD2D

Hydrograph type	= Combine	Peak discharge	= 11.92 cfs
Storm frequency	= 100 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 37,009 cuft
Inflow hyds.	= 8, 9	Contrib. drain. area	= 1.450 ac

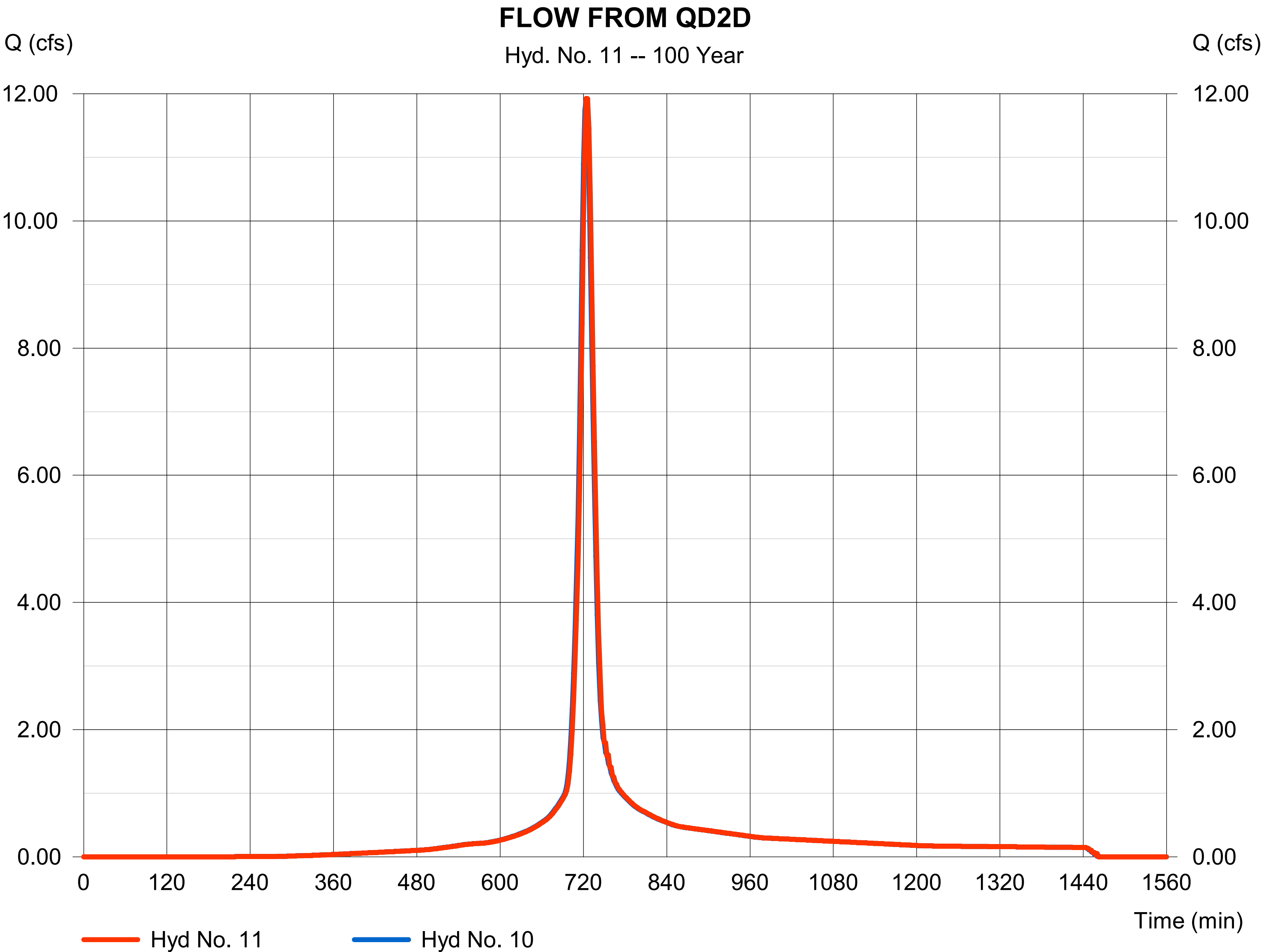


Hyd. No. 11

FLOW FROM QD2D

Hydrograph type	= Reach	Peak discharge	= 11.93 cfs
Storm frequency	= 100 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 37,008 cuft
Inflow hyd. No.	= 10 - FLOW FROM QD2D	Section type	= Circular
Reach length	= 225.0 ft	Channel slope	= 0.9 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 3.438	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.3844

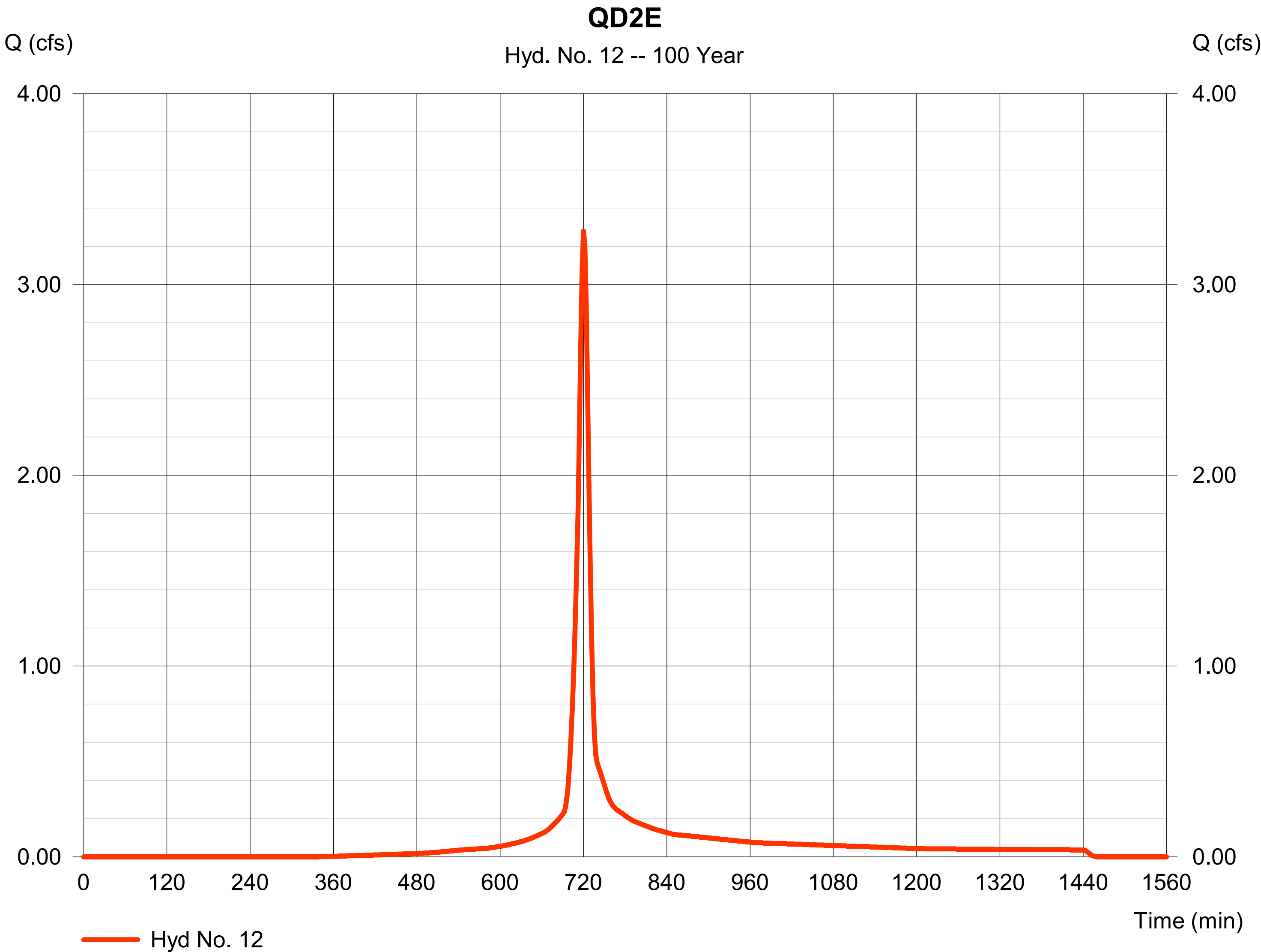
Modified Att-Kin routing method used.



Hyd. No. 12

QD2E

Hydrograph type	=	SCS Runoff	Peak discharge	=	3.280 cfs
Storm frequency	=	100 yrs	Time to peak	=	720 min
Time interval	=	2 min	Hyd. volume	=	8,633 cuft
Drainage area	=	1.050 ac	Curve number	=	90
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	11.90 min
Total precip.	=	3.23 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484

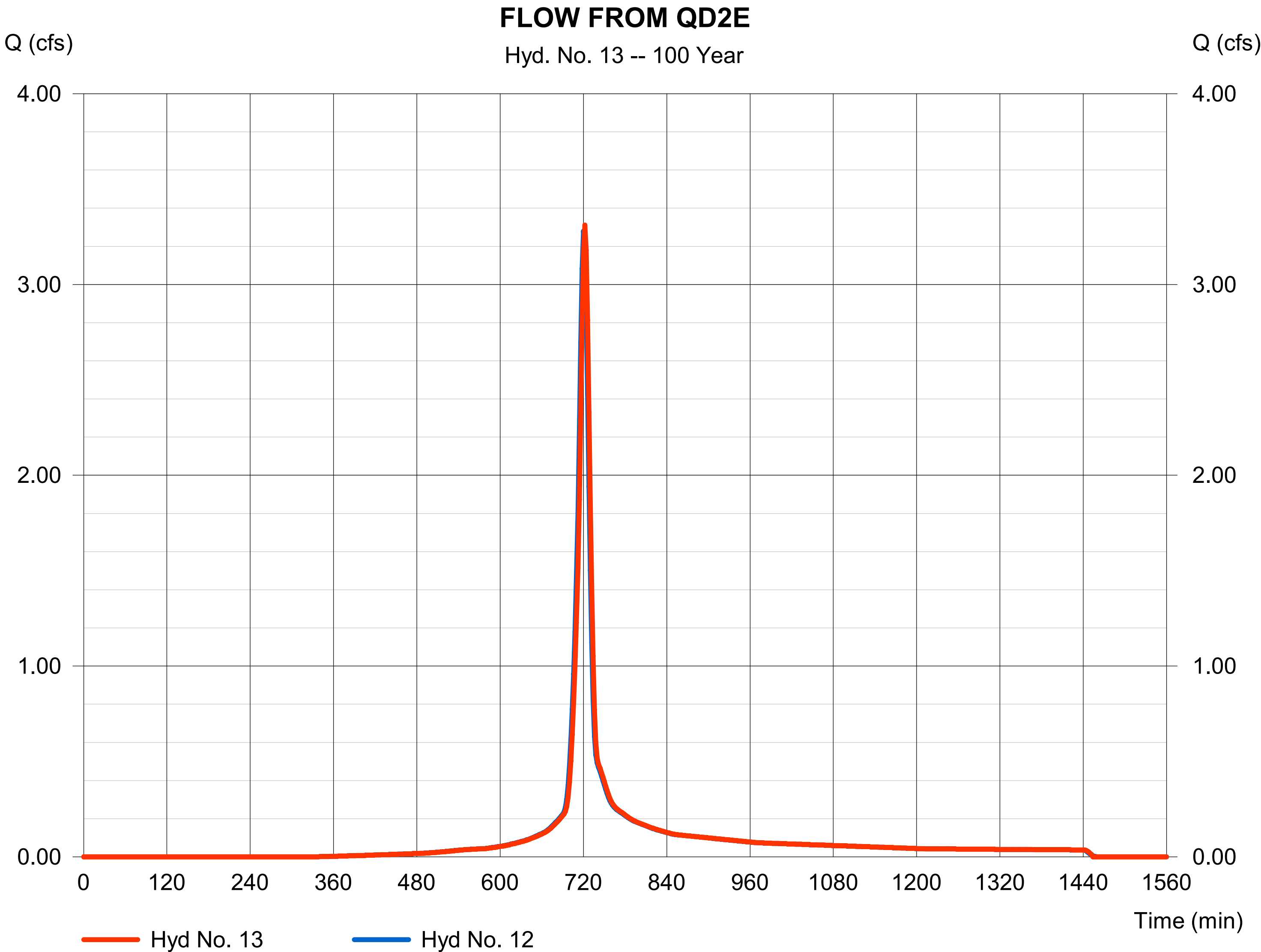


Hyd. No. 13

FLOW FROM QD2E

Hydrograph type	= Reach	Peak discharge	= 3.311 cfs
Storm frequency	= 100 yrs	Time to peak	= 722 min
Time interval	= 2 min	Hyd. volume	= 8,631 cuft
Inflow hyd. No.	= 12 - QD2E	Section type	= Circular
Reach length	= 84.5 ft	Channel slope	= 0.5 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.534	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.2446

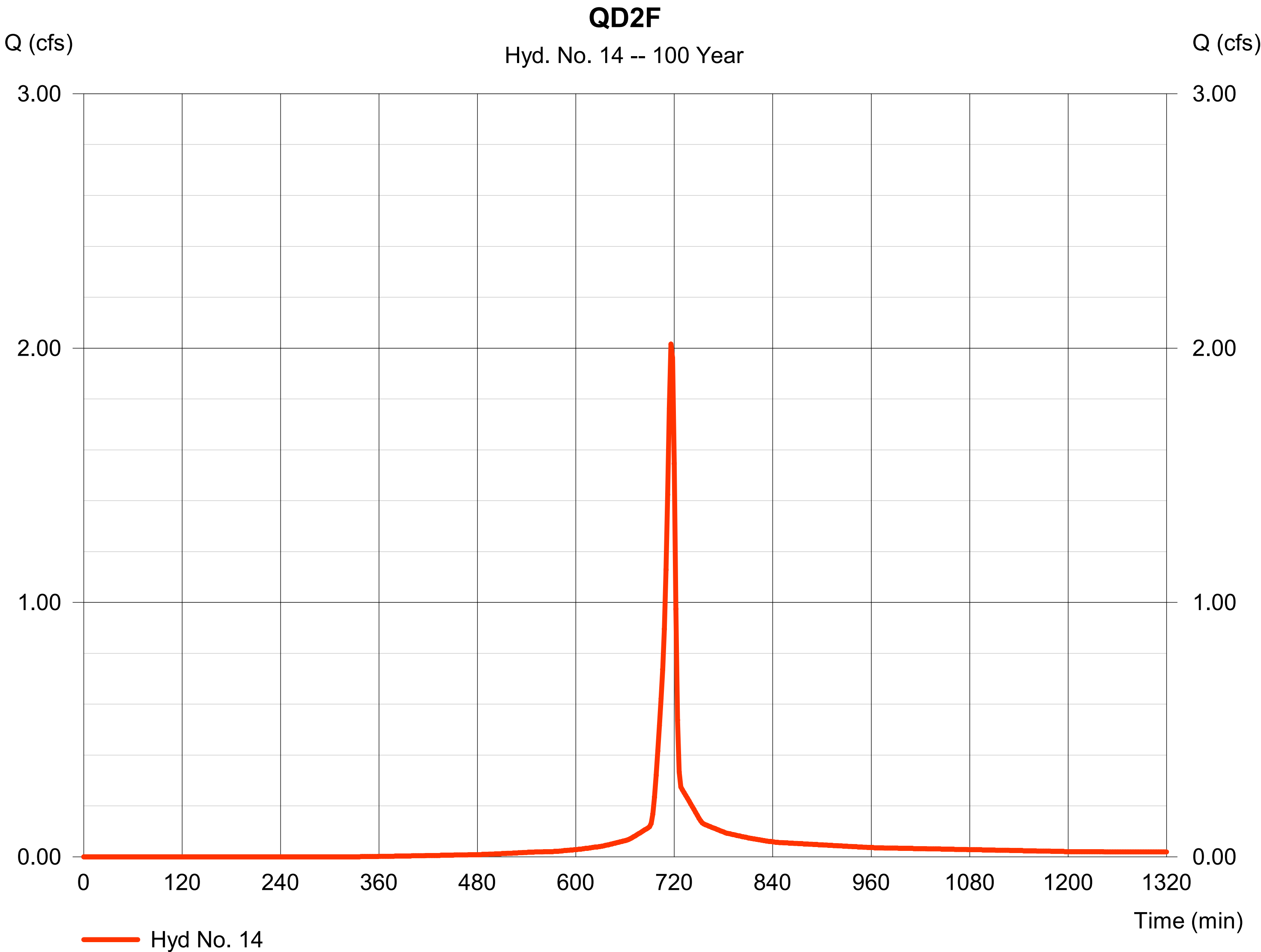
Modified Att-Kin routing method used.



Hyd. No. 14

QD2F

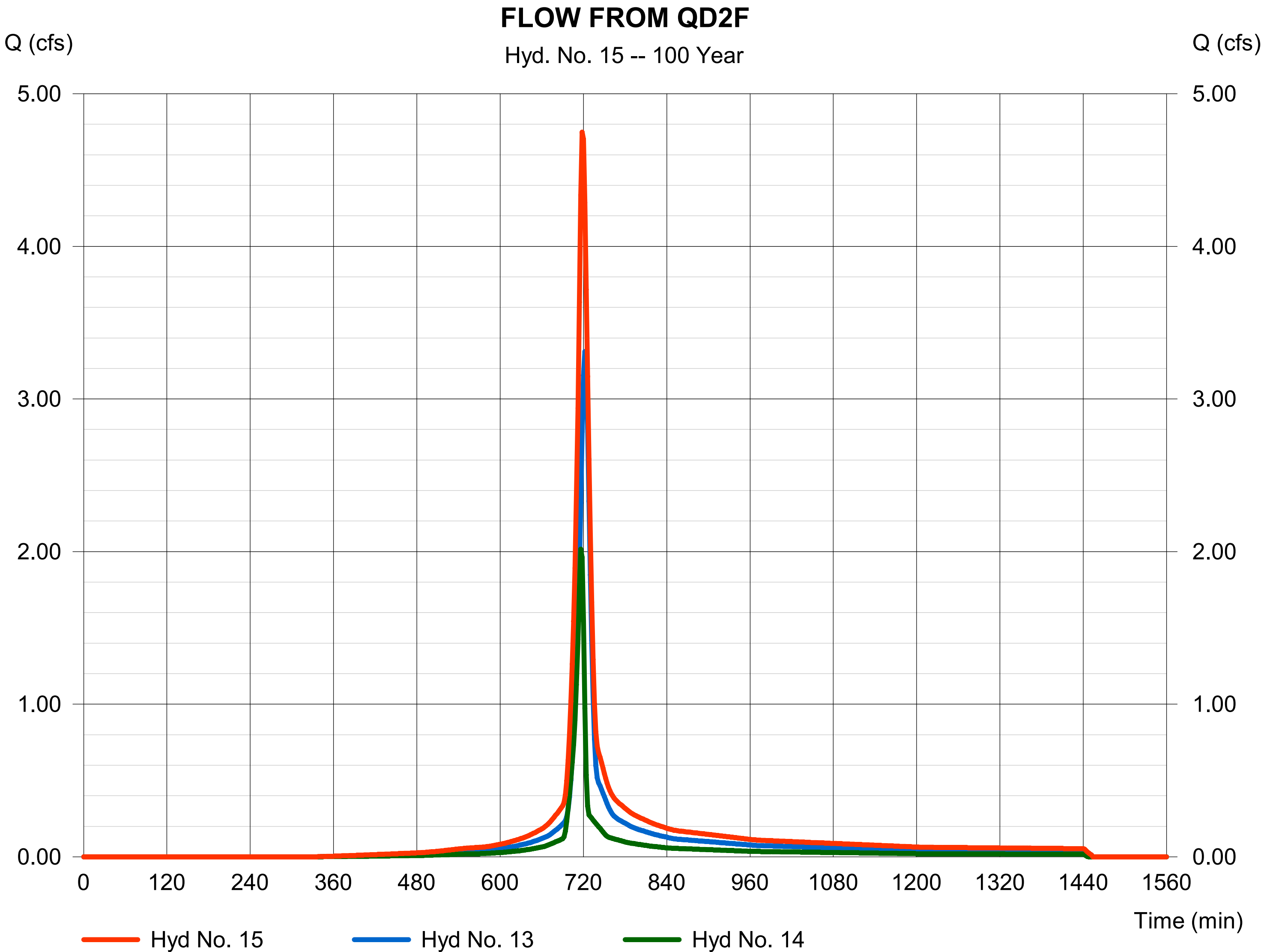
Hydrograph type	=	SCS Runoff	Peak discharge	=	2.015 cfs
Storm frequency	=	100 yrs	Time to peak	=	716 min
Time interval	=	2 min	Hyd. volume	=	4,186 cuft
Drainage area	=	0.560 ac	Curve number	=	90
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	6.00 min
Total precip.	=	3.23 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hyd. No. 15

FLOW FROM QD2F

Hydrograph type	= Combine	Peak discharge	= 4.748 cfs
Storm frequency	= 100 yrs	Time to peak	= 718 min
Time interval	= 2 min	Hyd. volume	= 12,817 cuft
Inflow hyds.	= 13, 14	Contrib. drain. area	= 0.560 ac

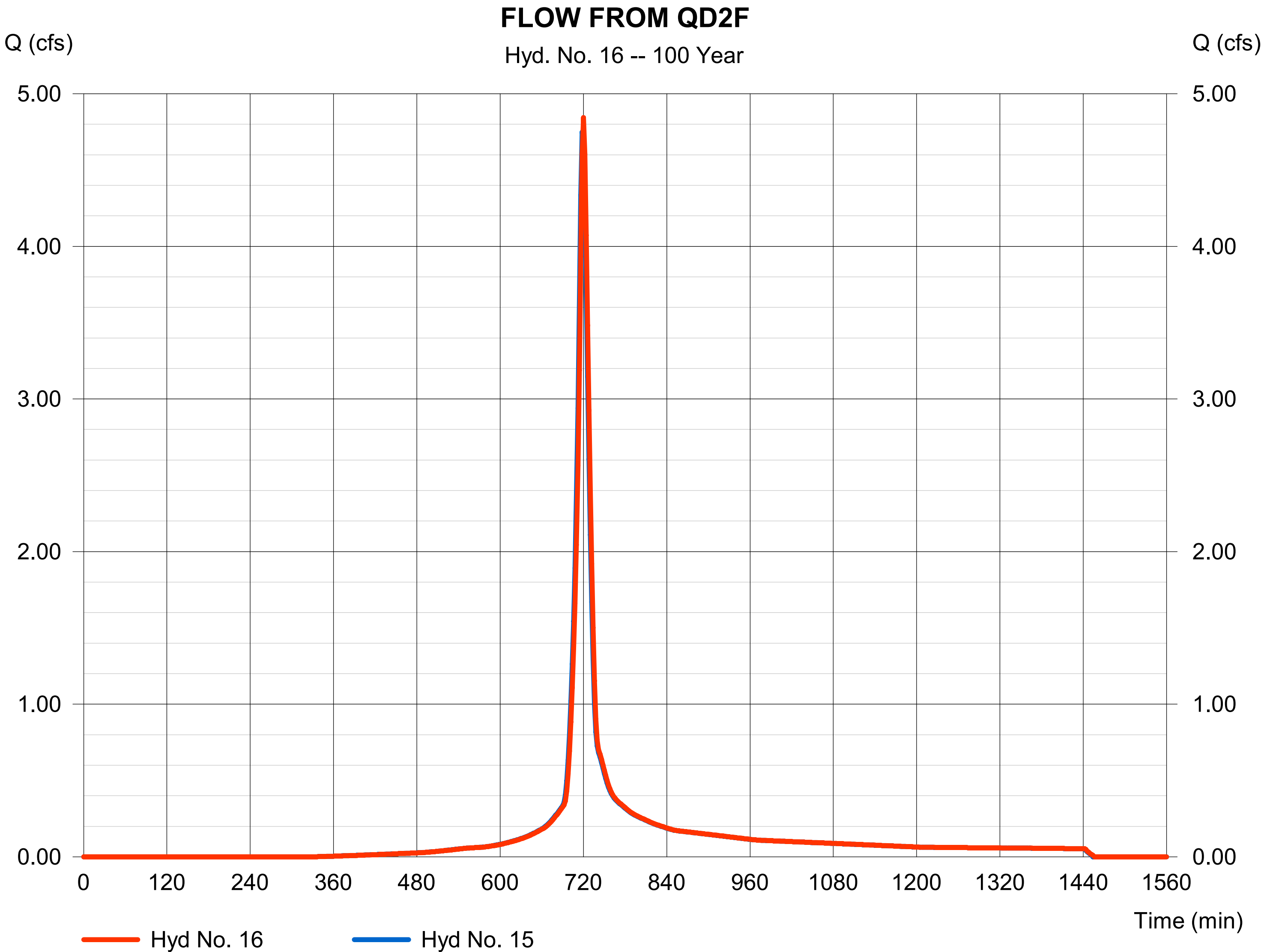


Hyd. No. 16

FLOW FROM QD2F

Hydrograph type	= Reach	Peak discharge	= 4.844 cfs
Storm frequency	= 100 yrs	Time to peak	= 720 min
Time interval	= 2 min	Hyd. volume	= 12,818 cuft
Inflow hyd. No.	= 15 - FLOW FROM QD2F	Section type	= Circular
Reach length	= 40.2 ft	Channel slope	= 0.5 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.534	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.6676

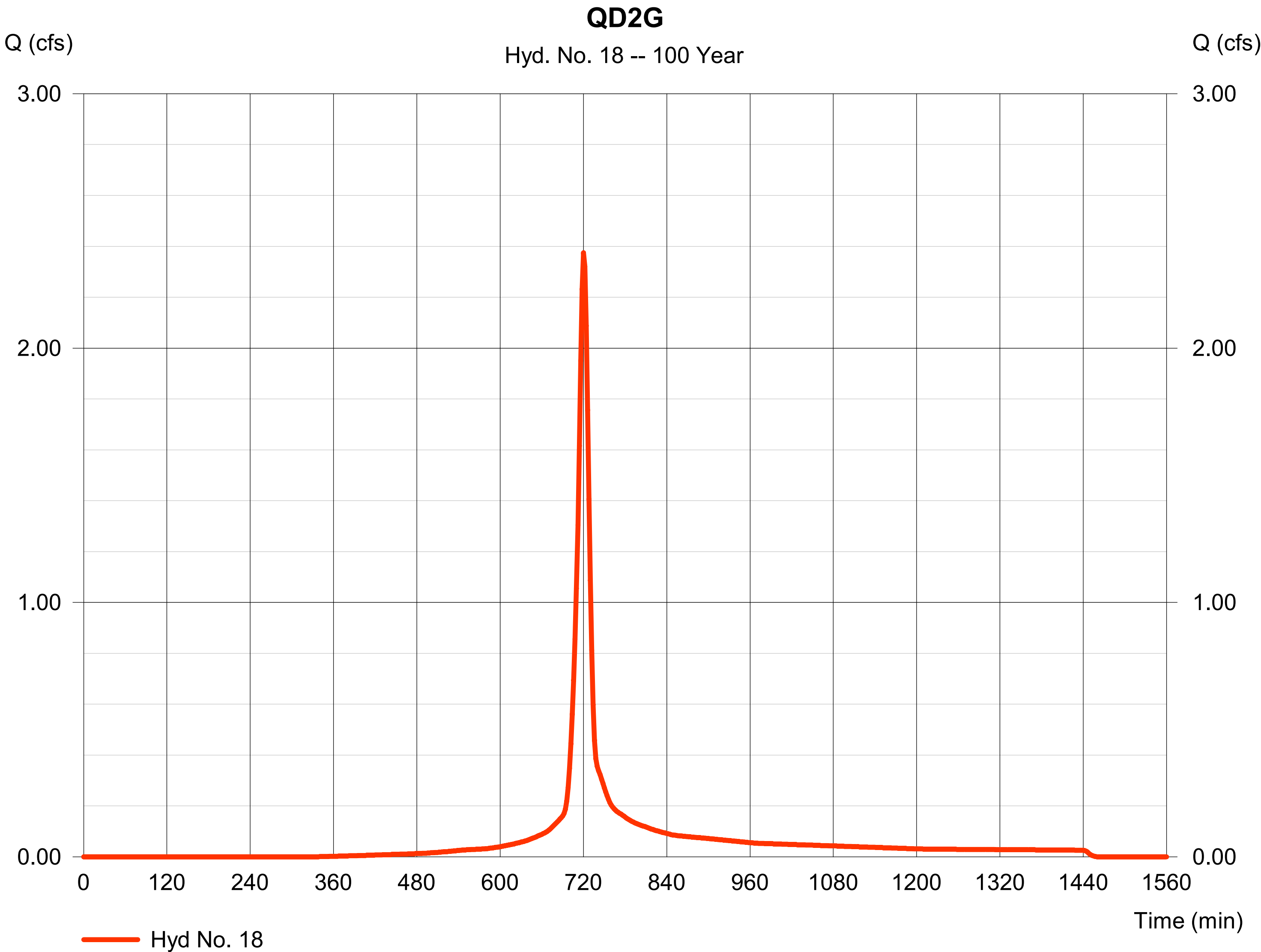
Modified Att-Kin routing method used.



Hyd. No. 18

QD2G

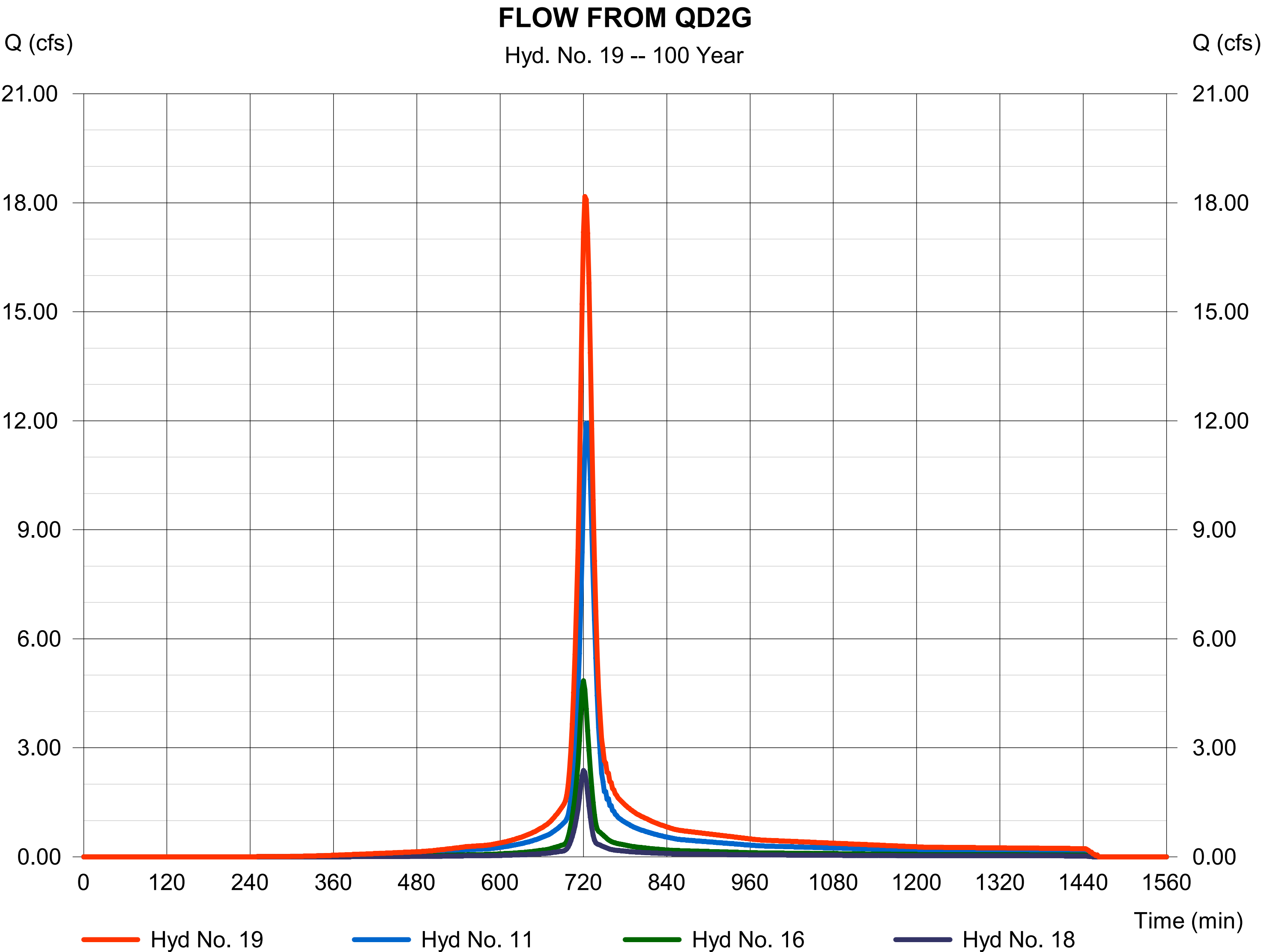
Hydrograph type	=	SCS Runoff	Peak discharge	=	2.374 cfs
Storm frequency	=	100 yrs	Time to peak	=	720 min
Time interval	=	2 min	Hyd. volume	=	6,249 cuft
Drainage area	=	0.760 ac	Curve number	=	90
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	12.80 min
Total precip.	=	3.23 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hyd. No. 19

FLOW FROM QD2G

Hydrograph type	= Combine	Peak discharge	= 18.17 cfs
Storm frequency	= 100 yrs	Time to peak	= 722 min
Time interval	= 2 min	Hyd. volume	= 56,075 cuft
Inflow hyds.	= 11, 16, 18	Contrib. drain. area	= 0.760 ac

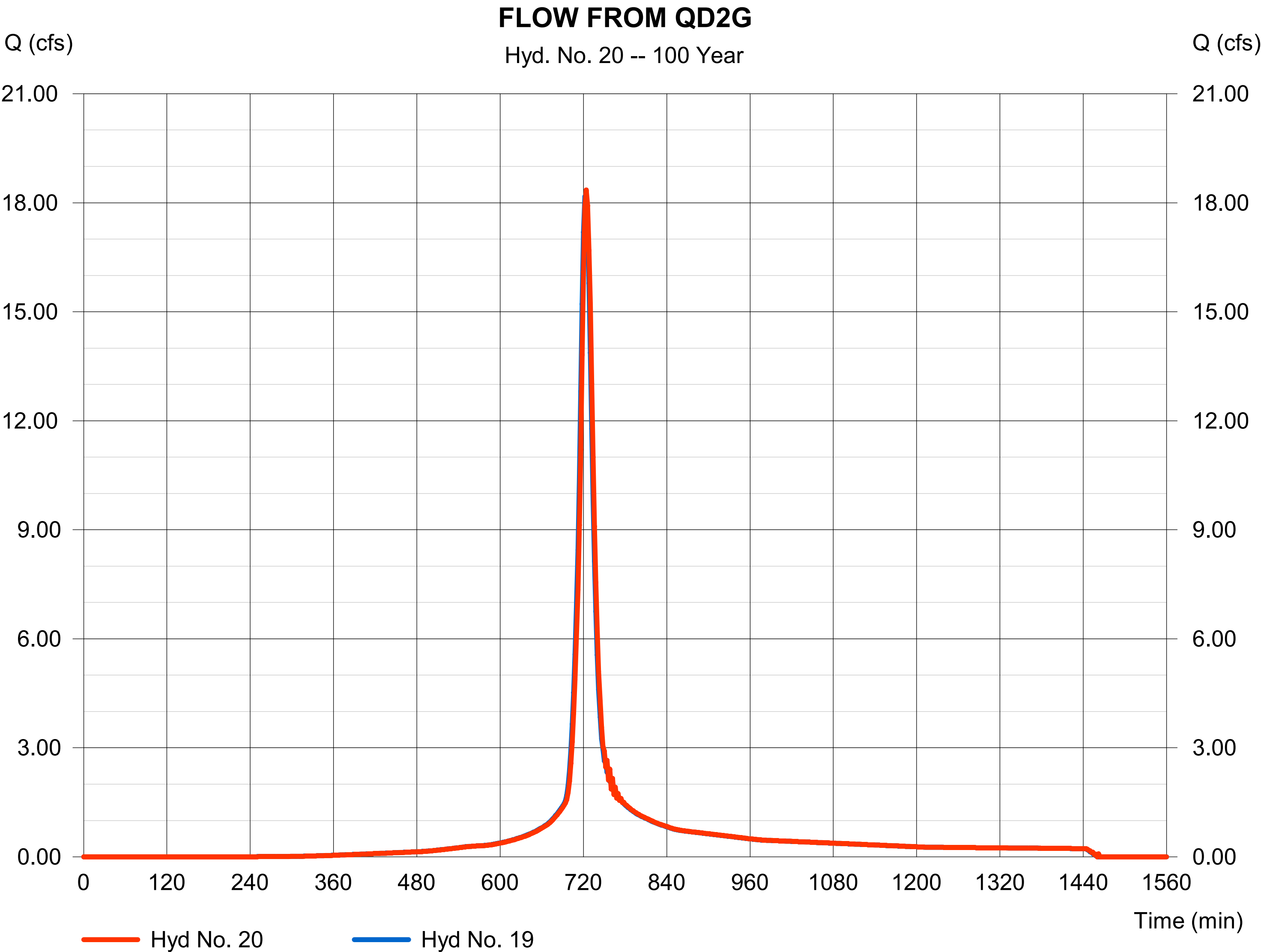


Hyd. No. 20

FLOW FROM QD2G

Hydrograph type	= Reach	Peak discharge	= 18.35 cfs
Storm frequency	= 100 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 56,078 cuft
Inflow hyd. No.	= 19 - FLOW FROM QD2G	Section type	= Circular
Reach length	= 176.6 ft	Channel slope	= 1.0 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 3.584	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.6274

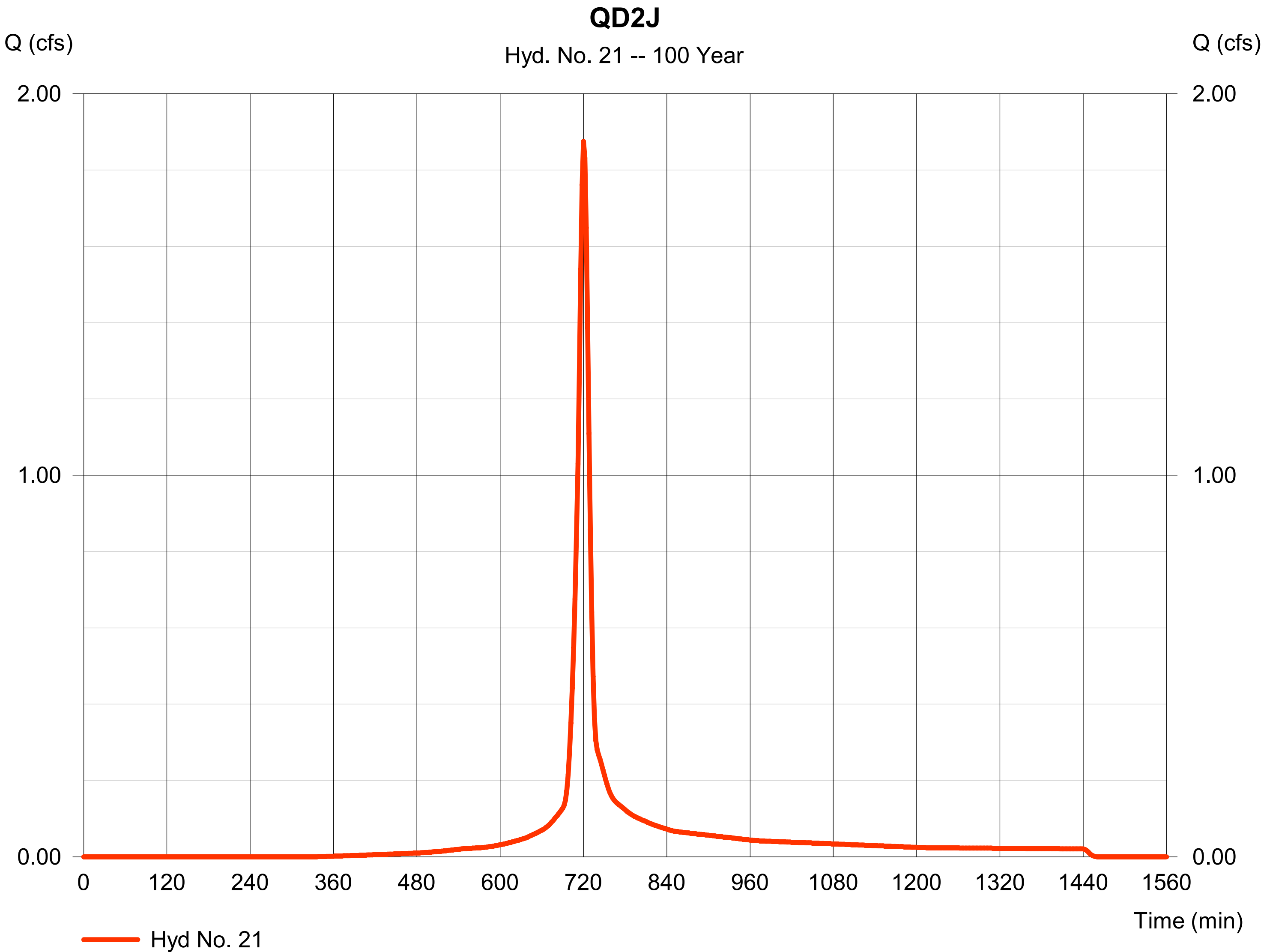
Modified Att-Kin routing method used.



Hyd. No. 21

QD2J

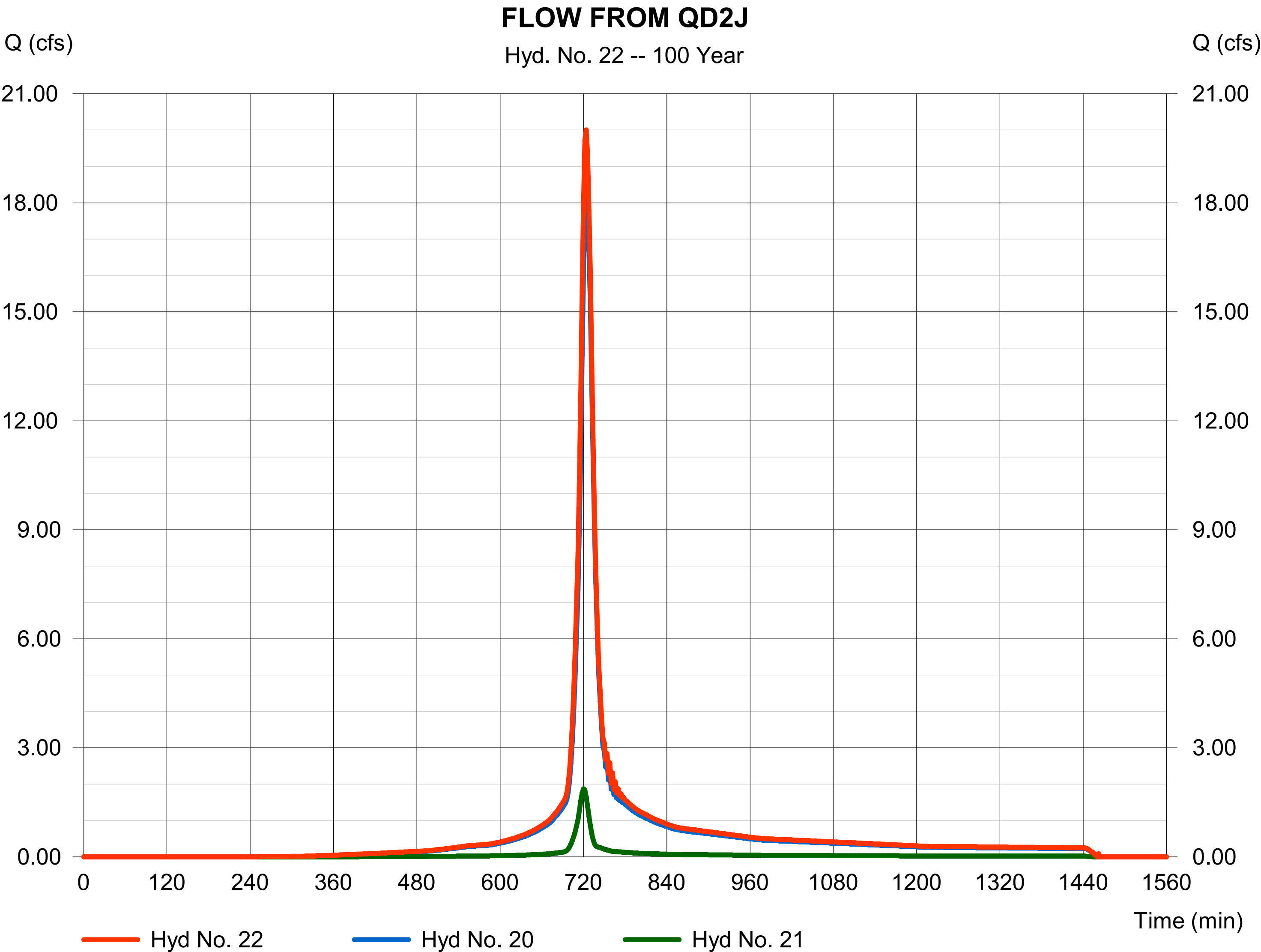
Hydrograph type	=	SCS Runoff	Peak discharge	=	1.875 cfs
Storm frequency	=	100 yrs	Time to peak	=	720 min
Time interval	=	2 min	Hyd. volume	=	4,933 cuft
Drainage area	=	0.600 ac	Curve number	=	90
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	12.20 min
Total precip.	=	3.23 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hyd. No. 22

FLOW FROM QD2J

Hydrograph type	= Combine	Peak discharge	= 20.00 cfs
Storm frequency	= 100 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 61,012 cuft
Inflow hyds.	= 20, 21	Contrib. drain. area	= 0.600 ac

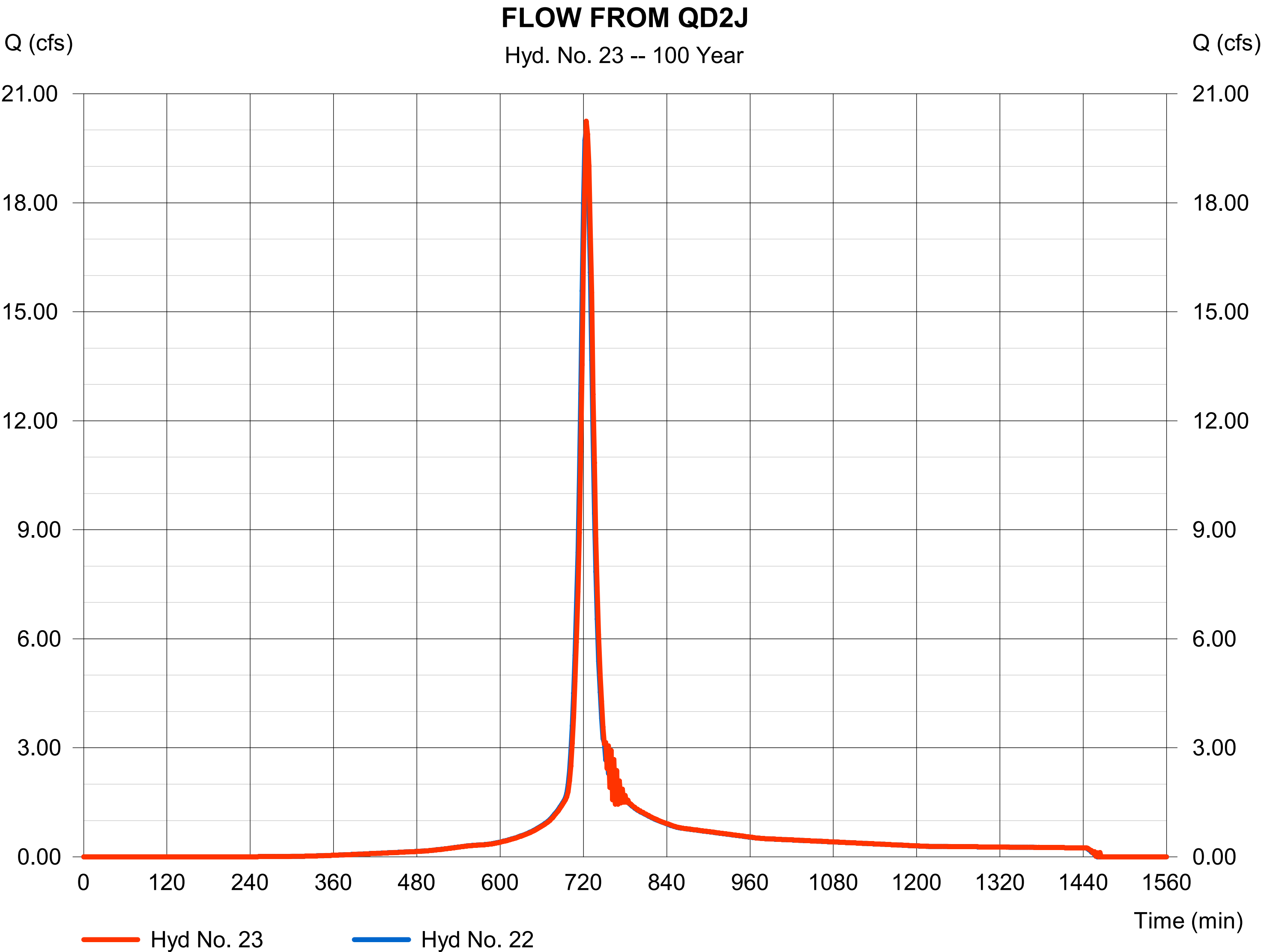


Hyd. No. 23

FLOW FROM QD2J

Hydrograph type	= Reach	Peak discharge	= 20.24 cfs
Storm frequency	= 100 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 61,021 cuft
Inflow hyd. No.	= 22 - FLOW FROM QD2J	Section type	= Circular
Reach length	= 158.4 ft	Channel slope	= 0.5 %
Manning's n	= 0.024	Bottom width	= 2.0 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.659	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.5018

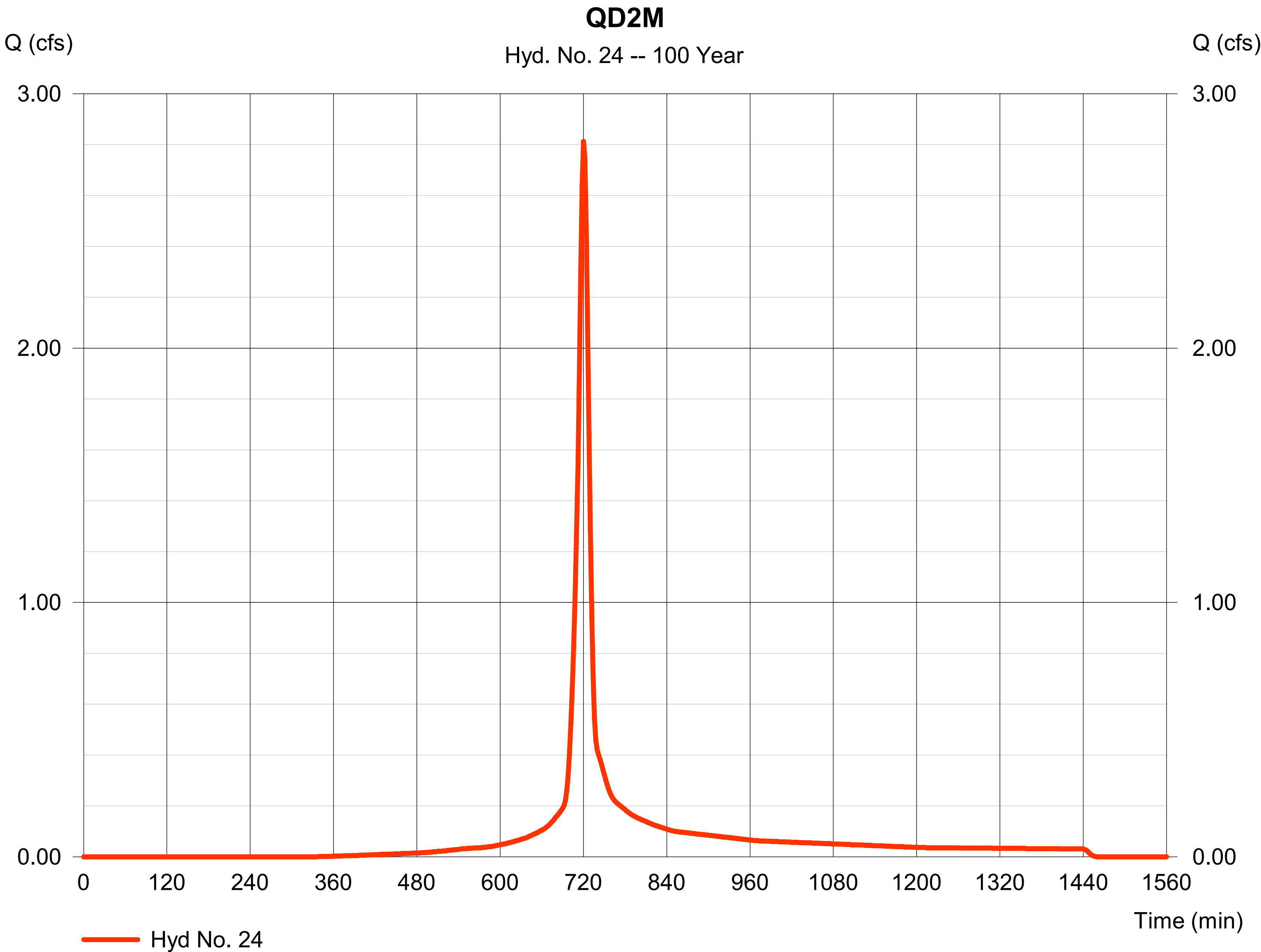
Modified Att-Kin routing method used.



Hyd. No. 24

QD2M

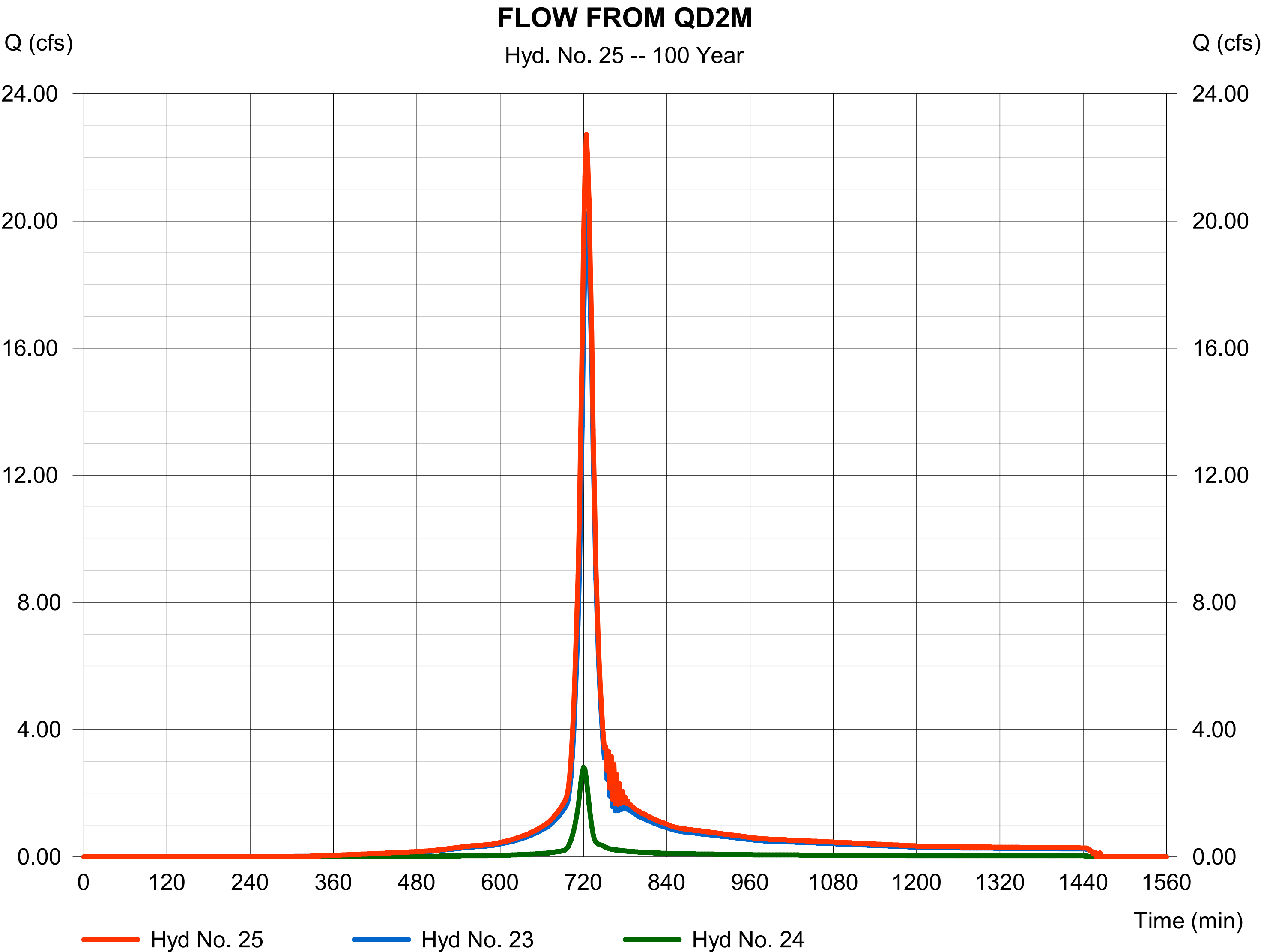
Hydrograph type	=	SCS Runoff	Peak discharge	=	2.812 cfs
Storm frequency	=	100 yrs	Time to peak	=	720 min
Time interval	=	2 min	Hyd. volume	=	7,400 cuft
Drainage area	=	0.900 ac	Curve number	=	90
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	11.90 min
Total precip.	=	3.23 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hyd. No. 25

FLOW FROM QD2M

Hydrograph type	= Combine	Peak discharge	= 22.72 cfs
Storm frequency	= 100 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 68,421 cuft
Inflow hyds.	= 23, 24	Contrib. drain. area	= 0.900 ac

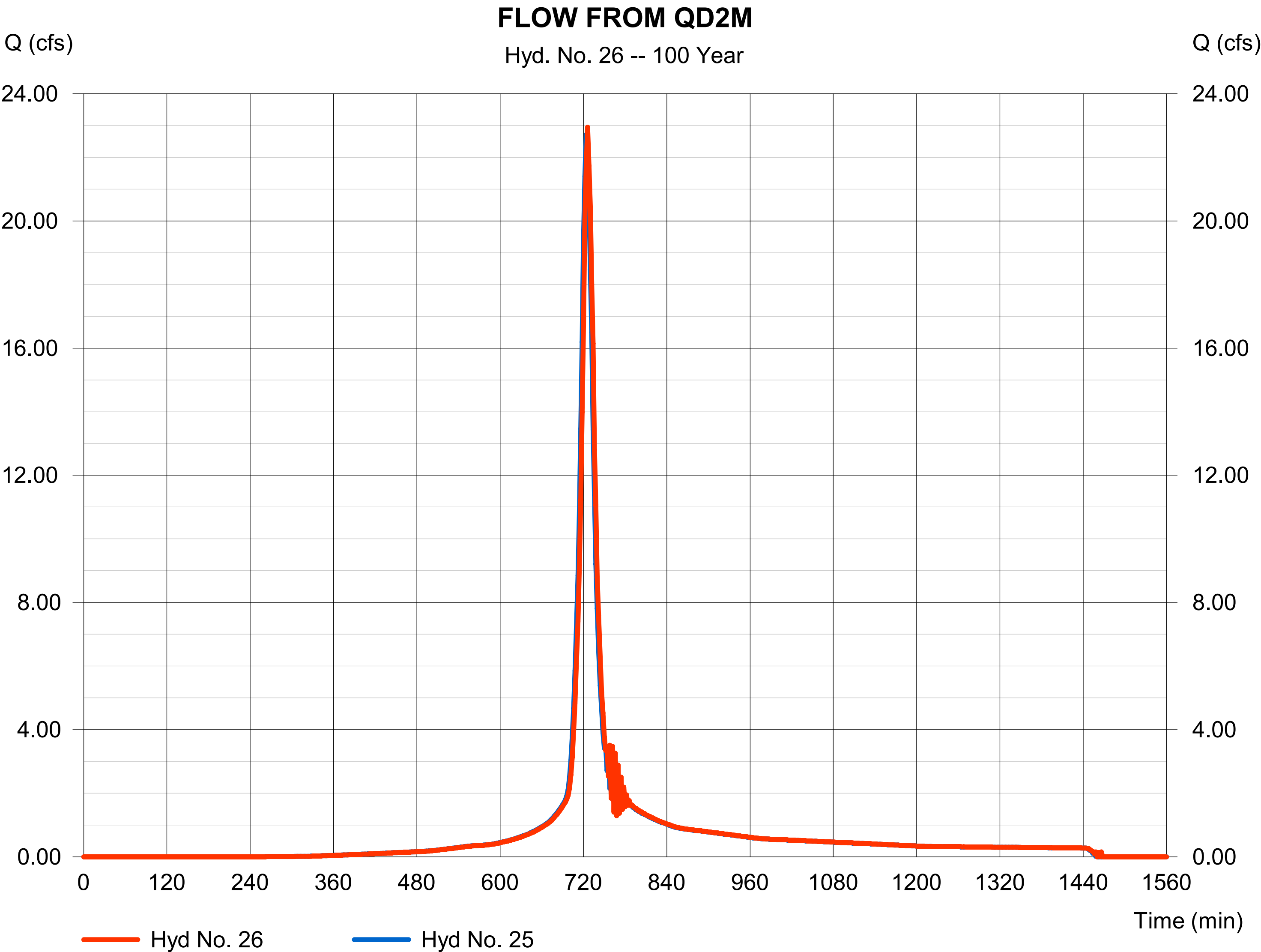


Hyd. No. 26

FLOW FROM QD2M

Hydrograph type	= Reach	Peak discharge	= 22.95 cfs
Storm frequency	= 100 yrs	Time to peak	= 726 min
Time interval	= 2 min	Hyd. volume	= 68,429 cuft
Inflow hyd. No.	= 25 - FLOW FROM QD2M	Section type	= Circular
Reach length	= 336.3 ft	Channel slope	= 0.3 %
Manning's n	= 0.024	Bottom width	= 2.0 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.060	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.2344

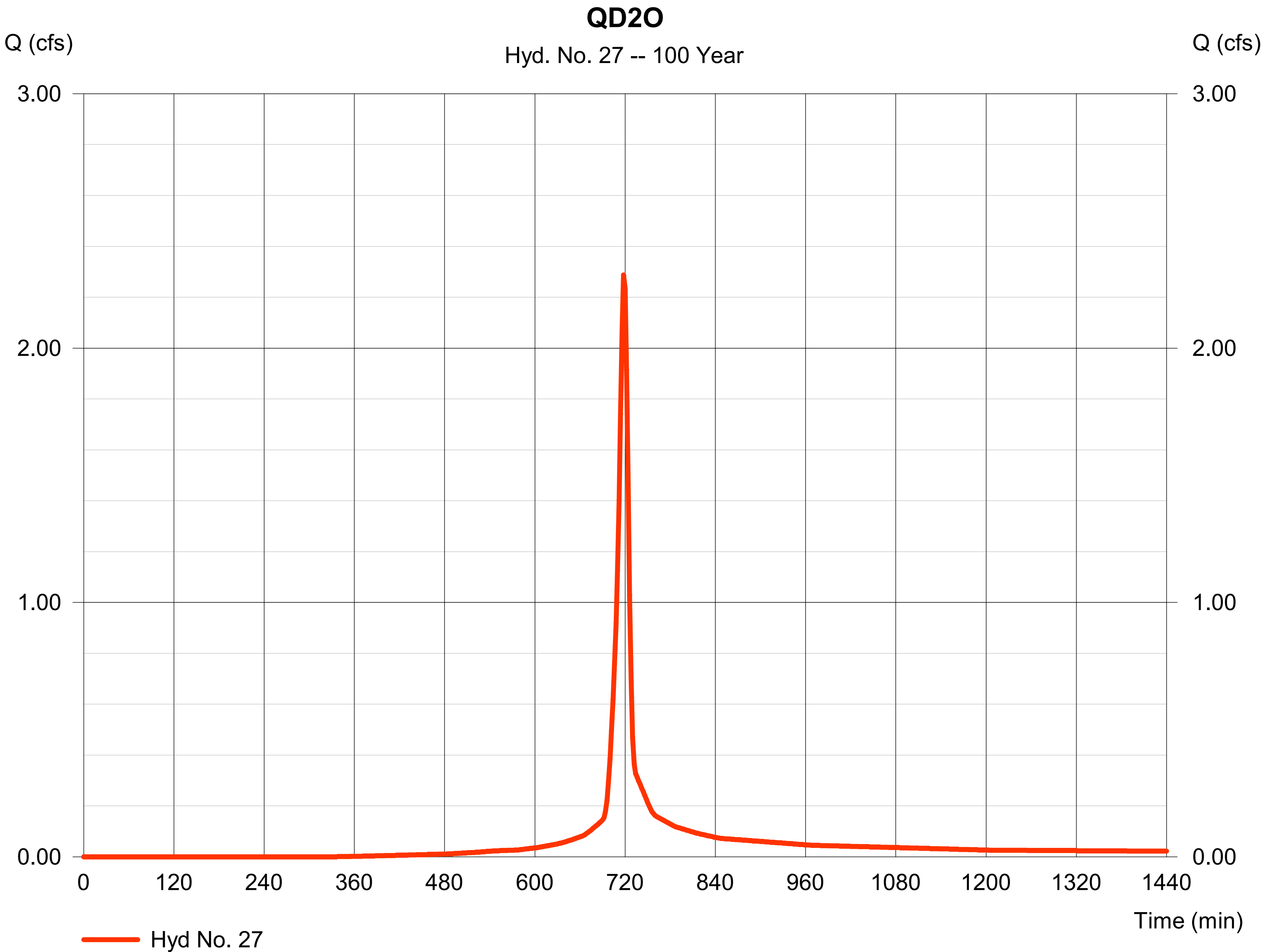
Modified Att-Kin routing method used.



Hyd. No. 27

QD2O

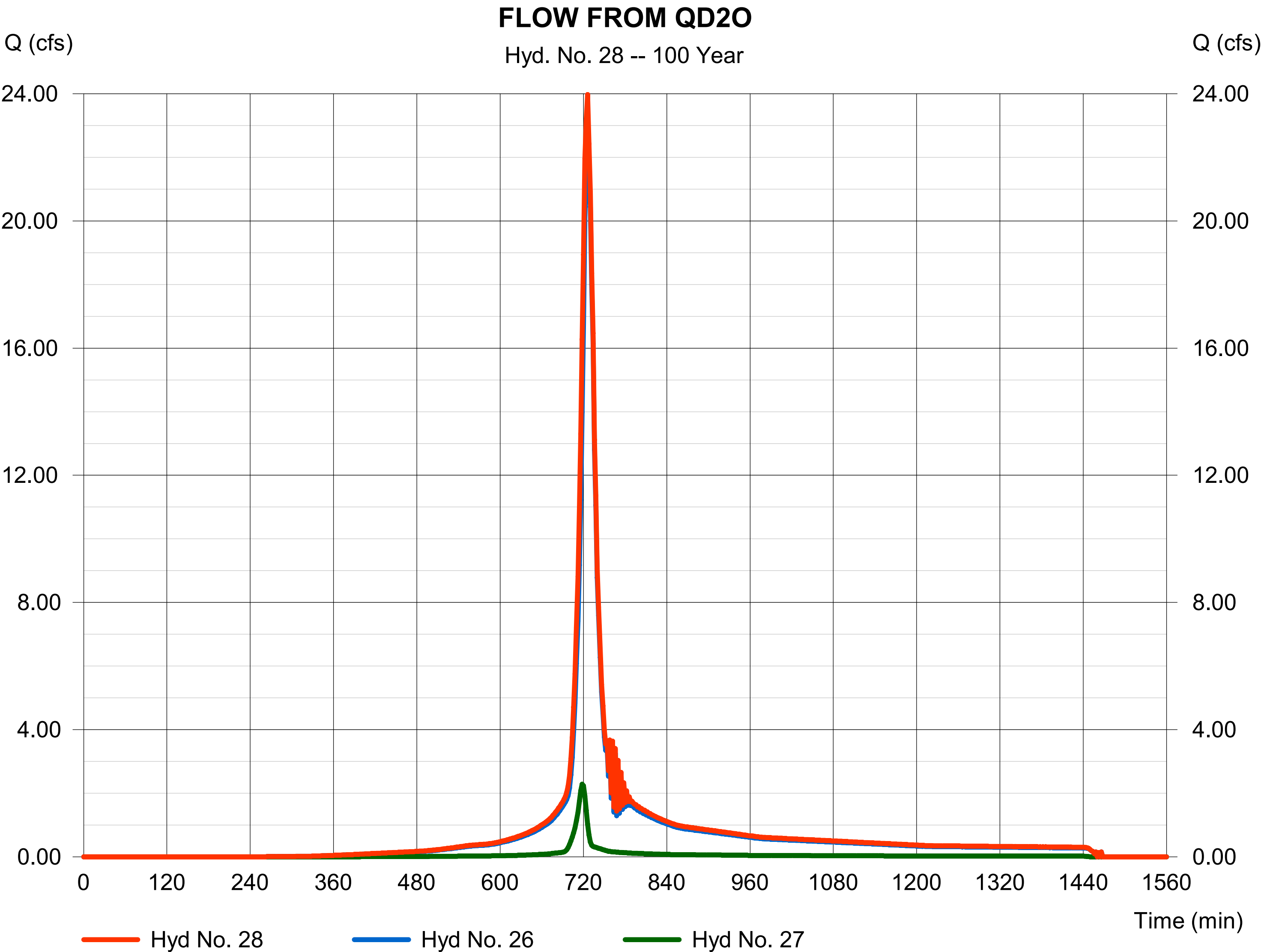
Hydrograph type	= SCS Runoff	Peak discharge	= 2.287 cfs
Storm frequency	= 100 yrs	Time to peak	= 718 min
Time interval	= 2 min	Hyd. volume	= 5,342 cuft
Drainage area	= 0.670 ac	Curve number	= 90
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 7.40 min
Total precip.	= 3.23 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hyd. No. 28

FLOW FROM QD2O

Hydrograph type	= Combine	Peak discharge	= 23.98 cfs
Storm frequency	= 100 yrs	Time to peak	= 726 min
Time interval	= 2 min	Hyd. volume	= 73,771 cuft
Inflow hyds.	= 26, 27	Contrib. drain. area	= 0.670 ac

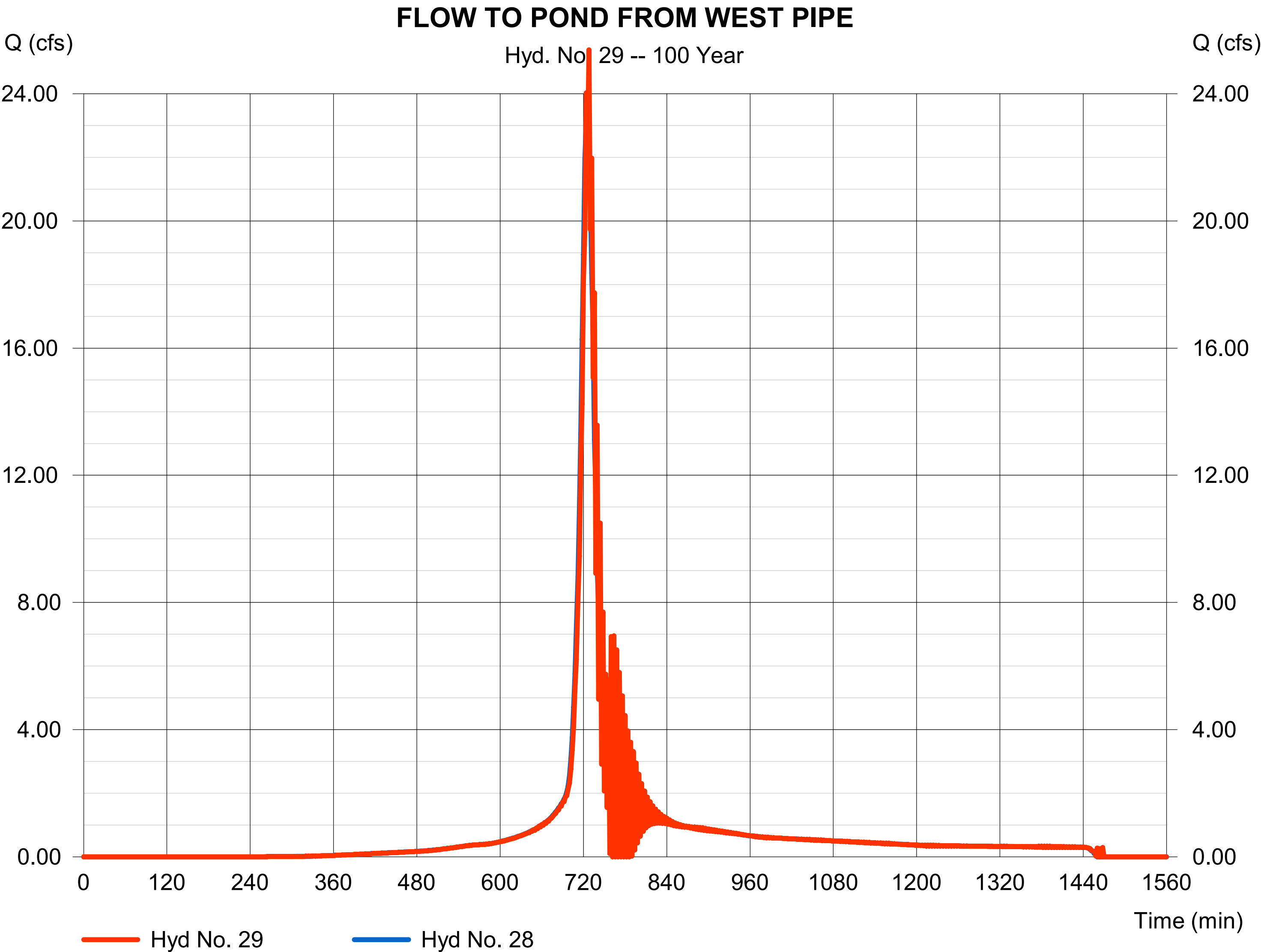


Hyd. No. 29

FLOW TO POND FROM WEST PIPE

Hydrograph type	= Reach	Peak discharge	= 25.37 cfs
Storm frequency	= 100 yrs	Time to peak	= 728 min
Time interval	= 2 min	Hyd. volume	= 74,693 cuft
Inflow hyd. No.	= 28 - FLOW FROM QD2O	Section type	= Circular
Reach length	= 27.1 ft	Channel slope	= 0.3 %
Manning's n	= 0.024	Bottom width	= 2.0 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.060	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.9094

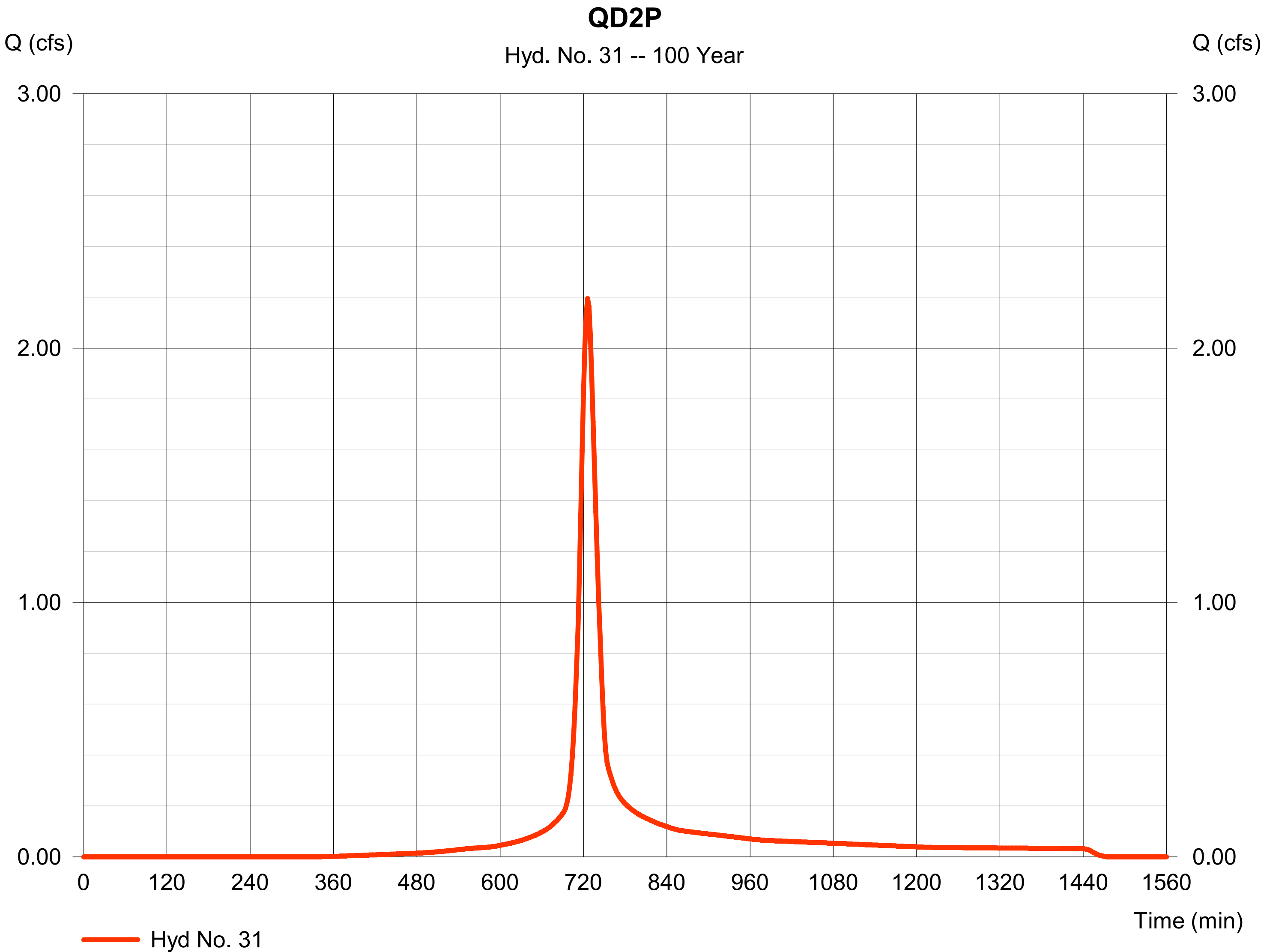
Modified Att-Kin routing method used.



Hyd. No. 31

QD2P

Hydrograph type	=	SCS Runoff	Peak discharge	=	2.194 cfs
Storm frequency	=	100 yrs	Time to peak	=	726 min
Time interval	=	2 min	Hyd. volume	=	7,628 cuft
Drainage area	=	0.940 ac	Curve number	=	90
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	22.40 min
Total precip.	=	3.23 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484

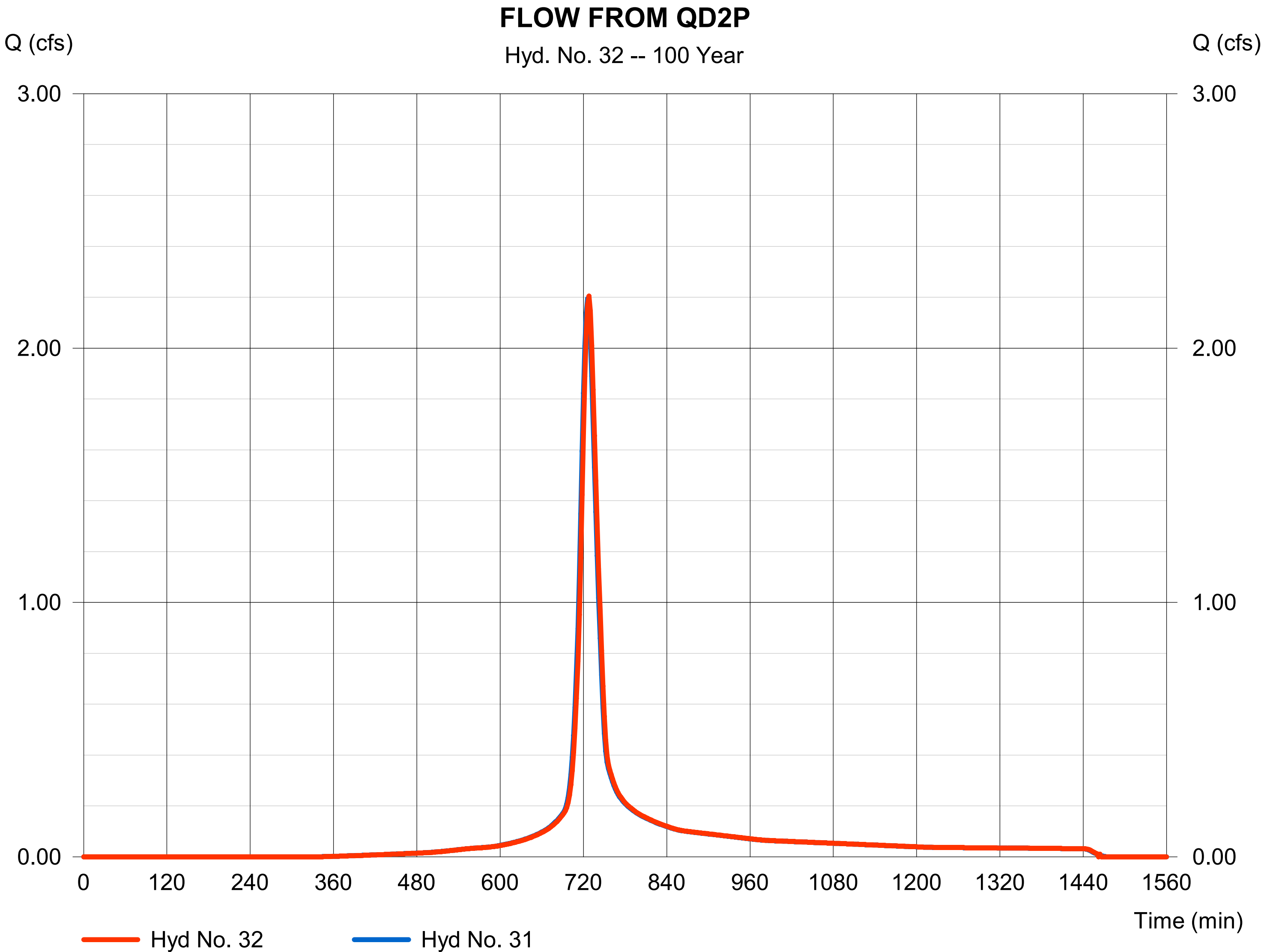


Hyd. No. 32

FLOW FROM QD2P

Hydrograph type	= Reach	Peak discharge	= 2.204 cfs
Storm frequency	= 100 yrs	Time to peak	= 728 min
Time interval	= 2 min	Hyd. volume	= 7,626 cuft
Inflow hyd. No.	= 31 - QD2P	Section type	= Circular
Reach length	= 36.3 ft	Channel slope	= 1.0 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 3.584	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.4396

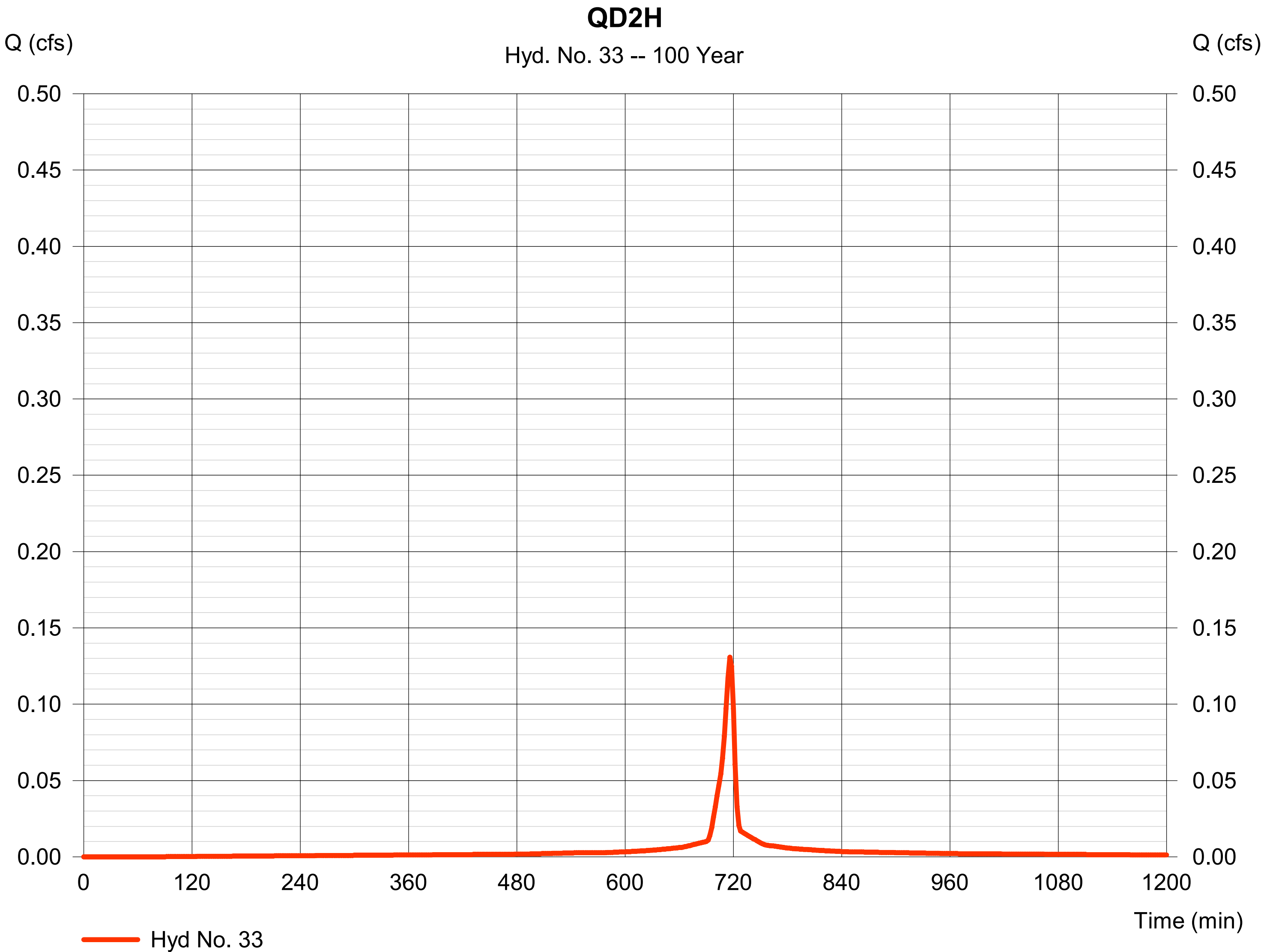
Modified Att-Kin routing method used.



Hyd. No. 33

QD2H

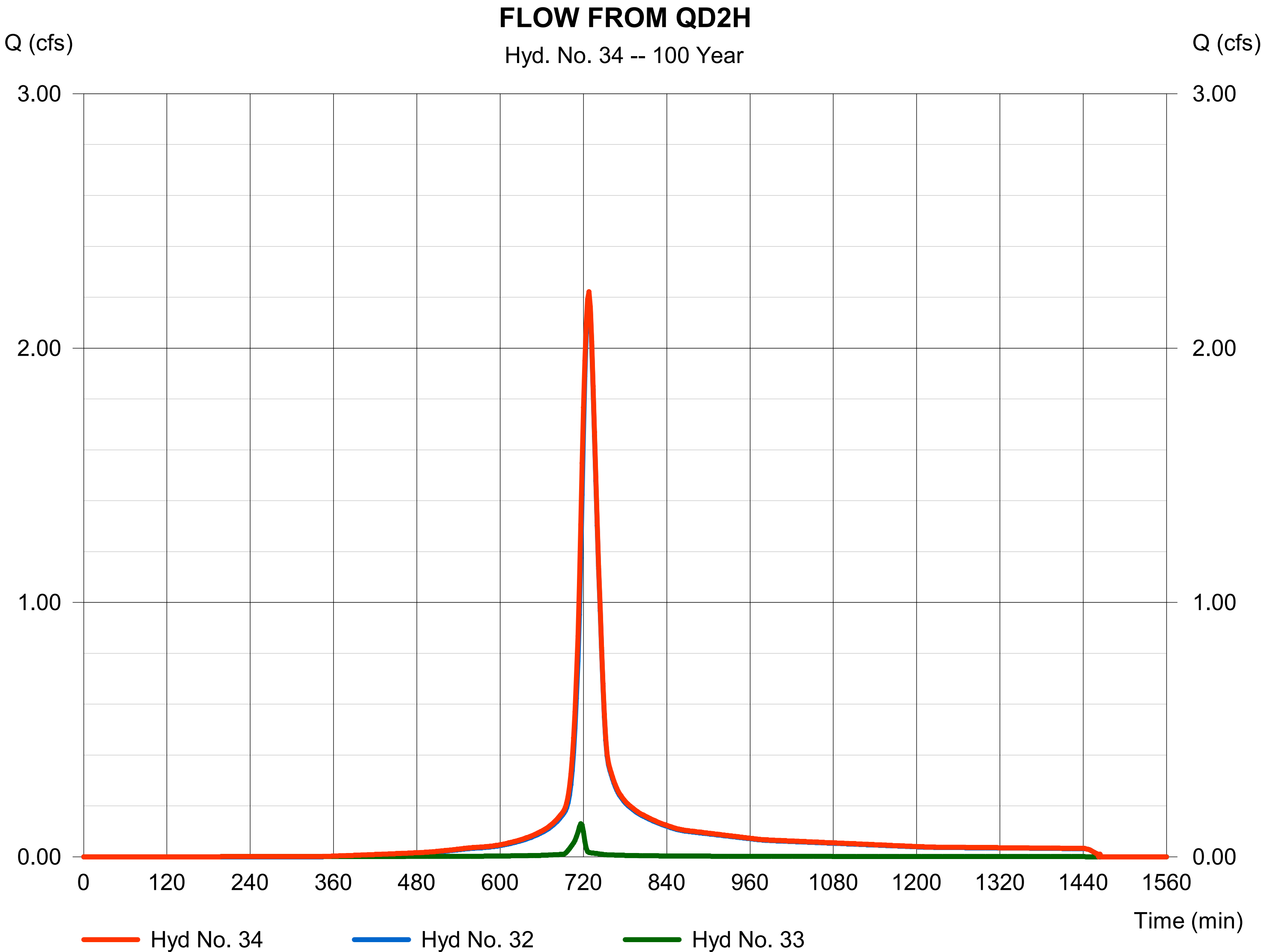
Hydrograph type	=	SCS Runoff	Peak discharge	=	0.131 cfs
Storm frequency	=	100 yrs	Time to peak	=	716 min
Time interval	=	2 min	Hyd. volume	=	306 cuft
Drainage area	=	0.030 ac	Curve number	=	98
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	6.00 min
Total precip.	=	3.23 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hyd. No. 34

FLOW FROM QD2H

Hydrograph type	= Combine	Peak discharge	= 2.221 cfs
Storm frequency	= 100 yrs	Time to peak	= 728 min
Time interval	= 2 min	Hyd. volume	= 7,932 cuft
Inflow hyds.	= 32, 33	Contrib. drain. area	= 0.030 ac

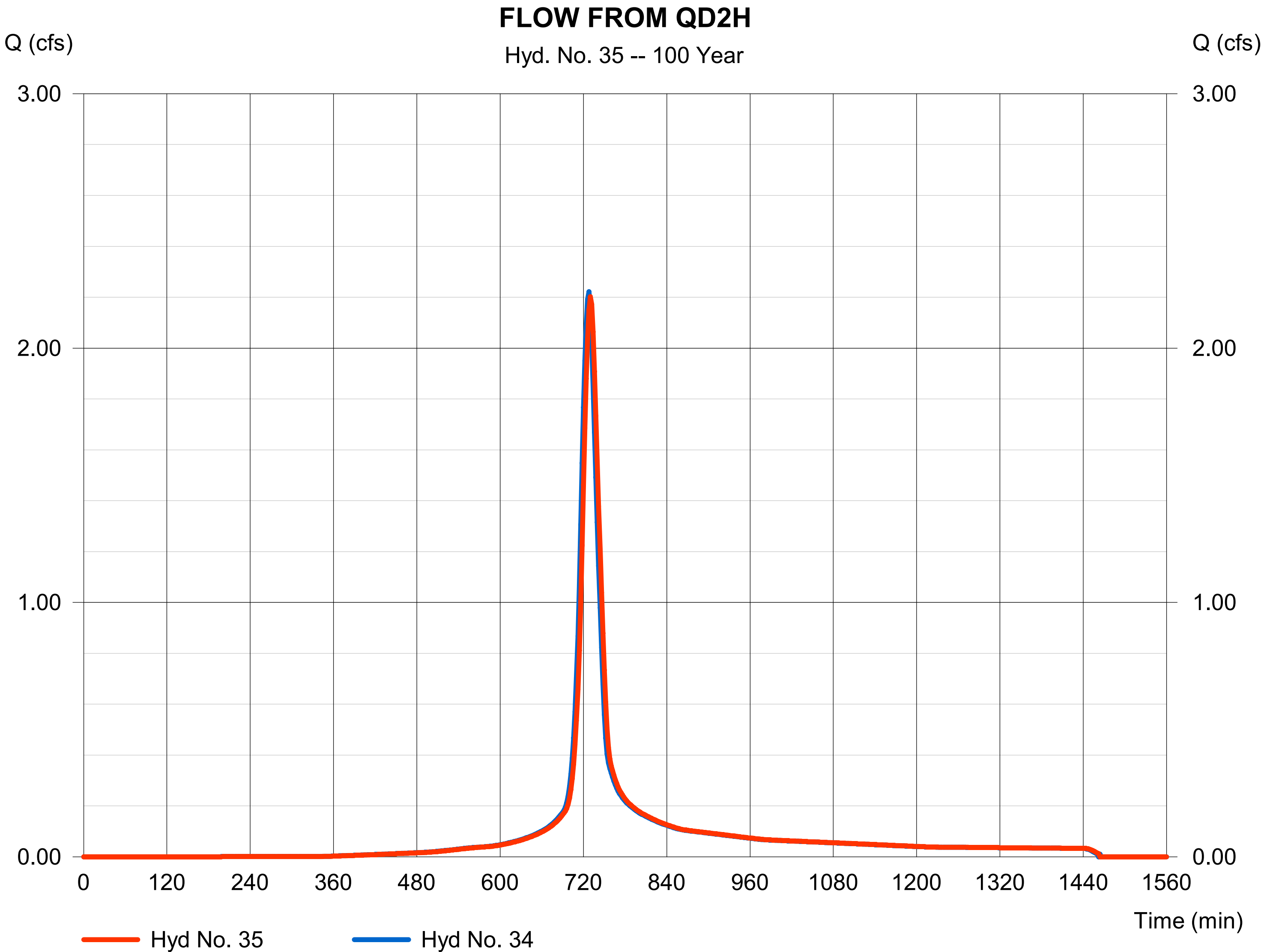


Hyd. No. 35

FLOW FROM QD2H

Hydrograph type	= Reach	Peak discharge	= 2.202 cfs
Storm frequency	= 100 yrs	Time to peak	= 730 min
Time interval	= 2 min	Hyd. volume	= 7,930 cuft
Inflow hyd. No.	= 34 - FLOW FROM QD2H	Section type	= Circular
Reach length	= 162.5 ft	Channel slope	= 1.0 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 3.584	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 0.7342

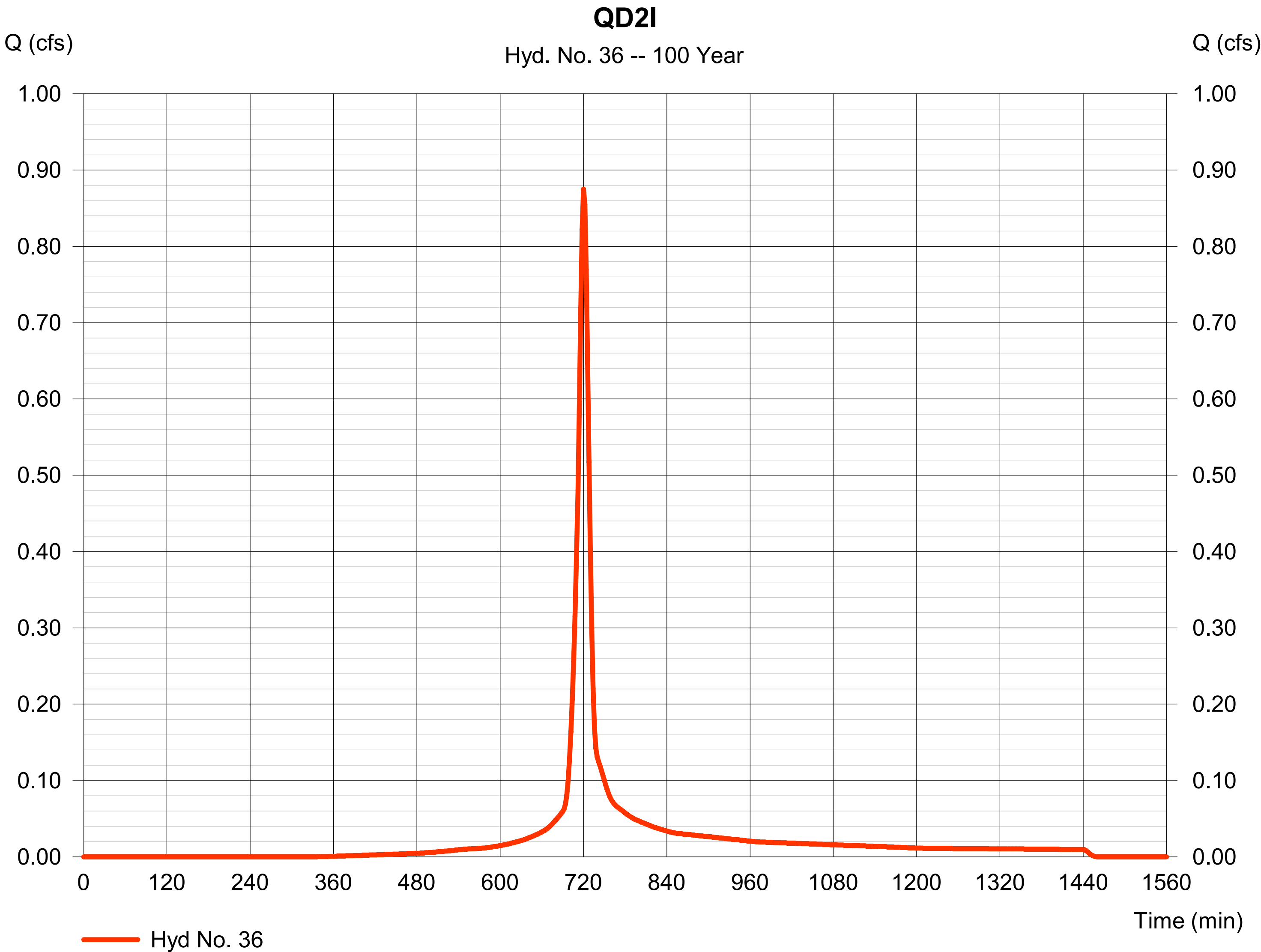
Modified Att-Kin routing method used.



Hyd. No. 36

QD2I

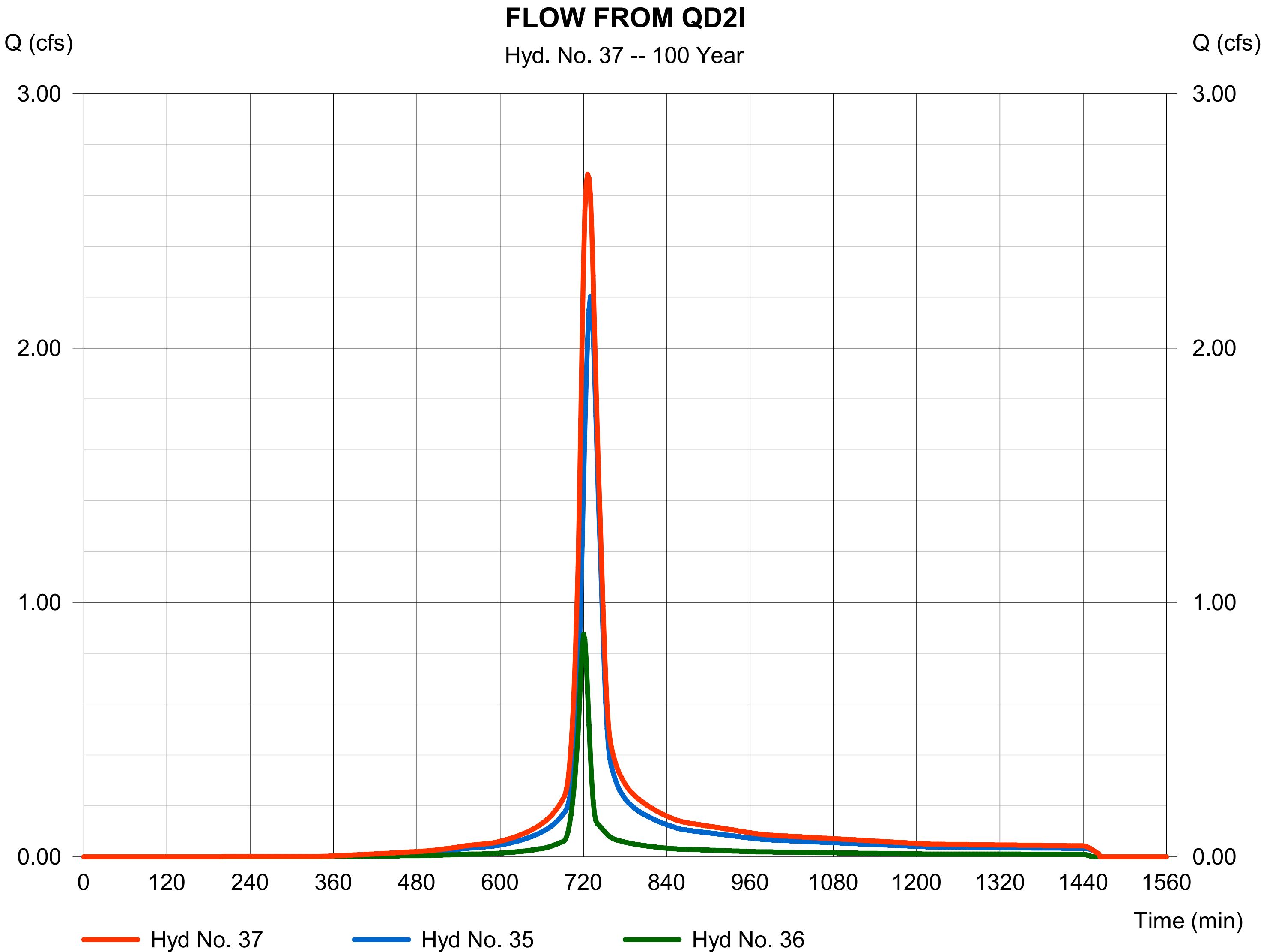
Hydrograph type	= SCS Runoff	Peak discharge	= 0.875 cfs
Storm frequency	= 100 yrs	Time to peak	= 720 min
Time interval	= 2 min	Hyd. volume	= 2,302 cuft
Drainage area	= 0.280 ac	Curve number	= 90
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 12.30 min
Total precip.	= 3.23 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hyd. No. 37

FLOW FROM QD2I

Hydrograph type	= Combine	Peak discharge	= 2.683 cfs
Storm frequency	= 100 yrs	Time to peak	= 726 min
Time interval	= 2 min	Hyd. volume	= 10,232 cuft
Inflow hyds.	= 35, 36	Contrib. drain. area	= 0.280 ac

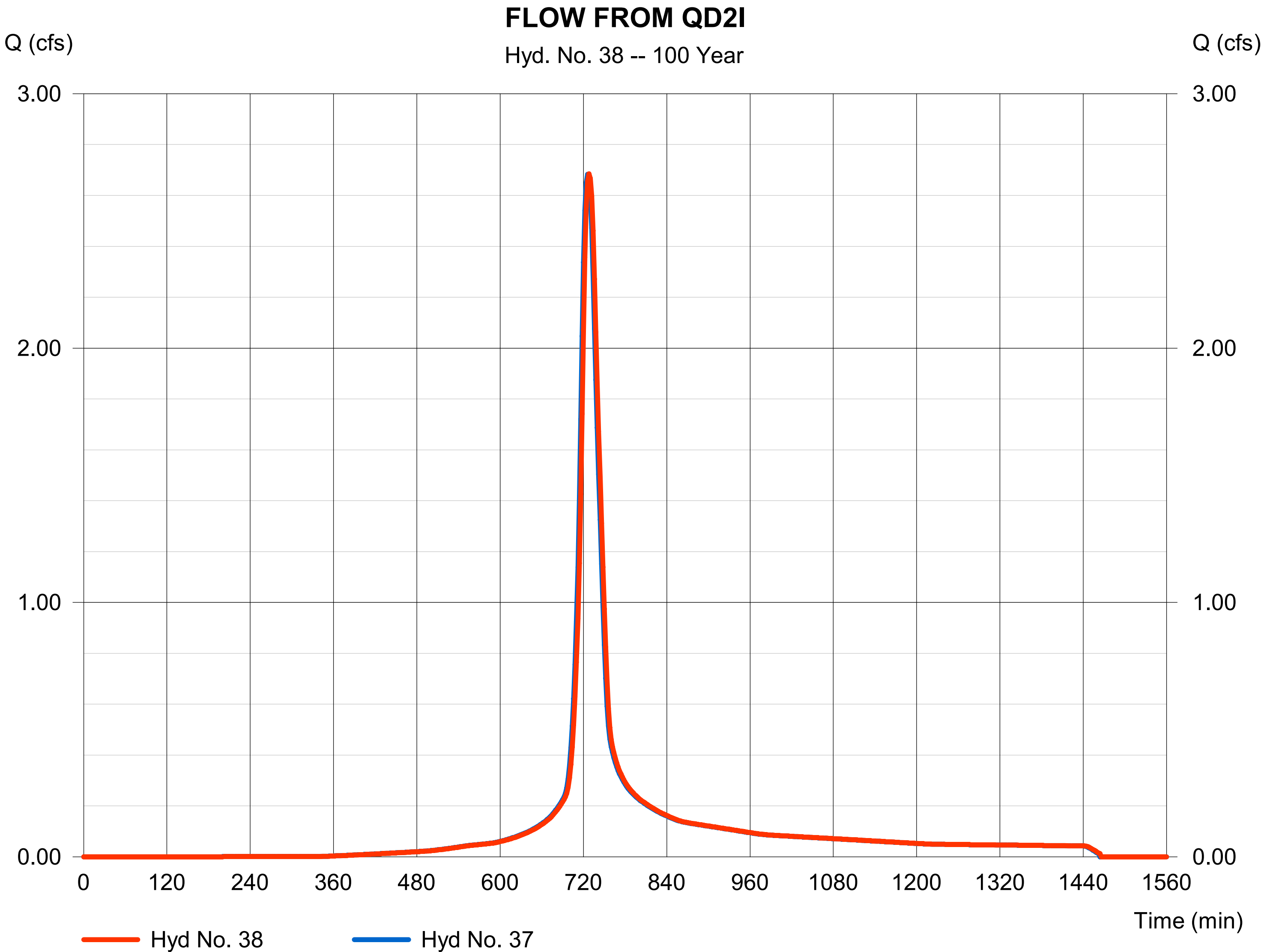


Hyd. No. 38

FLOW FROM QD2I

Hydrograph type	= Reach	Peak discharge	= 2.685 cfs
Storm frequency	= 100 yrs	Time to peak	= 728 min
Time interval	= 2 min	Hyd. volume	= 10,233 cuft
Inflow hyd. No.	= 37 - FLOW FROM QD2I	Section type	= Circular
Reach length	= 97.5 ft	Channel slope	= 0.7 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.934	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.0777

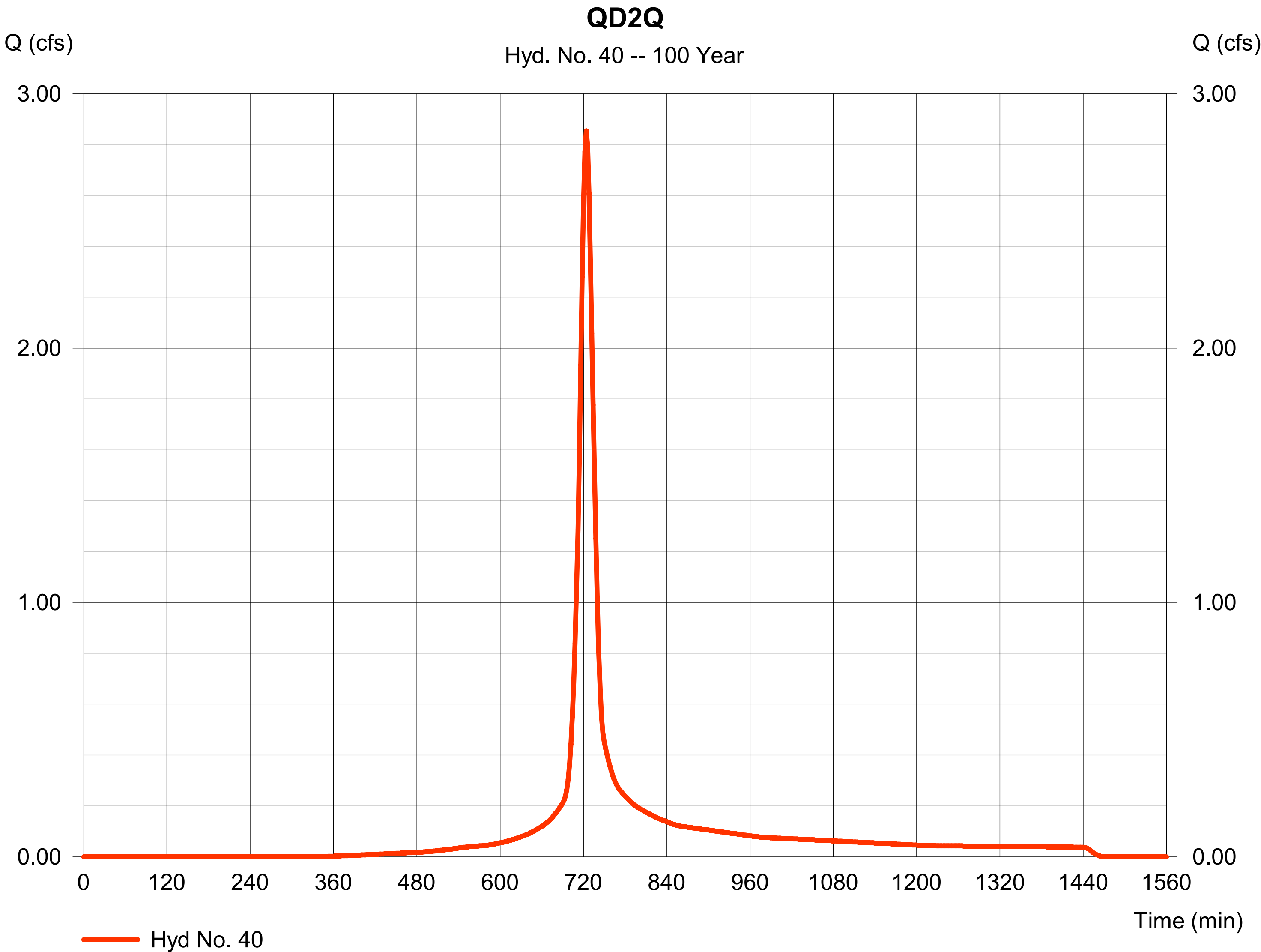
Modified Att-Kin routing method used.



Hyd. No. 40

QD2Q

Hydrograph type	=	SCS Runoff	Peak discharge	=	2.854 cfs
Storm frequency	=	100 yrs	Time to peak	=	724 min
Time interval	=	2 min	Hyd. volume	=	9,009 cuft
Drainage area	=	1.130 ac	Curve number	=	90
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	16.90 min
Total precip.	=	3.23 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484

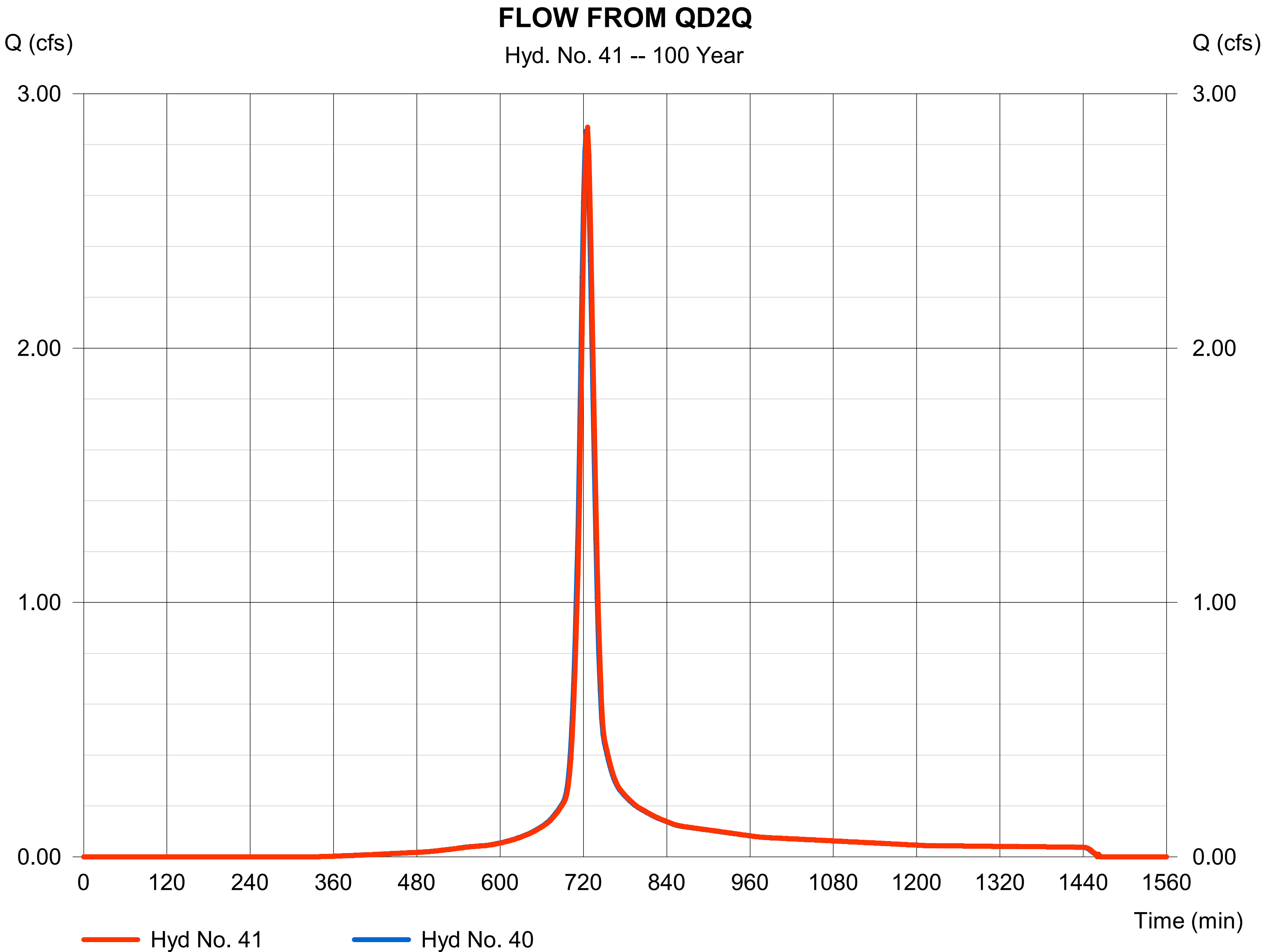


Hyd. No. 41

FLOW FROM QD2Q

Hydrograph type	= Reach	Peak discharge	= 2.868 cfs
Storm frequency	= 100 yrs	Time to peak	= 726 min
Time interval	= 2 min	Hyd. volume	= 9,008 cuft
Inflow hyd. No.	= 40 - QD2Q	Section type	= Circular
Reach length	= 36.9 ft	Channel slope	= 0.7 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.999	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.5326

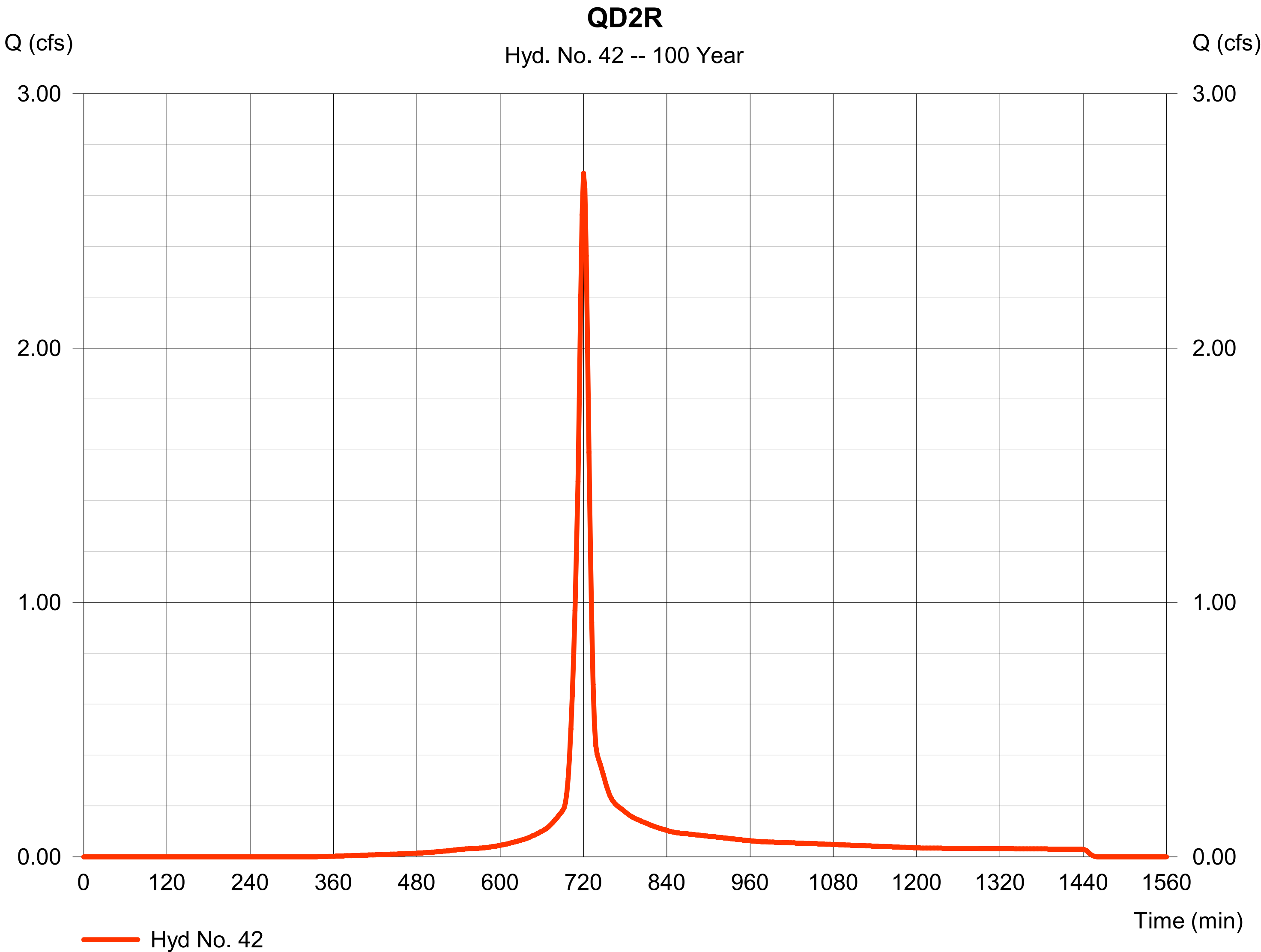
Modified Att-Kin routing method used.



Hyd. No. 42

QD2R

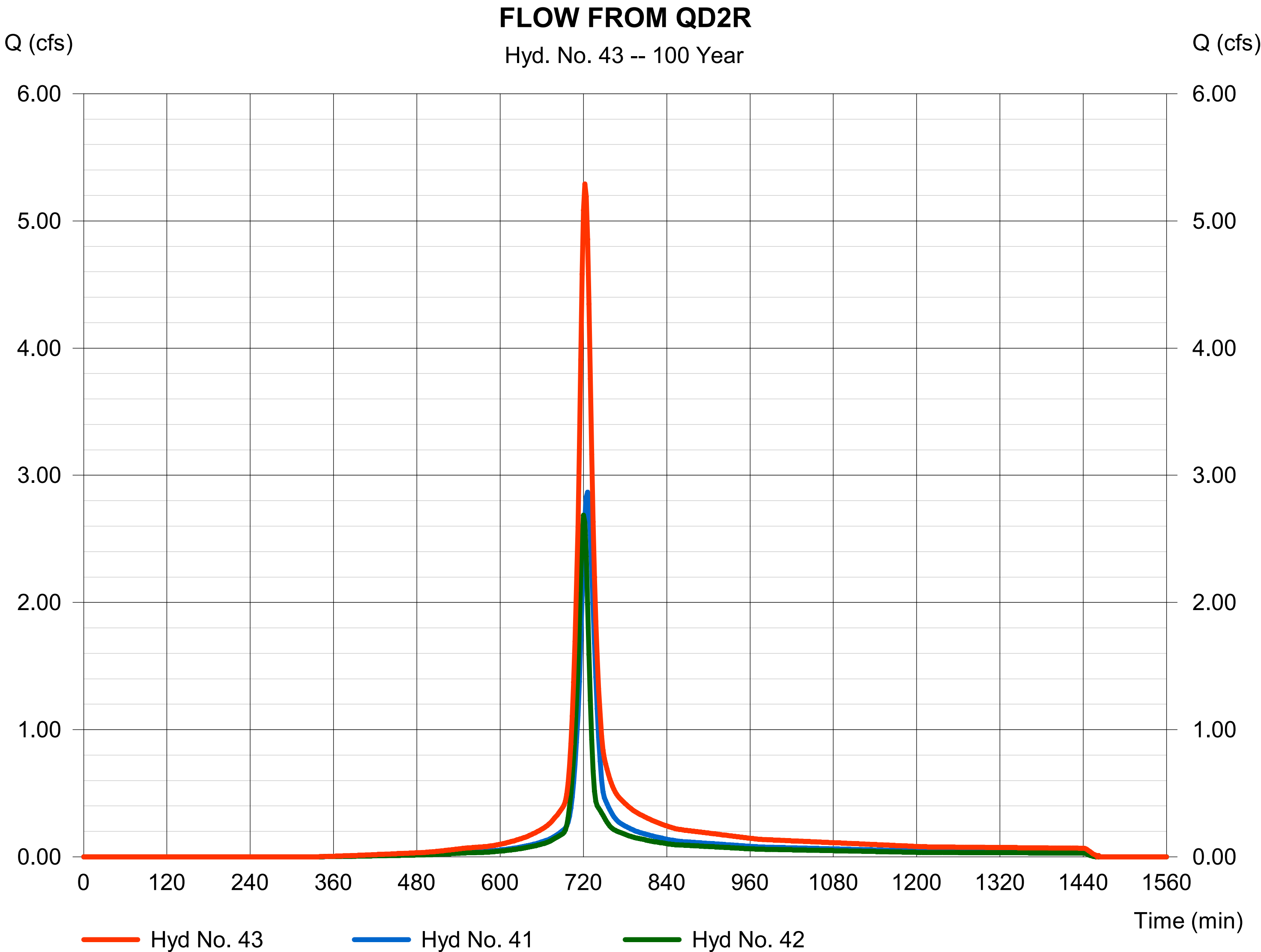
Hydrograph type	=	SCS Runoff	Peak discharge	=	2.687 cfs
Storm frequency	=	100 yrs	Time to peak	=	720 min
Time interval	=	2 min	Hyd. volume	=	7,071 cuft
Drainage area	=	0.860 ac	Curve number	=	90
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	13.10 min
Total precip.	=	3.23 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hyd. No. 43

FLOW FROM QD2R

Hydrograph type	= Combine	Peak discharge	= 5.291 cfs
Storm frequency	= 100 yrs	Time to peak	= 722 min
Time interval	= 2 min	Hyd. volume	= 16,079 cuft
Inflow hyds.	= 41, 42	Contrib. drain. area	= 0.860 ac

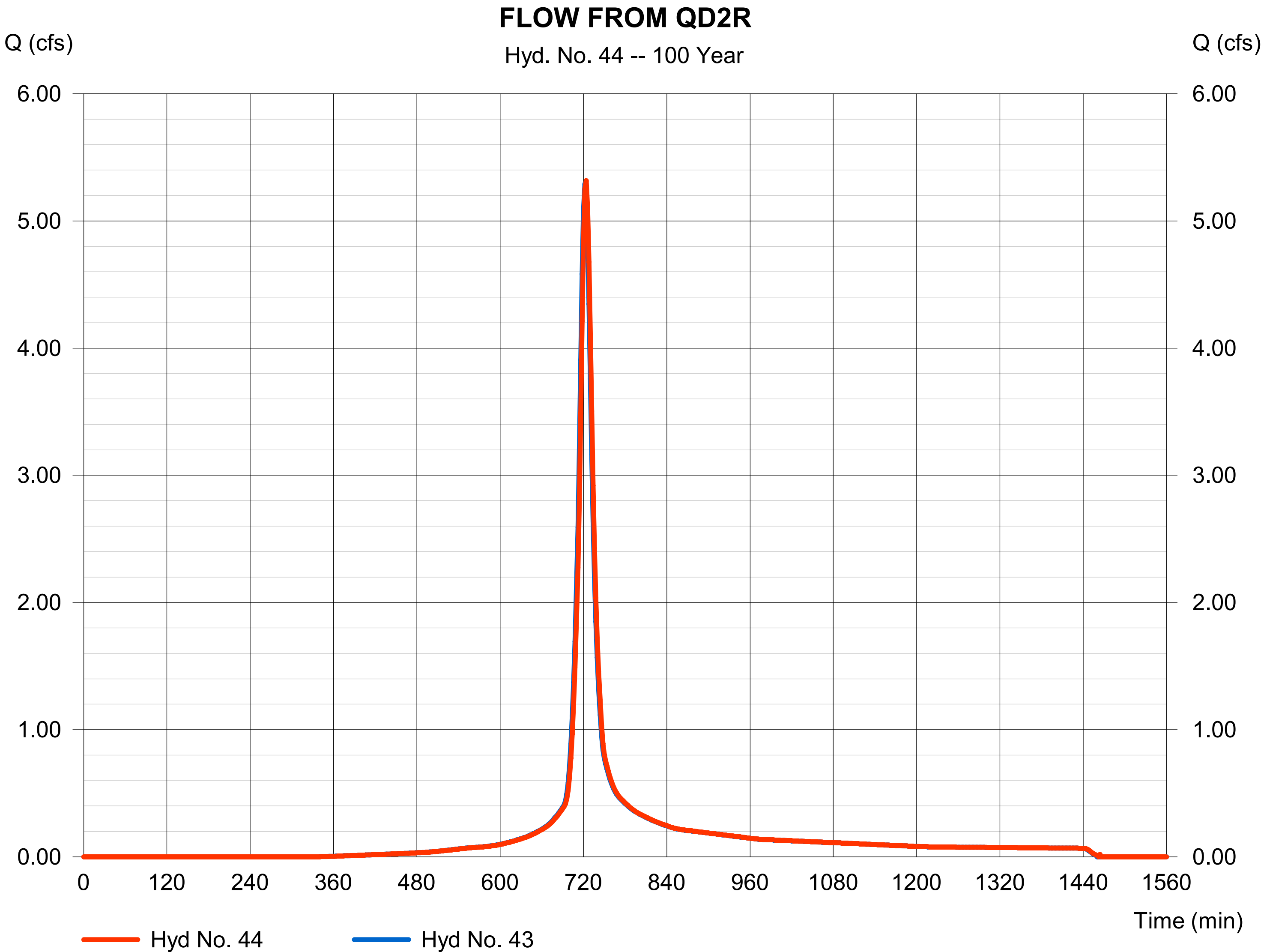


Hyd. No. 44

FLOW FROM QD2R

Hydrograph type	= Reach	Peak discharge	= 5.317 cfs
Storm frequency	= 100 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 16,081 cuft
Inflow hyd. No.	= 43 - FLOW FROM QD2R	Section type	= Circular
Reach length	= 36.3 ft	Channel slope	= 1.3 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 4.102	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.7220

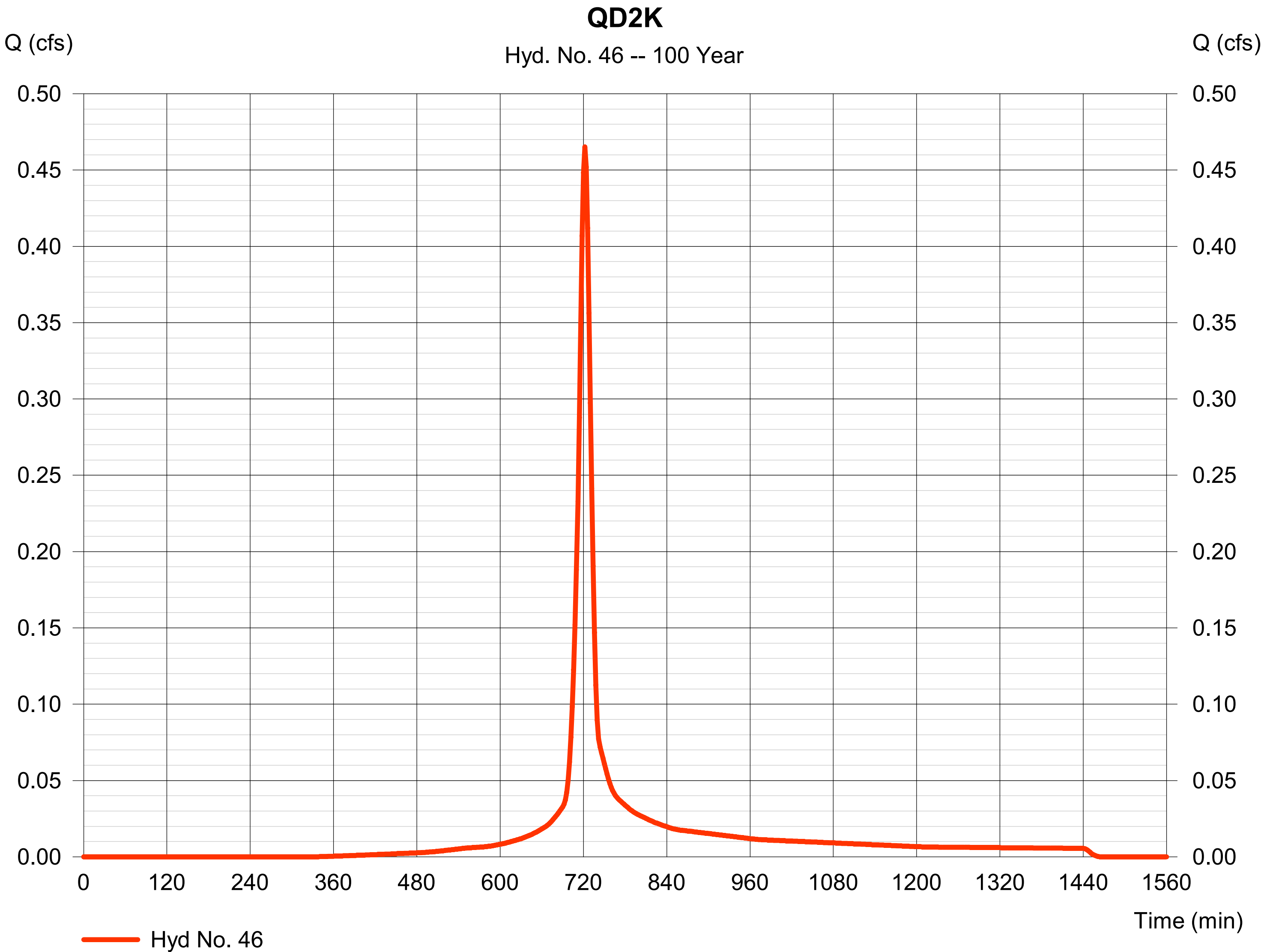
Modified Att-Kin routing method used.



Hyd. No. 46

QD2K

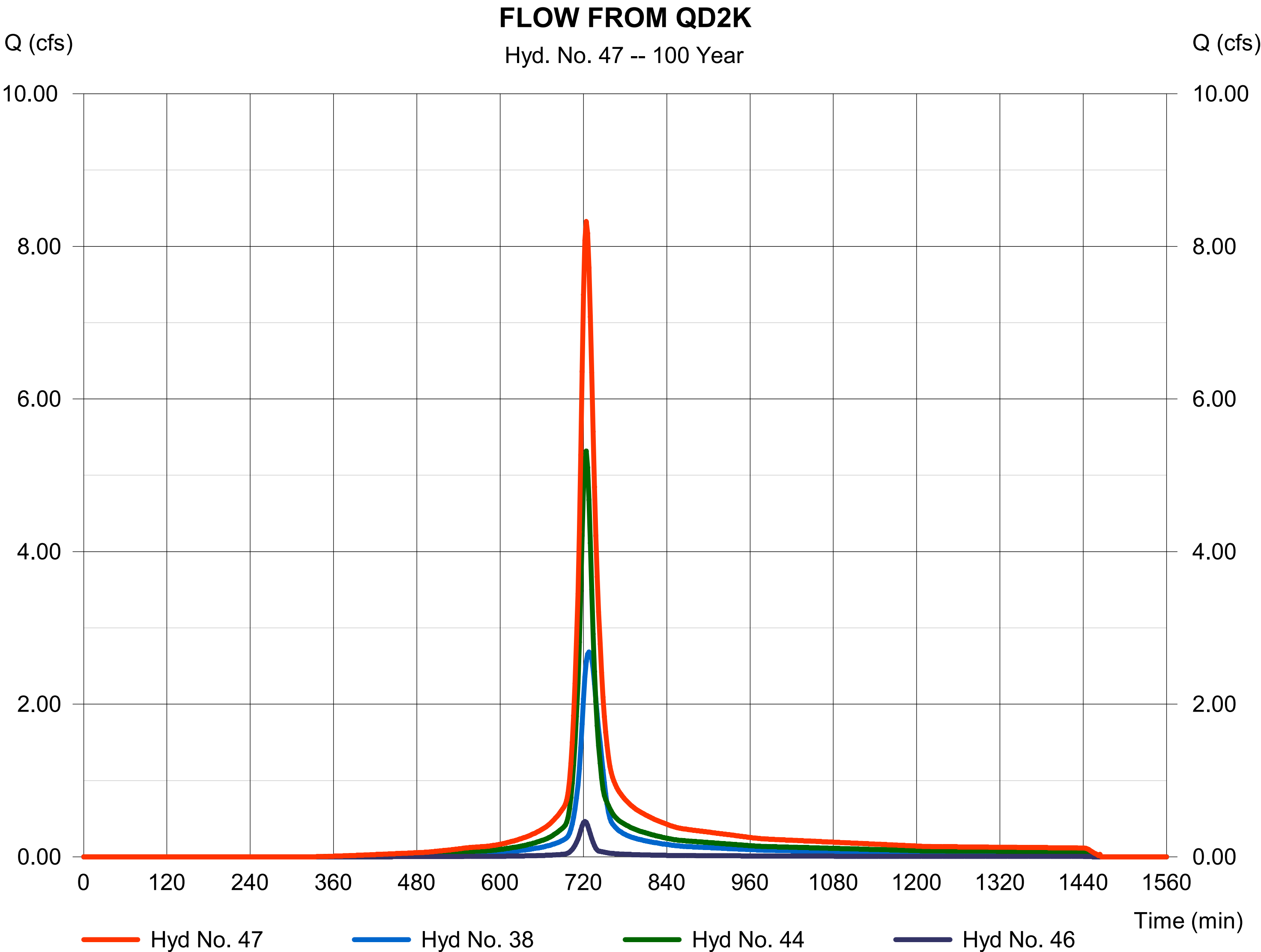
Hydrograph type	= SCS Runoff	Peak discharge	= 0.465 cfs
Storm frequency	= 100 yrs	Time to peak	= 722 min
Time interval	= 2 min	Hyd. volume	= 1,322 cuft
Drainage area	= 0.170 ac	Curve number	= 90
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 13.80 min
Total precip.	= 3.23 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hyd. No. 47

FLOW FROM QD2K

Hydrograph type	= Combine	Peak discharge	= 8.324 cfs
Storm frequency	= 100 yrs	Time to peak	= 724 min
Time interval	= 2 min	Hyd. volume	= 27,635 cuft
Inflow hyds.	= 38, 44, 46	Contrib. drain. area	= 0.170 ac

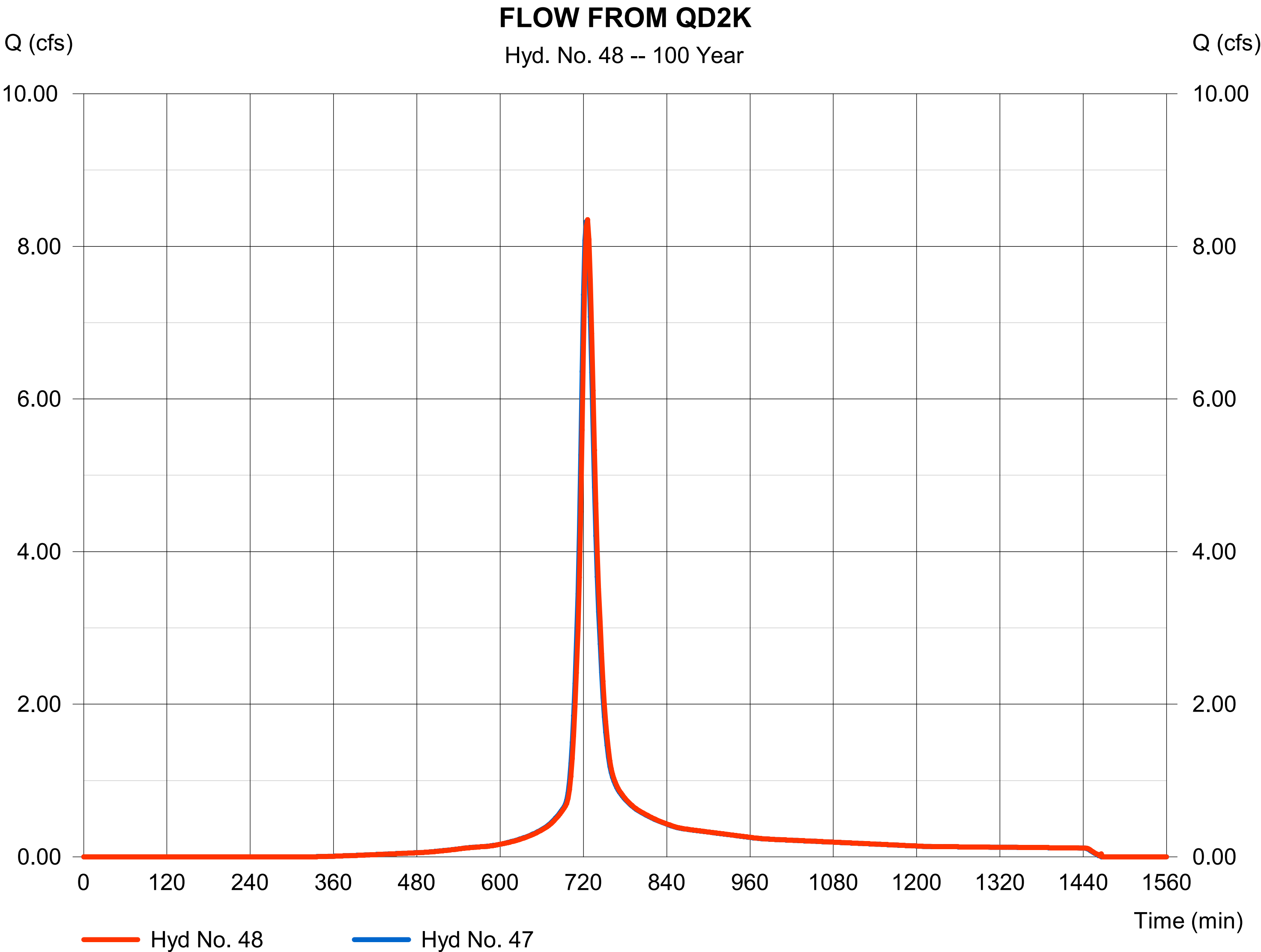


Hyd. No. 48

FLOW FROM QD2K

Hydrograph type	= Reach	Peak discharge	= 8.352 cfs
Storm frequency	= 100 yrs	Time to peak	= 726 min
Time interval	= 2 min	Hyd. volume	= 27,636 cuft
Inflow hyd. No.	= 47 - FLOW FROM QD2K	Section type	= Circular
Reach length	= 75.6 ft	Channel slope	= 0.3 %
Manning's n	= 0.024	Bottom width	= 2.0 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.060	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.4491

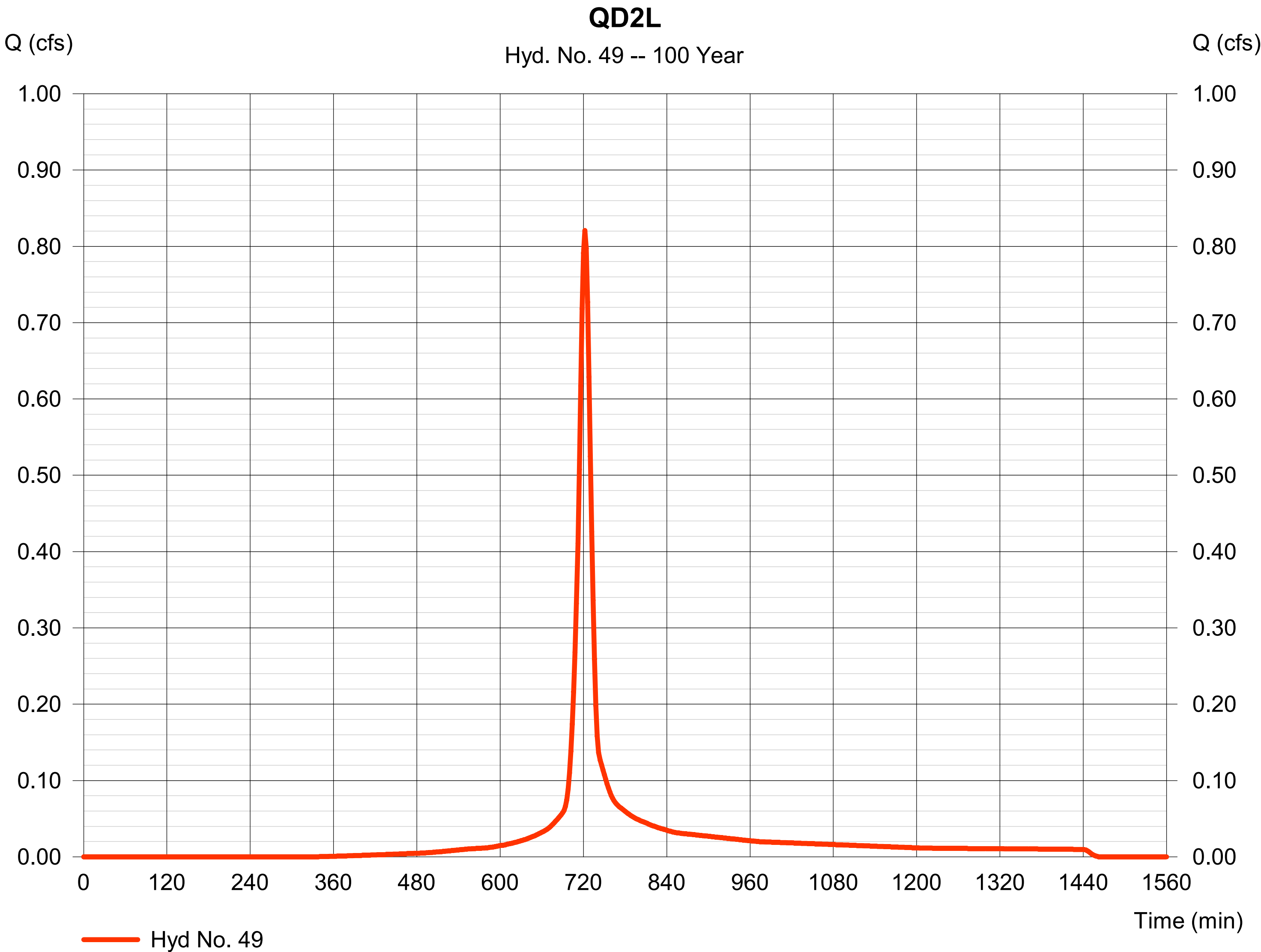
Modified Att-Kin routing method used.



Hyd. No. 49

QD2L

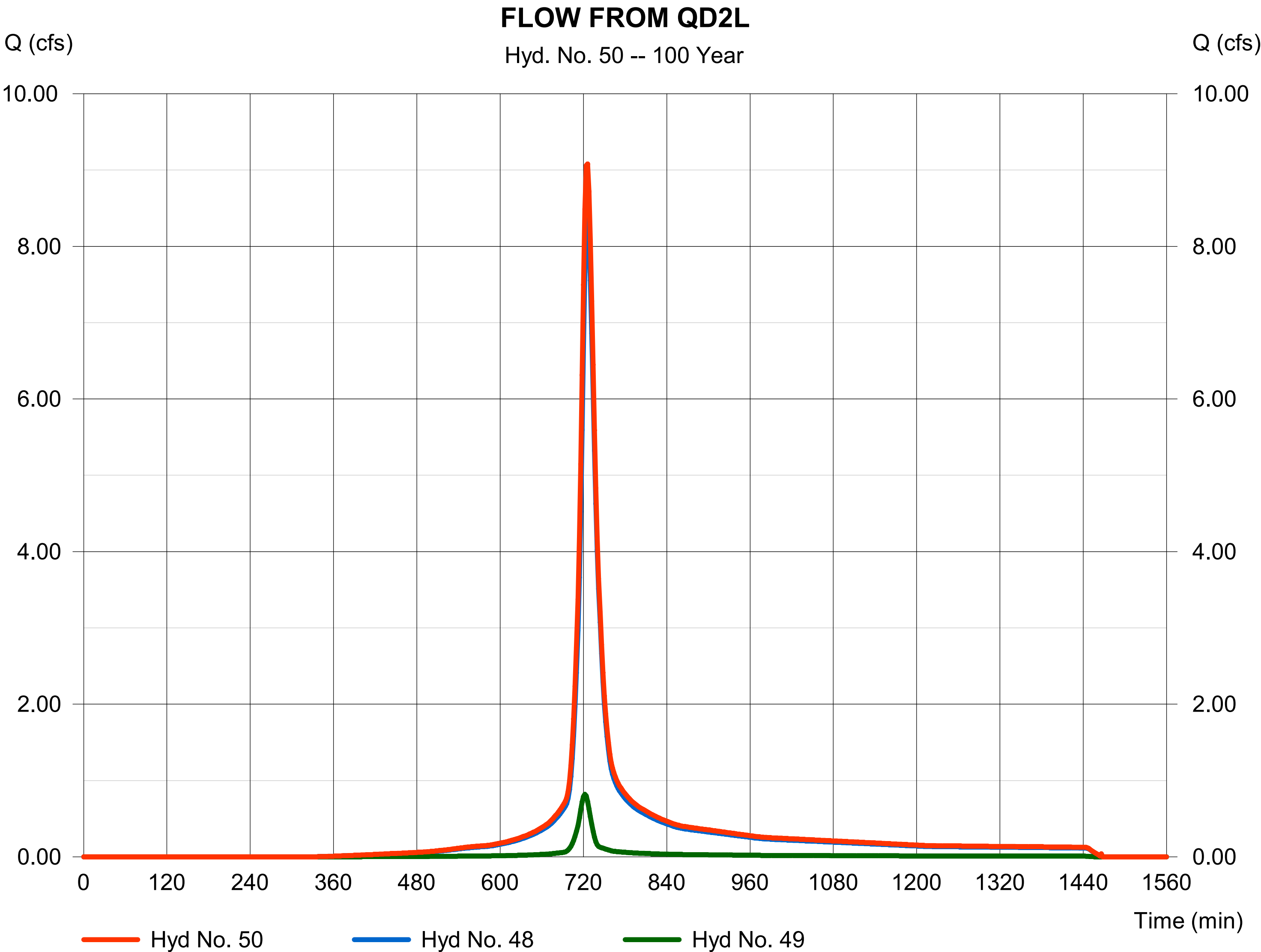
Hydrograph type	=	SCS Runoff	Peak discharge	=	0.821 cfs
Storm frequency	=	100 yrs	Time to peak	=	722 min
Time interval	=	2 min	Hyd. volume	=	2,332 cuft
Drainage area	=	0.300 ac	Curve number	=	90
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	14.30 min
Total precip.	=	3.23 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hyd. No. 50

FLOW FROM QD2L

Hydrograph type	= Combine	Peak discharge	= 9.079 cfs
Storm frequency	= 100 yrs	Time to peak	= 726 min
Time interval	= 2 min	Hyd. volume	= 29,968 cuft
Inflow hyds.	= 48, 49	Contrib. drain. area	= 0.300 ac

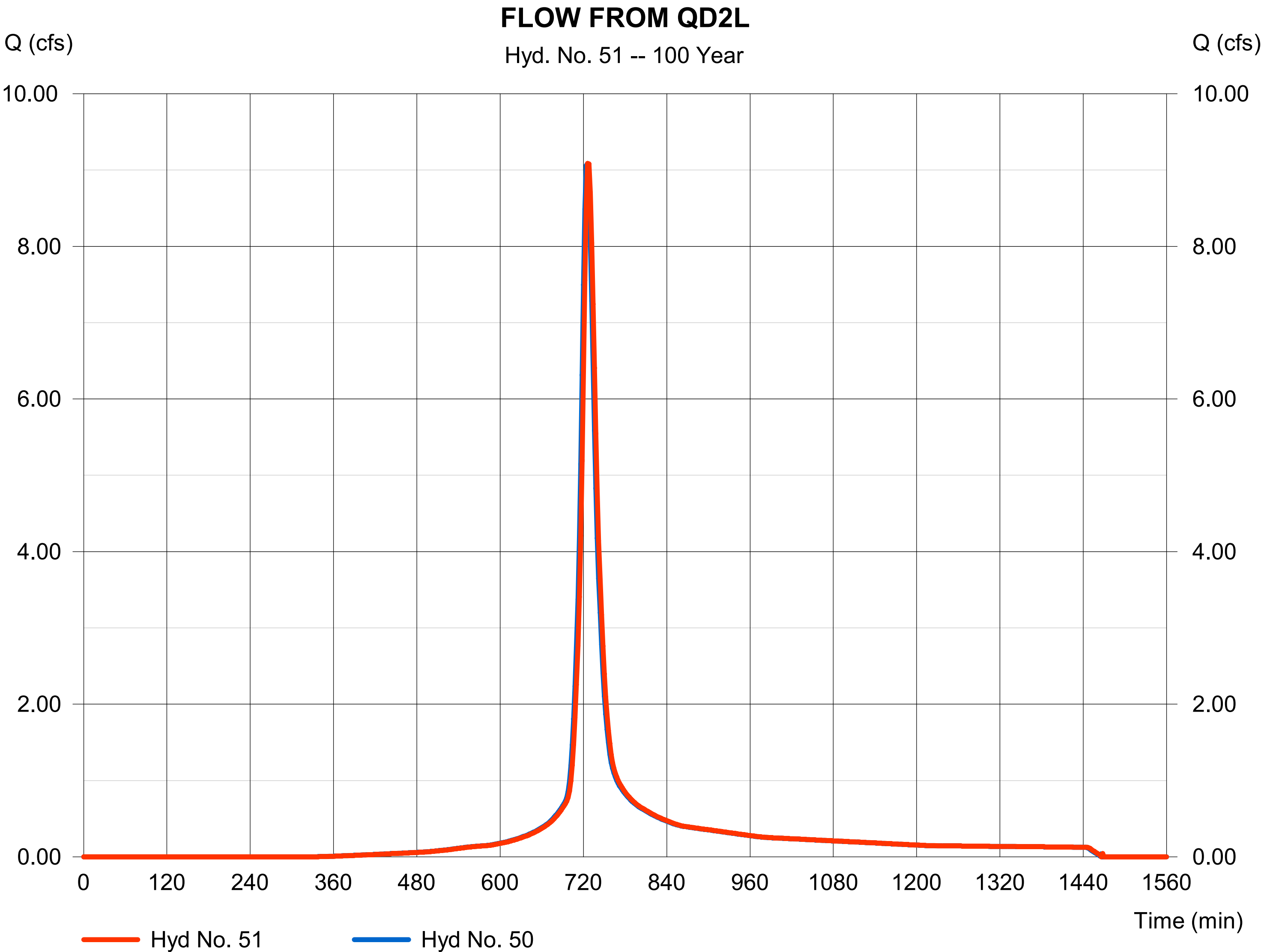


Hyd. No. 51

FLOW FROM QD2L

Hydrograph type	= Reach	Peak discharge	= 9.088 cfs
Storm frequency	= 100 yrs	Time to peak	= 726 min
Time interval	= 2 min	Hyd. volume	= 29,967 cuft
Inflow hyd. No.	= 50 - FLOW FROM QD2L	Section type	= Circular
Reach length	= 194.8 ft	Channel slope	= 0.3 %
Manning's n	= 0.024	Bottom width	= 2.0 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.060	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.0532

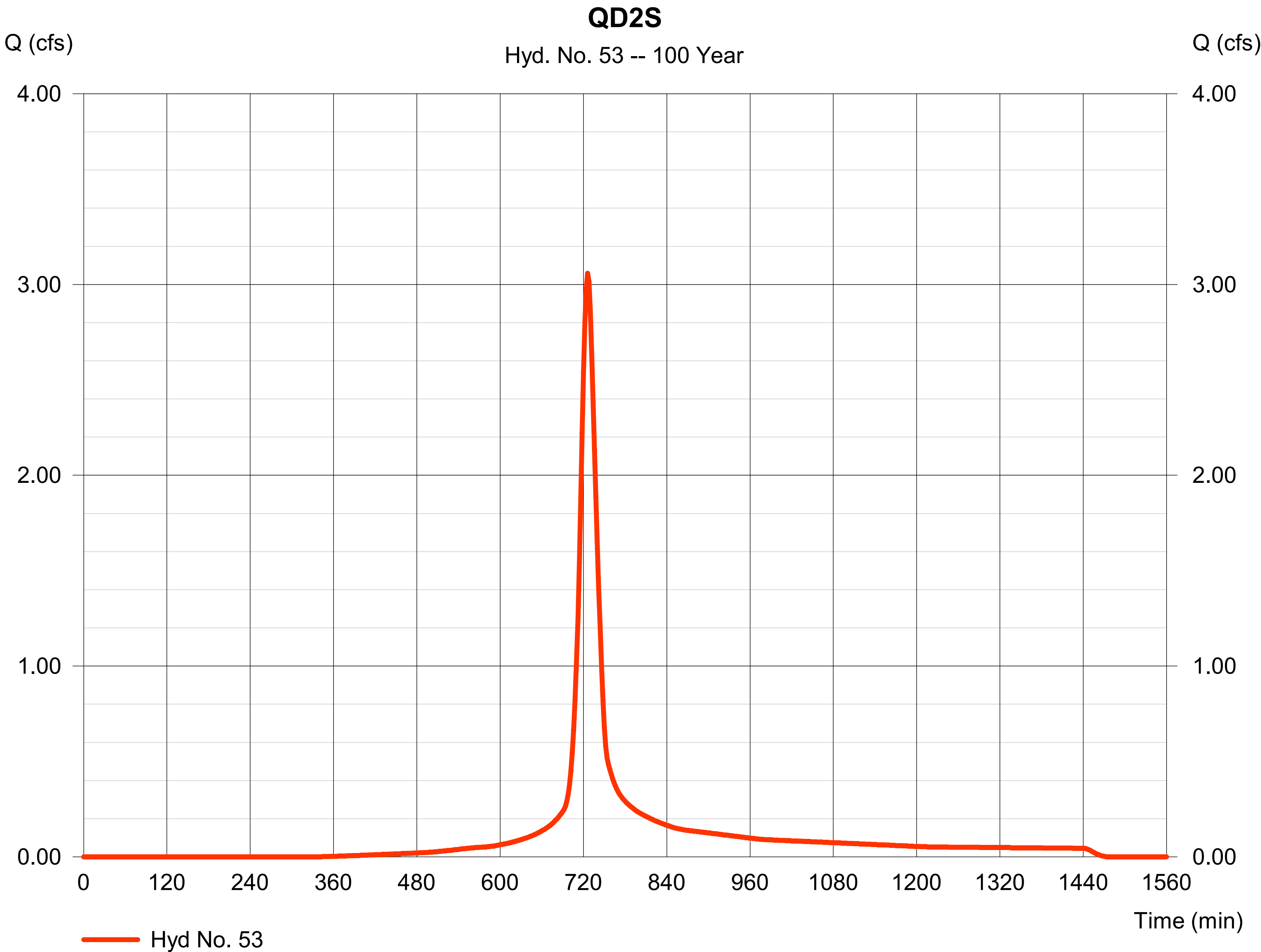
Modified Att-Kin routing method used.



Hyd. No. 53

QD2S

Hydrograph type	=	SCS Runoff	Peak discharge	=	3.058 cfs
Storm frequency	=	100 yrs	Time to peak	=	726 min
Time interval	=	2 min	Hyd. volume	=	10,631 cuft
Drainage area	=	1.310 ac	Curve number	=	90
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	23.20 min
Total precip.	=	3.23 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484

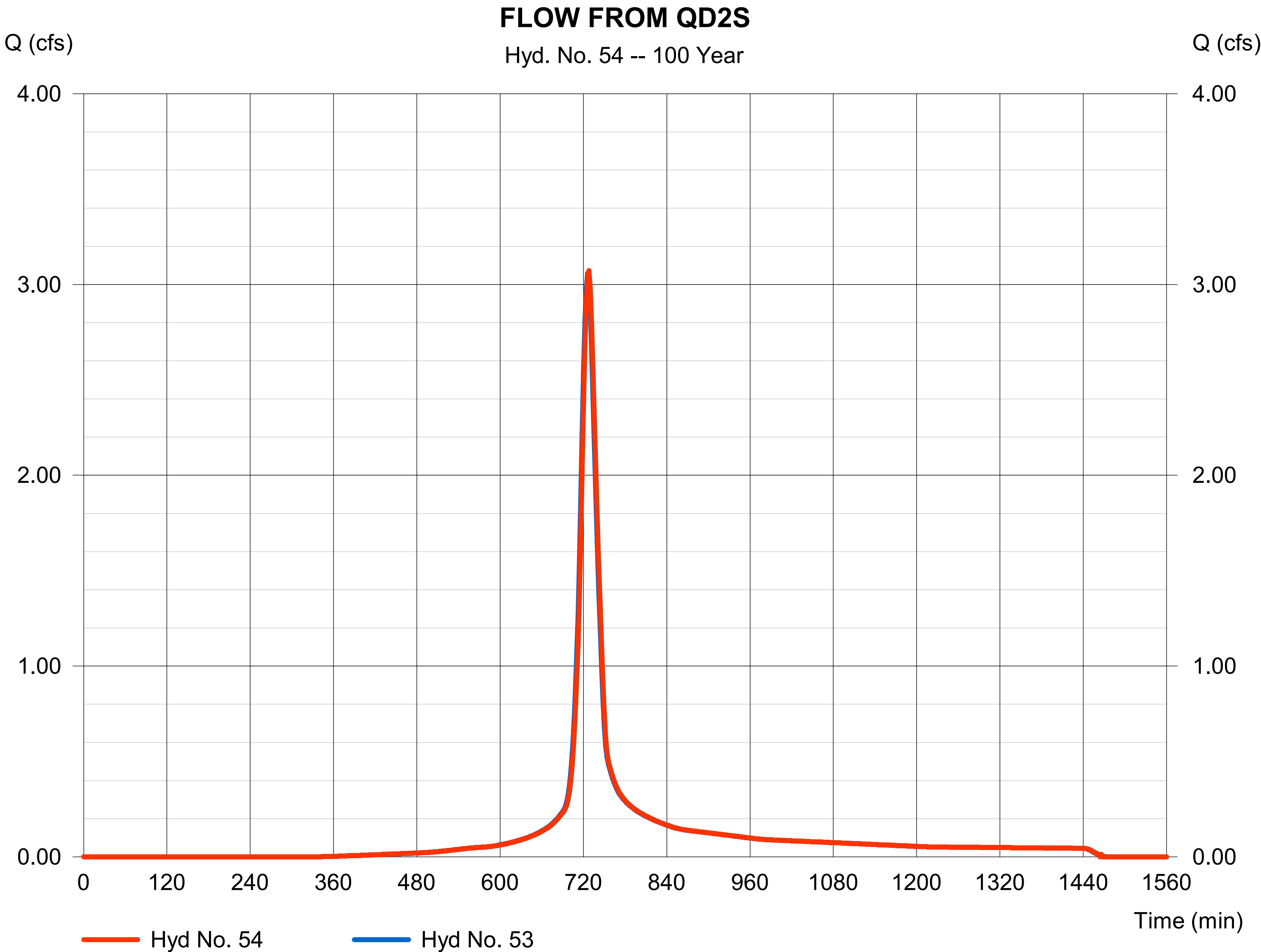


Hyd. No. 54

FLOW FROM QD2S

Hydrograph type	= Reach	Peak discharge	= 3.072 cfs
Storm frequency	= 100 yrs	Time to peak	= 728 min
Time interval	= 2 min	Hyd. volume	= 10,629 cuft
Inflow hyd. No.	= 53 - QD2S	Section type	= Circular
Reach length	= 28.7 ft	Channel slope	= 0.5 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.509	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.6381

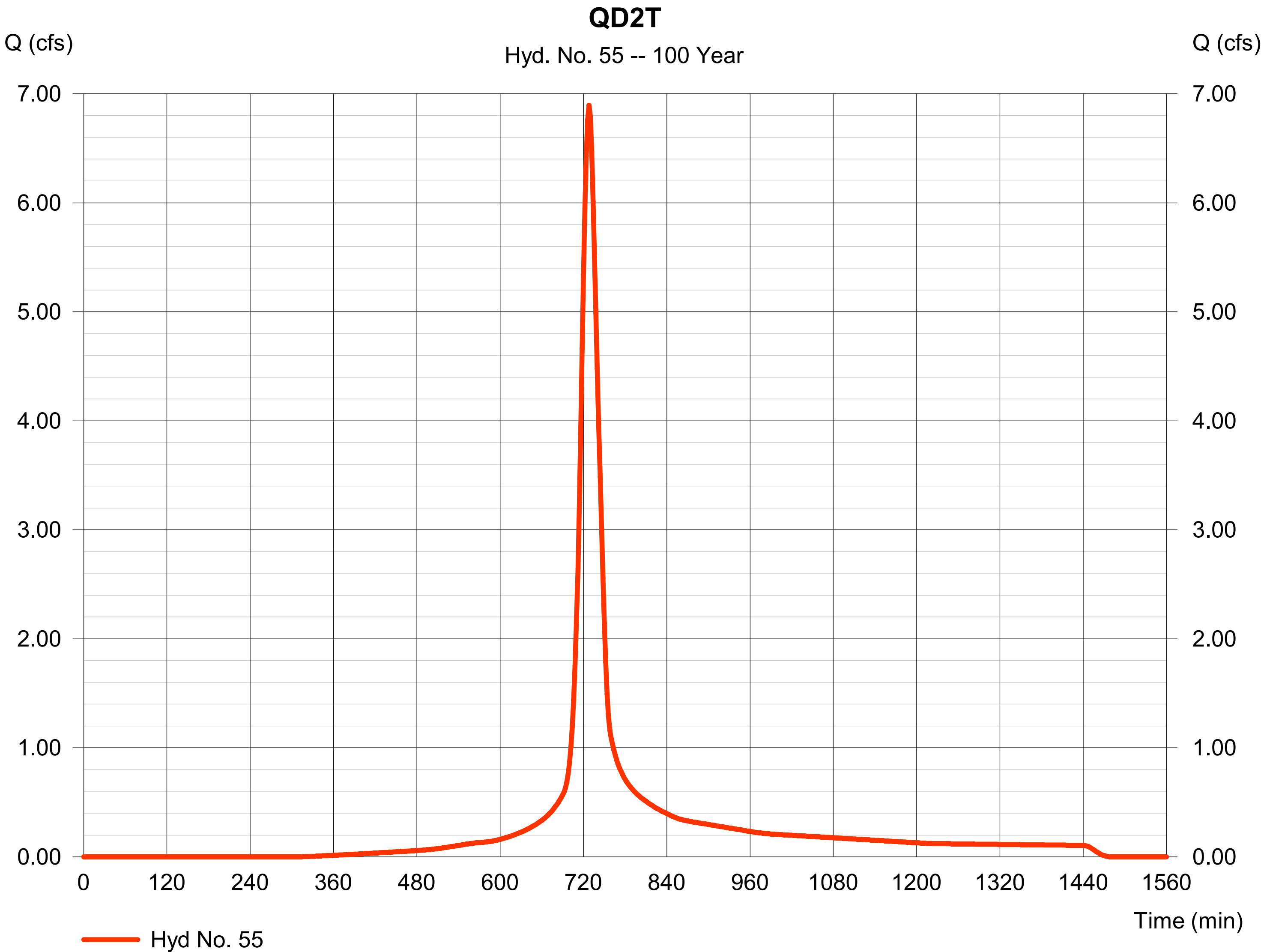
Modified Att-Kin routing method used.



Hyd. No. 55

QD2T

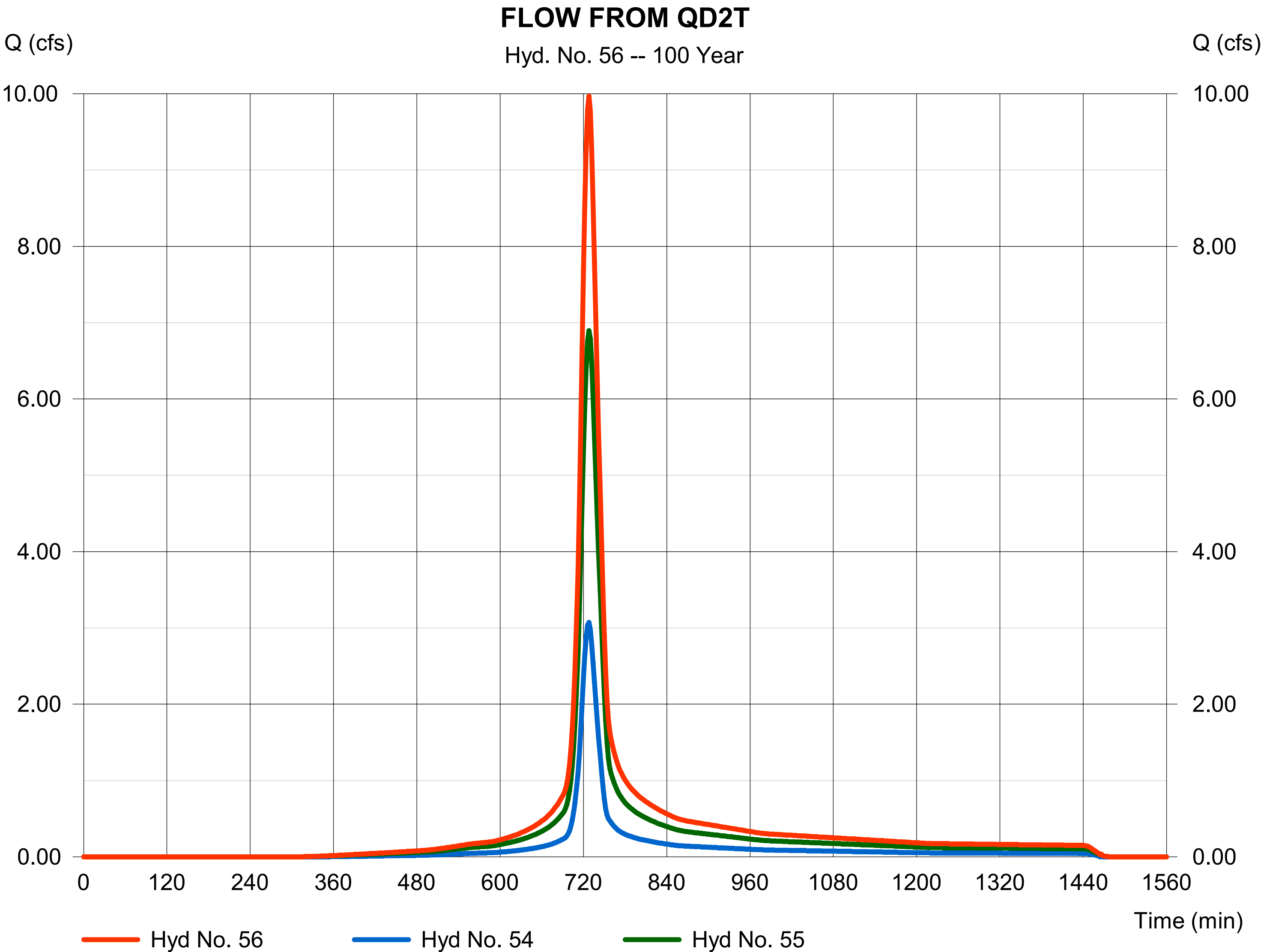
Hydrograph type	=	SCS Runoff	Peak discharge	=	6.894 cfs
Storm frequency	=	100 yrs	Time to peak	=	728 min
Time interval	=	2 min	Hyd. volume	=	25,572 cuft
Drainage area	=	3.130 ac	Curve number	=	91
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	24.20 min
Total precip.	=	3.23 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hyd. No. 56

FLOW FROM QD2T

Hydrograph type	= Combine	Peak discharge	= 9.967 cfs
Storm frequency	= 100 yrs	Time to peak	= 728 min
Time interval	= 2 min	Hyd. volume	= 36,202 cuft
Inflow hyds.	= 54, 55	Contrib. drain. area	= 3.130 ac

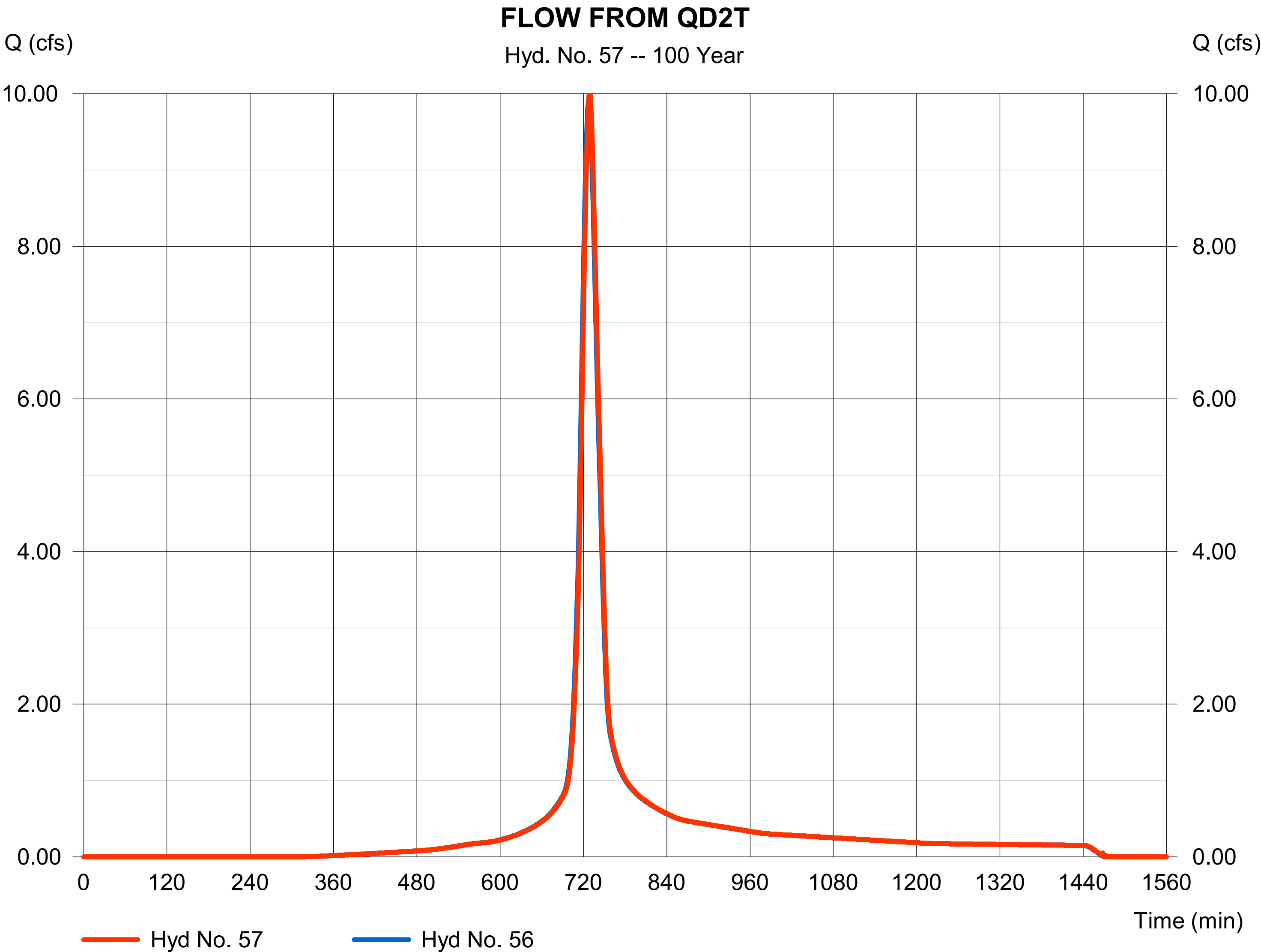


Hyd. No. 57

FLOW FROM QD2T

Hydrograph type	= Reach	Peak discharge	= 9.977 cfs
Storm frequency	= 100 yrs	Time to peak	= 730 min
Time interval	= 2 min	Hyd. volume	= 36,202 cuft
Inflow hyd. No.	= 56 - FLOW FROM QD2T	Section type	= Circular
Reach length	= 55.6 ft	Channel slope	= 0.5 %
Manning's n	= 0.024	Bottom width	= 1.5 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.509	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.7675

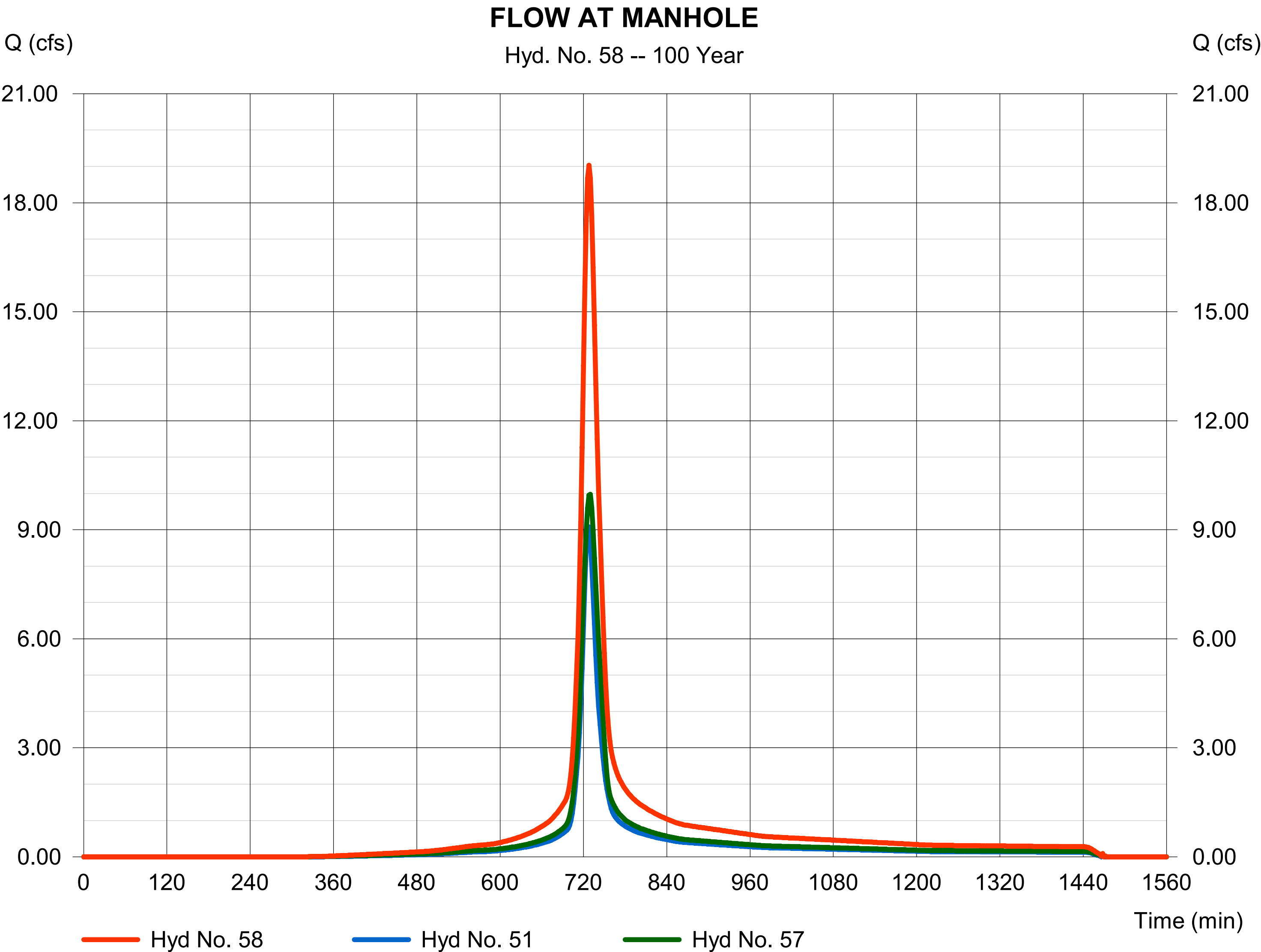
Modified Att-Kin routing method used.



Hyd. No. 58

FLOW AT MANHOLE

Hydrograph type	= Combine	Peak discharge	= 19.03 cfs
Storm frequency	= 100 yrs	Time to peak	= 728 min
Time interval	= 2 min	Hyd. volume	= 66,169 cuft
Inflow hyds.	= 51, 57	Contrib. drain. area	= 0.000 ac

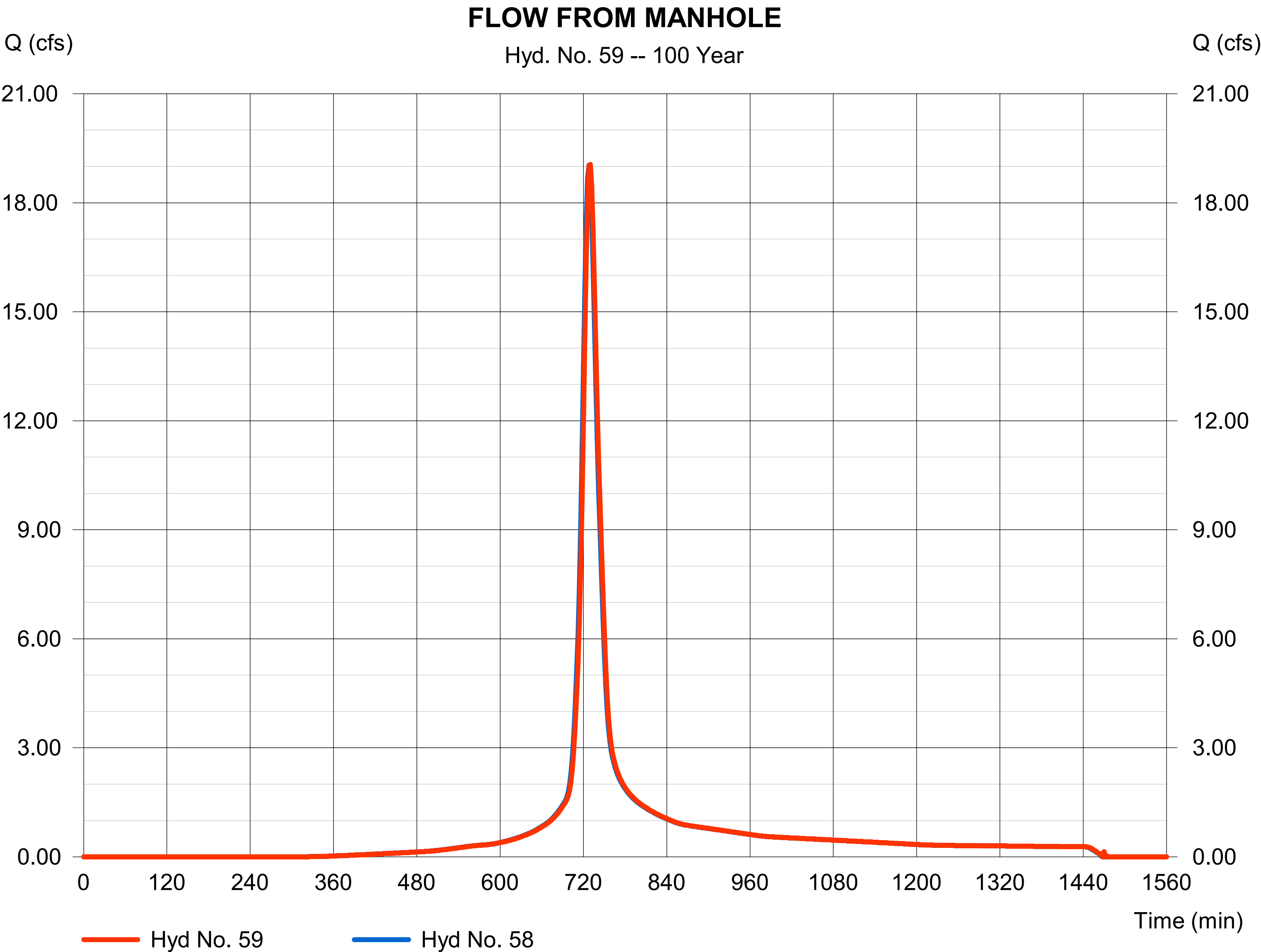


Hyd. No. 59

FLOW FROM MANHOLE

Hydrograph type	= Reach	Peak discharge	= 19.05 cfs
Storm frequency	= 100 yrs	Time to peak	= 730 min
Time interval	= 2 min	Hyd. volume	= 66,177 cuft
Inflow hyd. No.	= 58 - FLOW AT MANHOLE	Section type	= Circular
Reach length	= 141.5 ft	Channel slope	= 0.3 %
Manning's n	= 0.024	Bottom width	= 2.0 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.060	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.5250

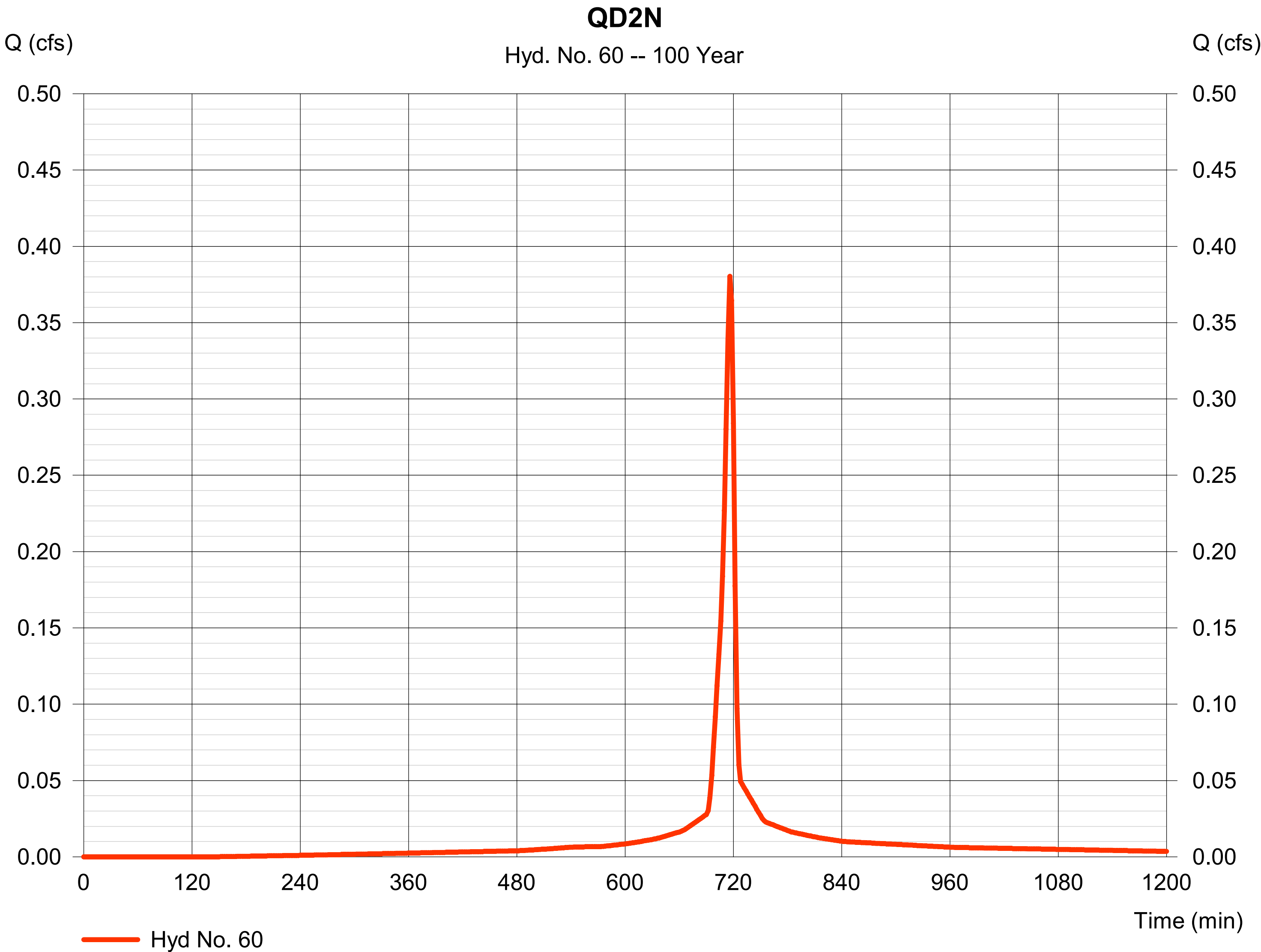
Modified Att-Kin routing method used.



Hyd. No. 60

QD2N

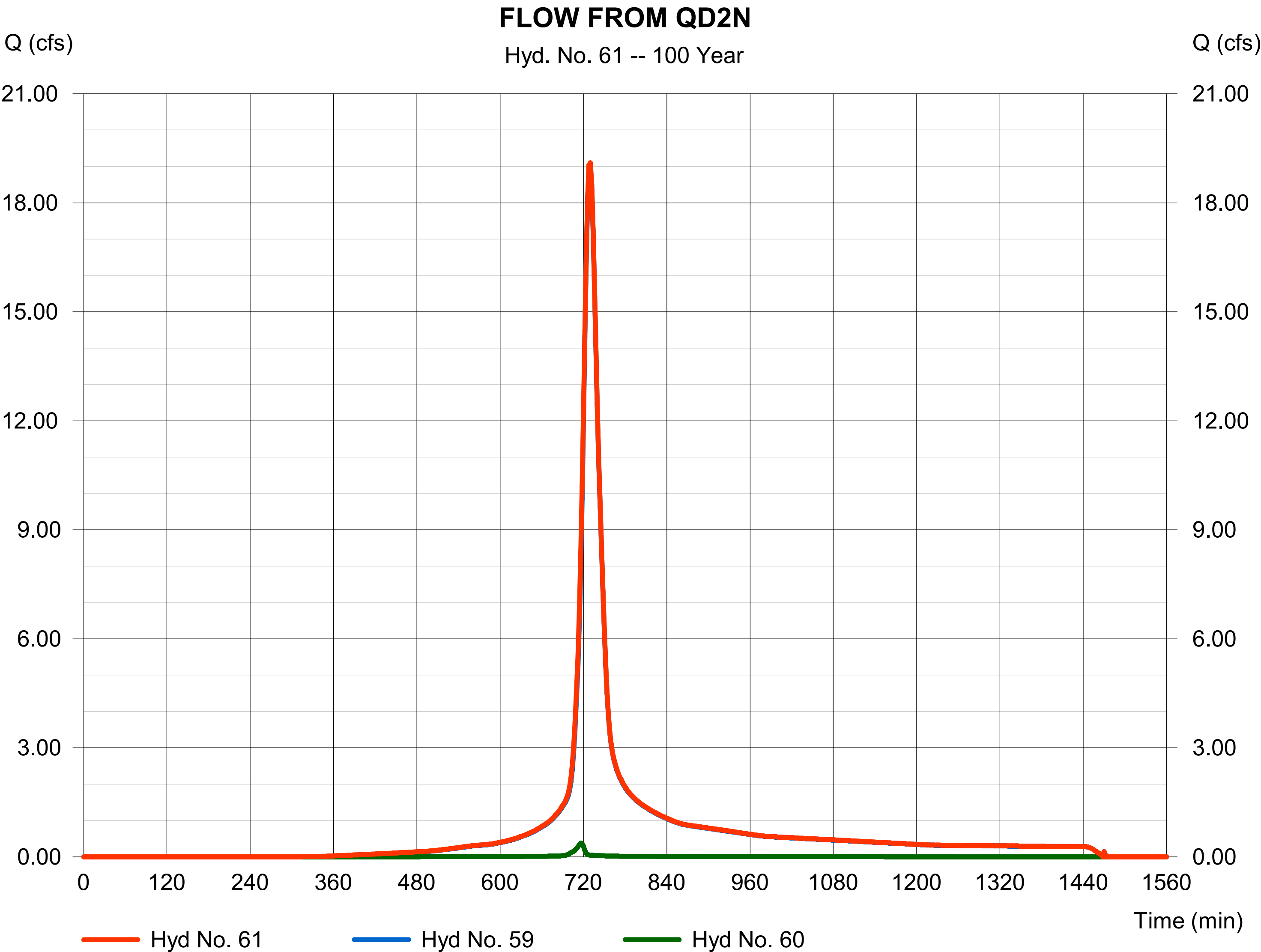
Hydrograph type	=	SCS Runoff	Peak discharge	=	0.380 cfs
Storm frequency	=	100 yrs	Time to peak	=	716 min
Time interval	=	2 min	Hyd. volume	=	851 cuft
Drainage area	=	0.090 ac	Curve number	=	96
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	6.00 min
Total precip.	=	3.23 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hyd. No. 61

FLOW FROM QD2N

Hydrograph type	= Combine	Peak discharge	= 19.10 cfs
Storm frequency	= 100 yrs	Time to peak	= 730 min
Time interval	= 2 min	Hyd. volume	= 67,028 cuft
Inflow hyds.	= 59, 60	Contrib. drain. area	= 0.090 ac

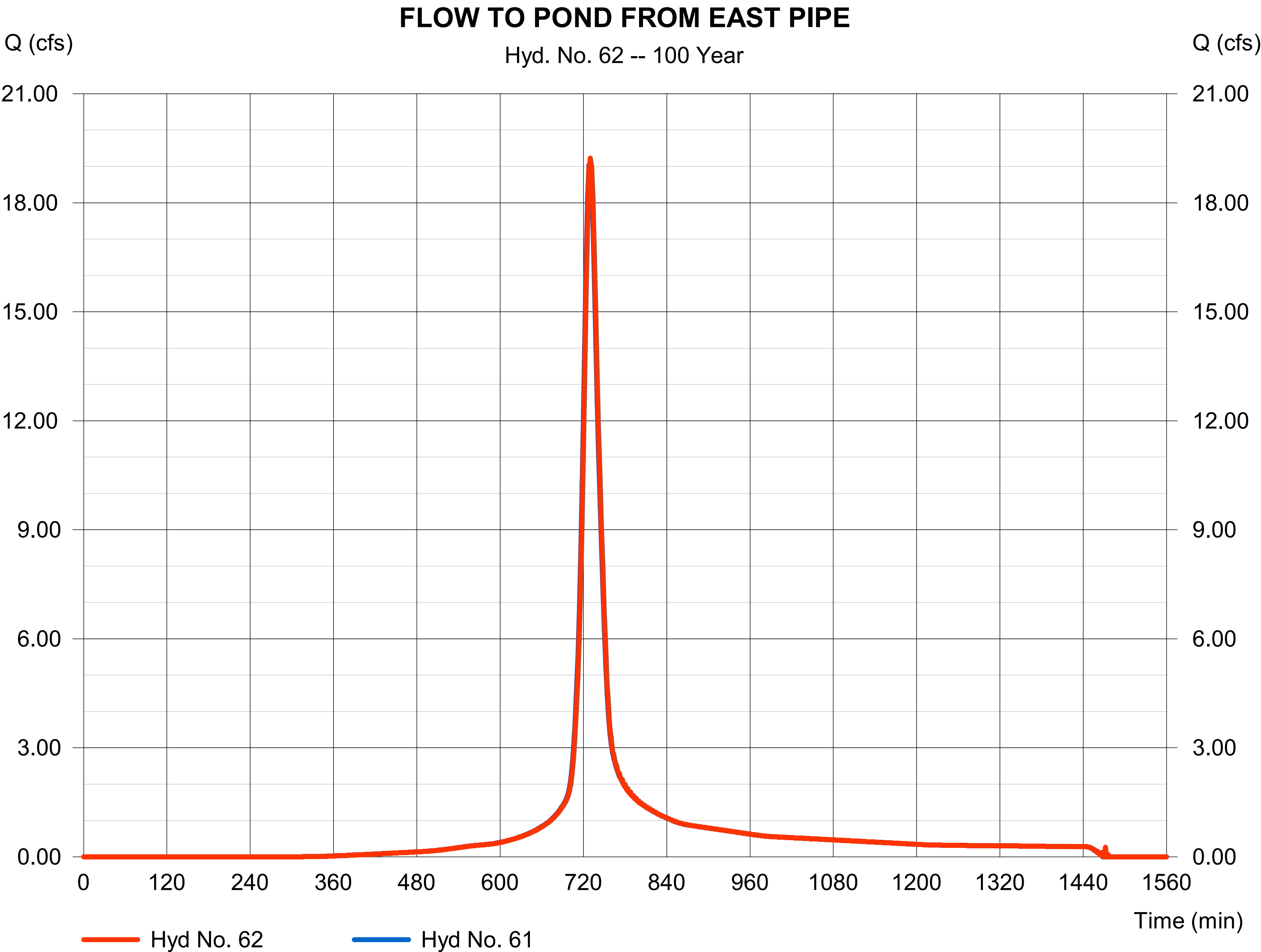


Hyd. No. 62

FLOW TO POND FROM EAST PIPE

Hydrograph type	= Reach	Peak discharge	= 19.23 cfs
Storm frequency	= 100 yrs	Time to peak	= 730 min
Time interval	= 2 min	Hyd. volume	= 67,051 cuft
Inflow hyd. No.	= 61 - FLOW FROM QD2N	Section type	= Circular
Reach length	= 23.1 ft	Channel slope	= 0.3 %
Manning's n	= 0.024	Bottom width	= 2.0 ft
Side slope	= 0.0:1	Max. depth	= 0.0 ft
Rating curve x	= 2.060	Rating curve m	= 1.250
Ave. velocity	= 0.00 ft/s	Routing coeff.	= 1.9036

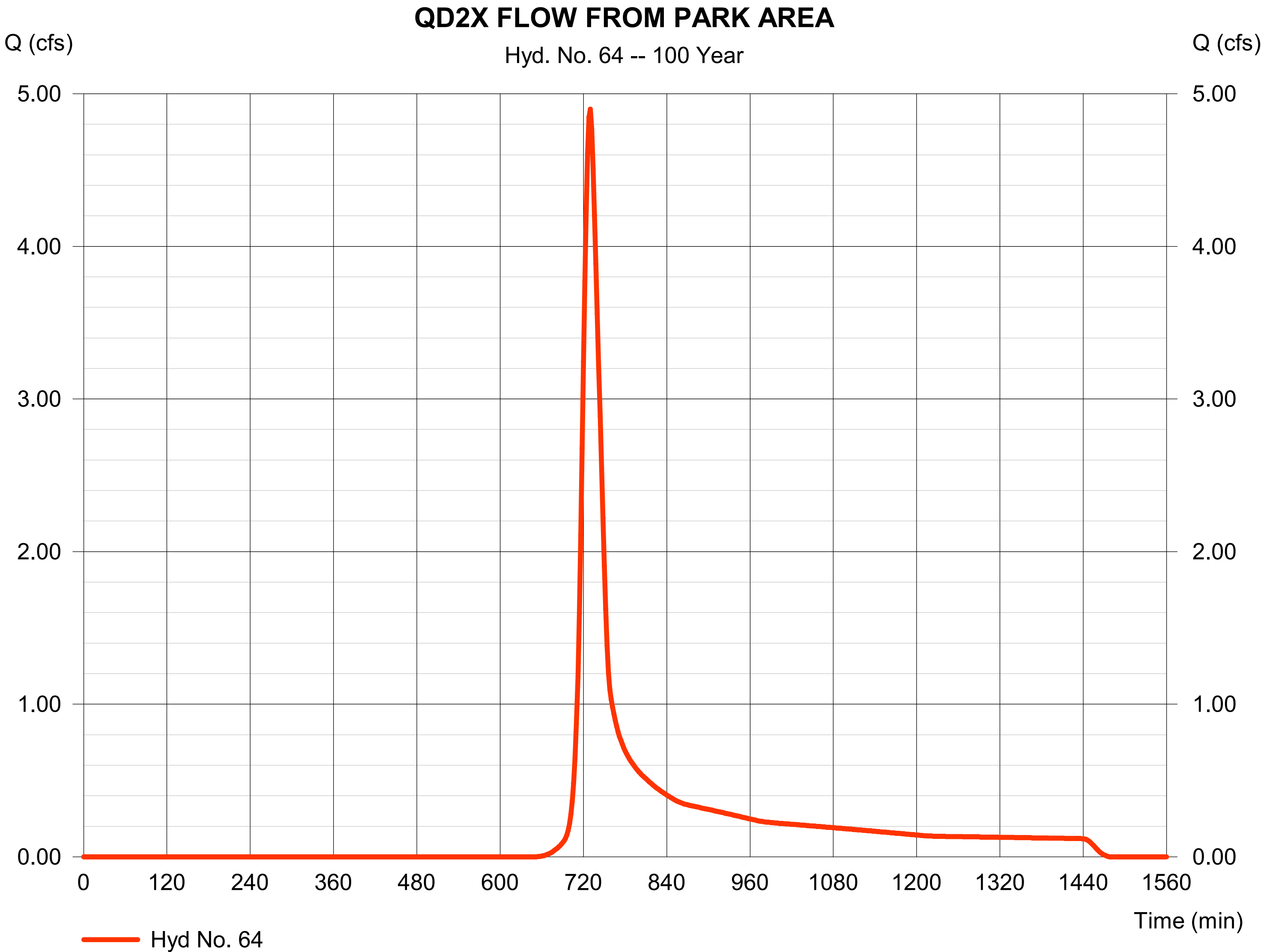
Modified Att-Kin routing method used.



Hyd. No. 64

QD2X FLOW FROM PARK AREA

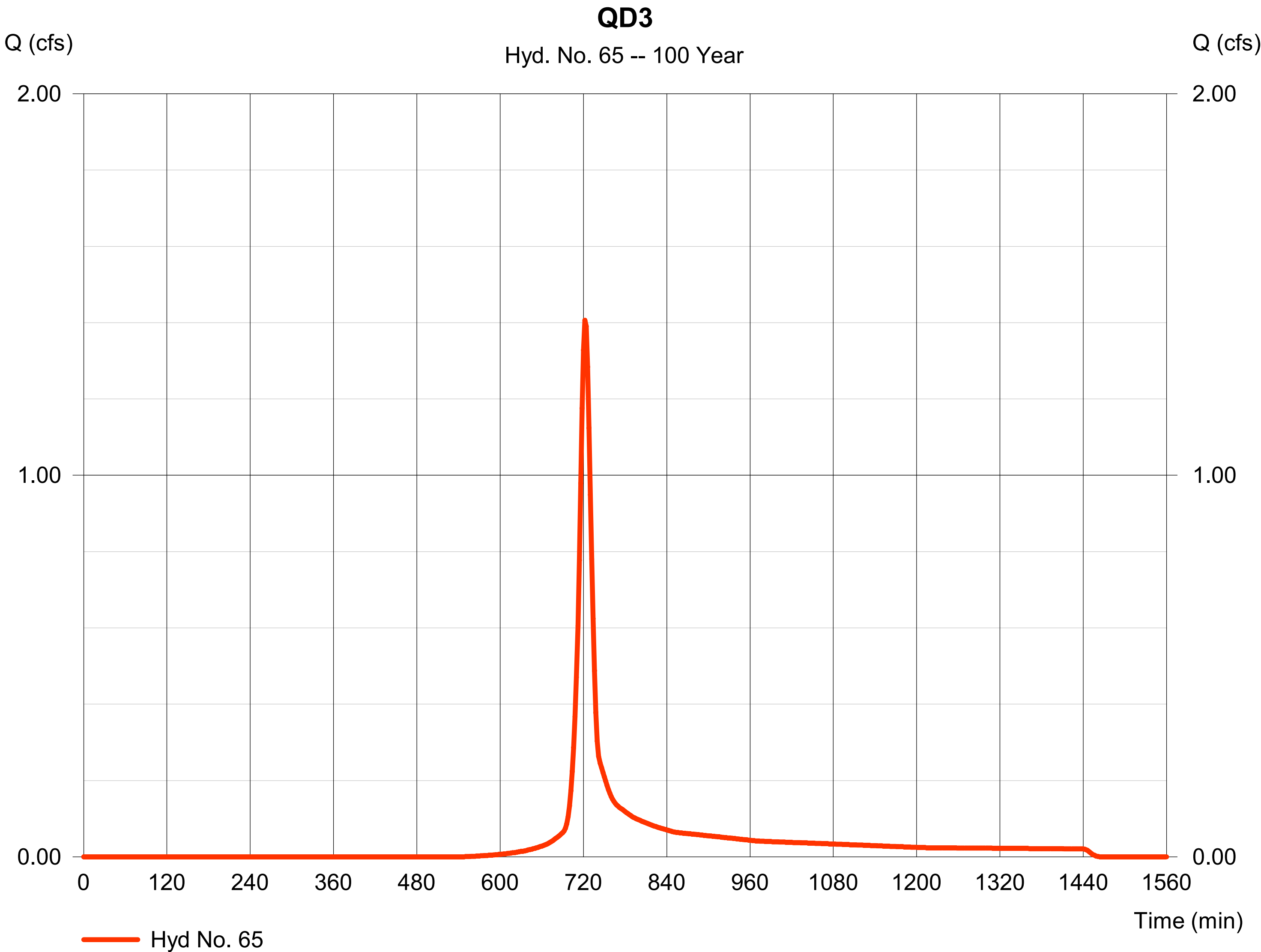
Hydrograph type	= SCS Runoff	Peak discharge	= 4.900 cfs
Storm frequency	= 100 yrs	Time to peak	= 730 min
Time interval	= 2 min	Hyd. volume	= 19,080 cuft
Drainage area	= 5.050 ac	Curve number	= 74
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 25.00 min
Total precip.	= 3.23 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hyd. No. 65

QD3

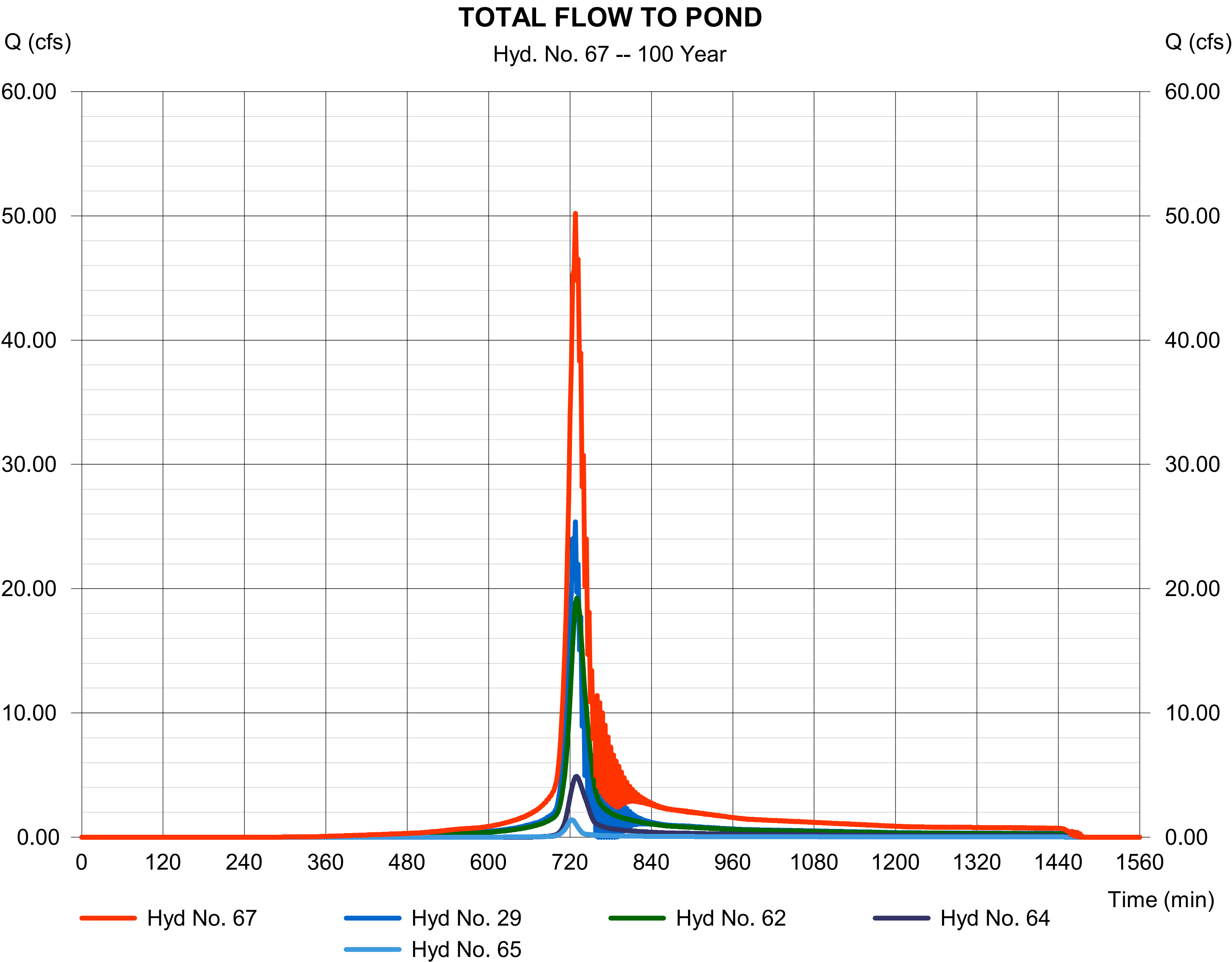
Hydrograph type	=	SCS Runoff	Peak discharge	=	1.407 cfs
Storm frequency	=	100 yrs	Time to peak	=	722 min
Time interval	=	2 min	Hyd. volume	=	3,962 cuft
Drainage area	=	0.750 ac	Curve number	=	81
Basin Slope	=	0.0 %	Hydraulic length	=	0 ft
Tc method	=	User	Time of conc. (Tc)	=	14.30 min
Total precip.	=	3.23 in	Distribution	=	Type II
Storm duration	=	24 hrs	Shape factor	=	484



Hyd. No. 67

TOTAL FLOW TO POND

Hydrograph type	= Combine	Peak discharge	= 50.18 cfs
Storm frequency	= 100 yrs	Time to peak	= 728 min
Time interval	= 2 min	Hyd. volume	= 164,786 cuft
Inflow hyds.	= 29, 62, 64, 65	Contrib. drain. area	= 5.800 ac

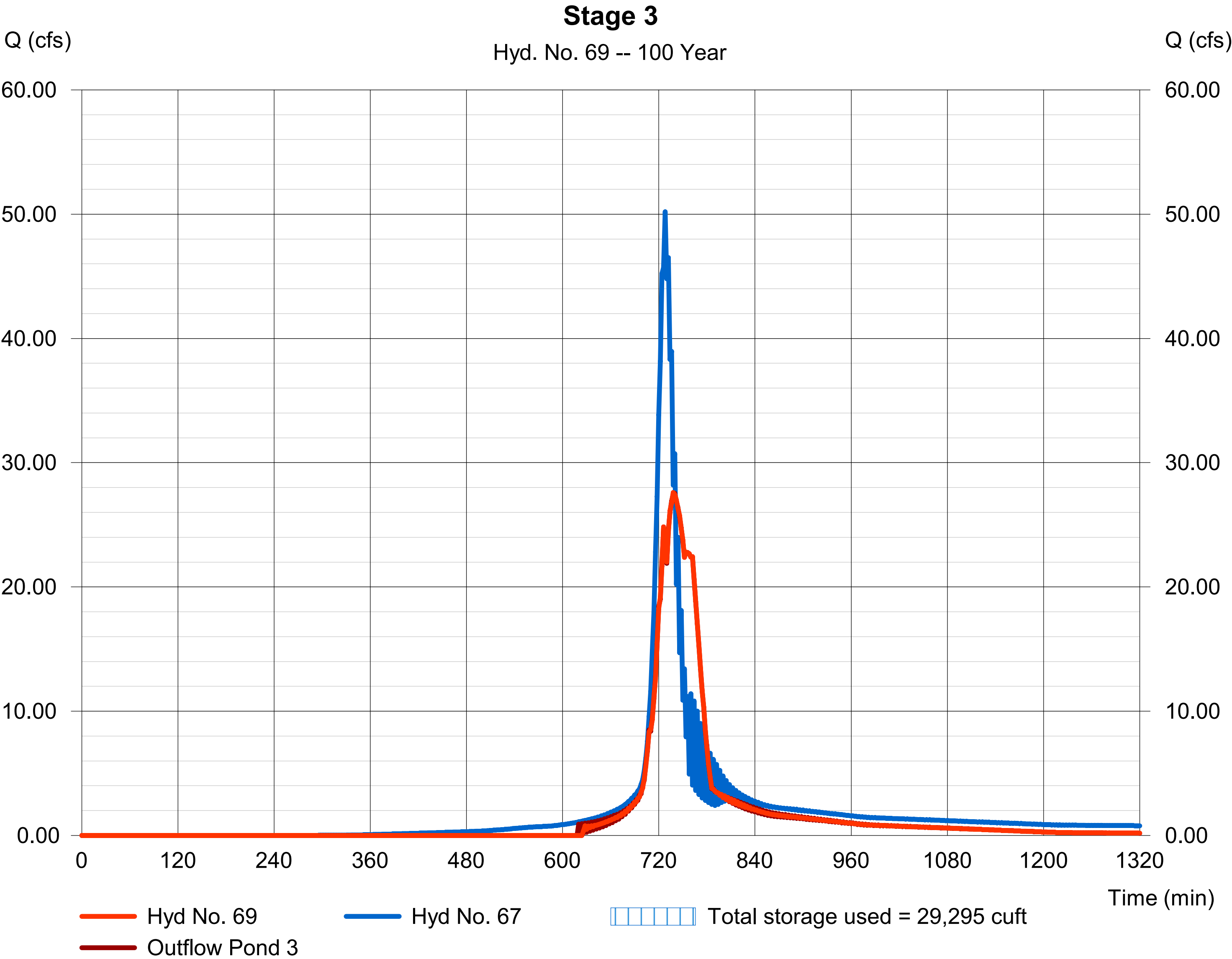


Hyd. No. 69

Stage 3

Hydrograph type	= Reservoir (Interconnected)	Peak discharge	= 27.60 cfs
Storm frequency	= 100 yrs	Time to peak	= 738 min
Time interval	= 2 min	Hyd. volume	= 126,853 cuft
Upper Pond	= Stage 3	Lower Pond	= outfall
Inflow hyd.	= 67 - TOTAL FLOW TO POND	Other Inflow hyd.	= None
Max. Elevation	= 6476.30 ft	Max. Elevation	= 6475.09 ft
Max. Storage	= 29,004 cuft	Max. Storage	= 292 cuft

Interconnected Pond Routing. Storage Indication method used.



Return Period (Yrs)	Intensity-Duration-Frequency Equation Coefficients (FHA)			
	B	D	E	(N/A)
1	0.0000	0.0000	0.0000	-----
2	69.8703	13.1000	0.8658	-----
3	0.0000	0.0000	0.0000	-----
5	79.2597	14.6000	0.8369	-----
10	88.2351	15.5000	0.8279	-----
25	102.6072	16.5000	0.8217	-----
50	114.8193	17.2000	0.8199	-----
100	127.1596	17.8000	0.8186	-----

File name: SampleFHA.idf

Intensity = B / (Tc + D)^E

Return Period (Yrs)	Intensity Values (in/hr)											
	5 min	10	15	20	25	30	35	40	45	50	55	60
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	5.69	4.61	3.89	3.38	2.99	2.69	2.44	2.24	2.07	1.93	1.81	1.70
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	6.57	5.43	4.65	4.08	3.65	3.30	3.02	2.79	2.59	2.42	2.27	2.15
10	7.24	6.04	5.21	4.59	4.12	3.74	3.43	3.17	2.95	2.77	2.60	2.46
25	8.25	6.95	6.03	5.34	4.80	4.38	4.02	3.73	3.48	3.26	3.07	2.91
50	9.04	7.65	6.66	5.92	5.34	4.87	4.49	4.16	3.88	3.65	3.44	3.25
100	9.83	8.36	7.30	6.50	5.87	5.36	4.94	4.59	4.29	4.03	3.80	3.60

Tc = time in minutes. Values may exceed 60.

Precip. file name: Sample.pcp

Storm Distribution	Rainfall Precipitation Table (in)							
	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr
SCS 24-hour	0.00	1.52	0.00	3.30	4.33	2.60	2.91	3.23
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-1st	0.00	0.00	0.00	2.75	0.00	0.00	0.00	0.00
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Custom	0.00	0.00	0.00	2.80	0.00	0.00	0.00	0.00

GEOTECHNICAL EVALUATION REPORT

LOS PRADOS DEVELOPMENT

3600 & 3740 South Meadows Road
Santa Fe, New Mexico
WT Job No. 32-224032-0

PREPARED FOR:

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February 29, 2024



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GEOTECHNICAL

ENVIRONMENTAL

INSPECTIONS

NDT

MATERIALS

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**GEOTECHNICAL EVALUATION
LOS PRADOS DEVELOPMENT
3600 & 3740 SOUTH MEADOWS ROAD
SANTA FE, NEW MEXICO
JOB NO. 32-224032-0**

1.0 PURPOSE

This report contains the results of our geotechnical evaluation for a proposed residential subdivision to be located at the above-referenced address in Santa Fe, New Mexico. The purpose of these services is to provide information and recommendations regarding:

- Subsurface conditions
- Foundation design parameters
- Lateral earth pressures
- Earthwork guidelines
- Drainage
- Percolation Rate
- Groundwater
- Corrosivity (soil to concrete)
- Seismic considerations
- Excavation conditions
- Pavement

Results of the field exploration, field tests, and laboratory testing program are presented in the Appendices.

2.0 PROJECT DESCRIPTION

Based on the information provided by Jaime Jaramillo, the proposed project will consist of the development of approximately a 17-acre site as a single-family residential subdivision. The proposed homes will be one- to two-story, slab-on-grade structure using wood concrete construction. We understand that the proposed development will include condominiums, retaining walls and a site-specific percolation rate evaluation. Maximum wall and column loads will not exceed 3 klf and 75 kips, respectively. We anticipate no extraordinary slab-on-grade criteria, and that ground floor level will be within a few feet of existing site grade. Residential paved areas for parking and driveways will be constructed. Any off-site improvements have not been included as part of this evaluation. Should any of our information or assumptions not be correct, we should be notified.

3.0 SCOPE OF SERVICES

3.1 Field Exploration

Eleven (11) borings were drilled to depths ranging from 5 to 21.5 feet below existing site grade in the proposed building areas. In addition, fourteen (14) borings were drilled to depths of about 5 feet in the proposed pavement areas. Also, five (5) percolation tests were performed within the proposed drainage basins. The borings were at the approximate locations shown on the attached Boring Location Diagram. The boring locations shown in Boring Location Diagram and elevations shown in the boring logs are based on Google Earth Pro (accessed January 19, 2024) and should be considered accurate only to the degree permitted by our data sources and implied by our measuring methods.

A field log was prepared for each boring. These logs contain visual classifications of the materials encountered during drilling as well as interpolation of the subsurface conditions between samples. Final logs, included in Appendix A, represent our interpretation of the field logs and may include modifications based on laboratory observations and tests of the field samples. The final logs describe the materials encountered, their thickness, and the locations where samples were obtained.

The Unified Soil Classification System was used to classify soils. The soil classification symbols appear on the boring logs and are briefly described in Appendix A. Local and regional geologic characteristics were used to estimate the seismic design criteria.

3.2 Laboratory Analyses

Laboratory analyses were performed on representative soil samples to aid in material classification and to estimate pertinent engineering properties of the on-site soils for preparation of this report. Testing was performed in general accordance with applicable standard test methods. The following tests were performed and the results are presented in Appendix B.

- Field moisture content
- In-situ soil density
- Expansion Potential
- Compression
- Gradation
- Liquid Limit and Plasticity Index
- Water Soluble Sulfate Content
- Water Soluble Chloride Content

3.3 Analyses and Report

This geotechnical engineering report includes a description of the project, a discussion of the field and laboratory testing programs, a discussion of the subsurface conditions, and design recommendations as appropriate to its purpose. The scope of services for this project does not include, either specifically or by implication, any environmental assessment of the site, discovery of underground storage tanks or other underground structures, or identification of contaminated or hazardous materials or conditions. If there is concern about the potential for such contamination, other studies should be undertaken. We are available to discuss the scope of such studies with you.

4.0 SITE CONDITIONS

4.1 Surface

At the time of our exploration, the site was essentially undeveloped. The ground surface was sloping down towards the northeast and contained a moderate to dense growth of grasses, shrubs, and trees. Site drainage trended to the northeast as sheet surface flow. The site is bound by South Meadows Road to the east, Morning Drive to the west and a residential home to the south. Photographs of the site at the time of our exploration are provided below.





4.2 Subsurface

As presented on the Boring Logs, surface soils to depths of 10 feet consisted of loose to very dense Silty SAND, Clayey SAND, Poorly-graded GRAVEL with Clay, Clayey GRAVEL, Silty Clayey SAND and firm to very stiff Lean CLAY. Near surface soils are of nil to high plasticity. The material underlying the surface soils and extending to the full depth of exploration consisted of loose to very dense Poorly-graded SAND, Silty SAND, Clayey SAND, Poorly-graded GRAVEL with Clay, Clayey GRAVEL and Silty Clayey SAND and stiff to hard Lean CLAY. Groundwater was not encountered in any boring at the time of exploration. A detailed description of the soils encountered can be found on the boring logs in Appendix A.

5.0 GEOTECHNICAL PROPERTIES & ANALYSIS

5.1 Laboratory Tests

Laboratory test results (see Appendix B) indicate that on-site subsoils near shallow foundation level exhibit low to moderate compressibility at existing water contents. Moderately high to very high levels of additional compression occurs when the water content is increased.

Near-surface soils are of nil to high plasticity. These soils exhibit low expansion potential when recompacted, confined by loads approximating floor loads and saturated. Slabs-on-grade supported on recompacted on-site soils have a low to medium potential for heaving if the water content of the soil increases.

Chemical tests were performed on representative samples of on-site soils to determine the amount of water-soluble sulfates and chlorides. The test results indicate that the soils classify as negligibly corrosive to concrete according to Table 19.3.1.1 of ACI 318-19.

5.2 Field Tests

The boring logs included in this report are indicators of subsurface conditions only at the specific location and date noted. Variations from the field conditions represented by the borings may become evident during construction. If variations appear, we should be contacted to re-evaluate our recommendations.

6.0 RECOMMENDATIONS

6.1 General

Recommendations contained in this report are based on our understanding of the project criteria described in Section 2.0 and the assumption that the soil and subsurface conditions are those disclosed by the explorations. Others may change the plans, final elevations, number and type of structures, foundation loads, and floor levels during design or construction. Substantially different subsurface conditions from those described herein may be encountered or become known. Any changes in the project criteria or subsurface conditions shall be brought to our attention in writing. This report does not encompass the

effects, if any, of underlying geologic hazards or regional groundwater withdrawal and expresses no opinion regarding their effects on surface movements at the project site.

6.2 Design Considerations

The borings indicate the presence of clay soils on the site. The clay soils may expand or swell with an increase in moisture content. Slabs-on-grade and related improvements situated on expansive clay soils could be subject to relatively large movements if the foundation soils experience an increase or decrease in moisture content. In addition, densification of the soil by the passage of construction equipment may increase the expansion potential of the on-site clayey soil. As expansive soils are encountered during earthwork operations, selective placement procedures should be implemented. Moderately to highly expansive soils should not be used as fill in the slab-on-grade areas within 24 inches of the final subgrade. It should be understood that if moisture penetrates expansive soils, there could be some heave and resultant cracking/distress of the proposed structures and related improvements. Conversely, as expansive soils dry, shrinkage and resultant cracking/distress of the proposed structures and related improvements may occur.

6.3 Building Foundations

Conventional spread-type foundation can be used to support the proposed structures. Since the on-site soils exhibit significant collapse potentials within the upper 5 feet, the foundations should bear on engineered fills achieved by removal and recompaction of the soils below the foundations. The depth and lateral extent of the engineered fills are presented in the **EARTHWORK** section of this report. Footings should bear a minimum of 24 inches below finished grade, which is the lowest adjacent grade for perimeter footings and floor level for interior footings. Recommended minimum widths of column, wood-frame and/or masonry wall footings are 24 and 16 inches, respectively.

Alternative footing depths and allowable bearing capacities are presented in the table below:

Footing Depth Below Finished Grade ⁴ (ft)	Allowable Bearing Capacity ⁵ (psf)
2.0 ⁶	2,500
3.0	3,000

We anticipate that total and differential settlement of the proposed structures, supported as recommended, should be less than 1 inch and ½ inch, respectively. Additional foundation movements could occur if water from any source infiltrates the foundation soils. Therefore, proper drainage should be provided in the final design and during construction.

The uplift resistance of spread footings supporting the structures may be calculated using the cone method. The equation for determining the ultimate uplift capacity as a function of footing width, footing depth, and soil weight is presented below:

$$T = \gamma D^2 \left(0.6W + 0.6L + 0.4D + \frac{WL}{D} \right)$$

Where:

- T = ultimate uplift capacity (lbs.)
- D = depth of footing below final grade (ft.)
- L = length of footing (ft.)
- W = width of footing (ft.)
- γ = unit weight of soil (pcf)*

*a unit weight of 100 pcf is recommended for the soils at this site

The design uplift resistance should be calculated by dividing the ultimate uplift capacity obtained from the equation above by a factor of safety. A factor of safety of at least 1.5 is recommended for live uplift loads.

All footings, stem walls and masonry walls should be reinforced to reduce the potential for distress caused by differential foundation movements. The use of joints at openings or other discontinuities in masonry walls is recommended.

⁴ Finished grade is the lowest adjacent grade for perimeter footings and floor level for interior footings.

⁵ Allowable bearing capacities assume fulfillment of **EARTHWORK** recommendations. Pounds per square foot (psf). Allowable Bearing Capacities also assume a minimum factor of safety equal to 3.

⁶ Minimum depth for frost protection of exterior footings or footings in unheated spaces.

We recommend that the geotechnical engineer or his representative observe the footing excavations before reinforcing steel and concrete are placed. This observation is to evaluate whether the soils exposed are similar to those anticipated for support of the footings. Any soft, loose, or unacceptable soils should be undercut to suitable materials and backfilled with approved fill materials or lean concrete. Soil backfill should be properly compacted.

6.4 **Lateral Design Criteria**

The allowable bearing capacities presented in Section 6.3 may should be used in proportioning the footings for the retaining walls. Lateral loads may be resisted by concrete interface friction and by passive resistance. For shallow foundations bearing on properly compacted fill at this site, we recommend the following lateral resistance criteria:

- Passive:
Shallow wall footings.....250 psf/ft
Shallow column footings400 psf/ft
- Coefficient of base friction (passive).....0.30

Earth retaining structures less than 10 feet in height, above any free water surface, with level backfill and no surcharge loads may be designed using the equivalent fluid pressure method. Recommended active equivalent fluid pressures and coefficients of base friction for unrestrained elements are:

- Active:
Undisturbed subsoil.....40 psf/ft
Compacted granular backfill30 psf/ft
Compacted site soils35 psf/ft
On-site clayey soilsnot recommended for use
- Coefficient of base friction (active).....0.40

Where the design includes restrained elements, the following equivalent fluid pressures are recommended:

- At-rest:
Undisturbed subsoil.....60 psf/ft
Compacted granular backfill55 psf/ft

The equivalent fluid pressures presented herein do not include the lateral pressures arising from the presence of:

- hydrostatic conditions, submergence or partial submergence
- sloping backfill, positively or negatively
- surcharge loading, permanent or temporary
- seismic or dynamic conditions

We recommend a free-draining soil layer or manufactured geosynthetic material be constructed adjacent to the back of any retaining walls. A filter may be required between the soil backfill and drainage layer. This drainage zone should help prevent development of hydrostatic pressure on the wall. This vertical drainage zone should be tied into a gravity drainage system at the base of the wall. It is important that all backfill be properly placed and compacted. Backfill should be mechanically compacted in layers. Flooding or jetting should not be permitted. Care should be taken not to damage the walls when placing the backfill. Backfills should be observed and tested during placement.

Fill against footings, stem walls, and retaining walls should be compacted to densities specified in **EARTHWORK**. Compaction of each lift adjacent to walls should be accomplished with hand-operated tampers or other lightweight compactors. Over-compaction may cause excessive lateral earth pressures that could result in wall movements.

6.5 Seismic Considerations

Structures should be designed in accordance with applicable building codes. The seismic design parameters presented in the following table, in accordance with the 2021 International Building Code/ASCE 7-16 are applicable to the project site:

Seismic Design Parameters 2021 IBC/ASCE 7-16	
Soil Site Class	D
Mapped Spectral Response Acceleration at 0.2 sec period (S_s)	0.411g
Mapped Spectral Response Acceleration at 1.0 sec period (S_1)	0.136g
Site Coefficient for 0.2 sec period (F_a)	1.472
Site Coefficient for 1.0 sec period (F_v)	2.328
Design Spectral Response Acceleration at 0.2 sec period (S_{DS})	0.403g
Design Spectral Response Acceleration at 1.0 sec period (S_{D1})	0.211g

The soil site class is based upon conditions identified in shallow explorations and local knowledge of the soil conditions in the vicinity of the site. Soil conditions extending beyond the depth of our explorations to a depth of 100 feet were assumed for the purposes of providing the information presented in the table.

6.6 Slab-on-Grade Support

Floor slabs can be supported on properly placed and compacted fill. The slab subgrade should be prepared by the procedures outlined in this report. A minimum 4-inch layer of base course should be provided beneath all slabs to help prevent capillary rise and a damp slab. The modulus of subgrade reaction (k) is estimated to be 150 pounds per cubic inch (pci), based upon a 30-inch diameter plate.

The use of vapor retarders or barriers is desirable for any slab-on-grade where the floor will be covered by products using water-based adhesives, wood, vinyl backed carpet, impermeable floor coatings (urethane, epoxy, acrylic terrazzo, etc.) or where the floor will be in contact with moisture sensitive equipment or product. When used, the design and installation should be in accordance with the recommendations given in ACI 302.1R and 302.2R. Final determination on the use of a vapor retarder should be left to the slab designer.

All concrete placement and curing operations should follow the American Concrete Institute manual recommendations. Improper curing techniques and/or high slump (high water-cement ratio) could cause excessive shrinkage, cracking or curling. Concrete slabs should be allowed to cure adequately before placing vinyl or other moisture sensitive floor covering.

6.7 Drainage

The major cause of soil problems in this vicinity is moisture increase in soils below structures. Therefore, it is extremely important that positive drainage be provided during construction and maintained throughout the life of the proposed structures. Infiltration of water into utility or foundation excavations must be prevented during construction. Planters or other surface features that could retain water adjacent to the proposed structures, should not be constructed. It is also important that proper planning and control of any landscape and irrigation practices be performed.

In areas where sidewalks or paving do not immediately adjoin the building, protective slopes should be provided with an outfall of 5 percent for at least 10 feet from perimeter walls. Scuppers and drainpipes should be designed to provide drainage away from the structures for a minimum of 10 feet. Backfill against footings, exterior walls, and in utility and sprinkler line trenches should be well compacted and free of all construction debris to minimize the possibility of moisture infiltration.

Water and sewer utility lines should be properly installed to avoid possible sources for subsurface saturation. It is important that all utility trenches be properly backfilled. If practicable, planters and/or landscaping should not be constructed adjacent to or near structures. If planters and/or landscaping are adjacent to or near the structures, we recommend the following:

- Planters should be sealed
- Grades should slope away from the proposed structures
- Only shallow rooted landscaping should be used
- Watering should be kept to a minimum

6.8 Corrosivity to Concrete

In order to be consistent with standard local practice and for reasons of material availability, we recommend a Type I/II portland cement be used for all concrete on and below grade.

6.9 Pavements

Based on existing subgrade conditions, the following pavement sections are recommended:

Traffic Area	Asphalt Concrete Pavement (inches)	Base Course (inches)
Residential streets	3.0	4

Base course and asphalt concrete should conform to New Mexico Department of Transportation (NMDOT) Standard Specifications for Road and Bridge Constructions. Bituminous surfacing should be constructed of dense-graded, central plant-mix, asphalt concrete of an SP-IV or SP-III mix design.

Material and compaction requirements should conform to recommendations presented under **EARTHWORK**. The gradient of paved surfaces should ensure positive drainage. Water should not pond in areas directly adjoining paved sections. The on-site subgrade soils may soften and lose stability if subjected to conditions that result in an increase in water content.

The "design life" (20 years) of a pavement is defined as the expected life at the end of which reconstruction of the pavement will need to occur. Normal maintenance, including crack sealing, slurry sealing, and/or chip sealing, should be performed during the life of the pavement.

6.9.1 Pavements on Expansive Soils

Pavement design methods are intended to provide an adequate thickness of structural materials over a particular subgrade such that wheel loads are reduced to a level the subgrade can support. The support characteristics of the subgrade for pavement design do not account for shrink and swell movements of an expansive clayey subgrade such as the soils encountered on this project. Consequently, the pavement may be adequate from a structural standpoint, yet still experience cracking and deformation due to shrink/swell movement of the subgrade. It is therefore important to minimize moisture changes in the subgrade in or to reduce shrink/swell movements. The pavement surface, subbase surface, and adjacent areas should be well drained. Excessive watering of landscaped areas adjacent to pavement should be avoided. Proper maintenance

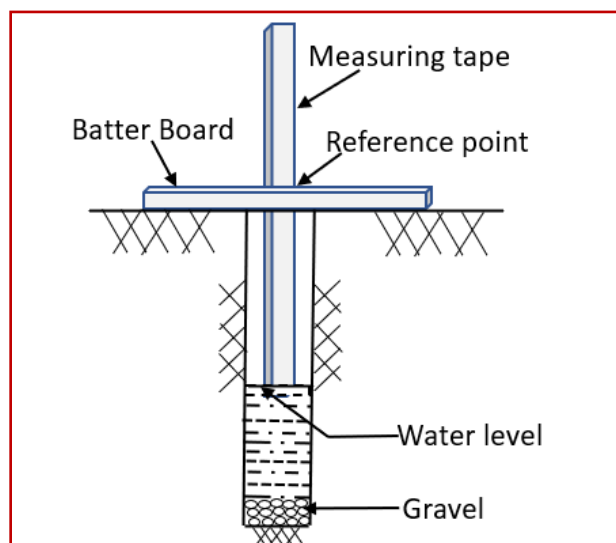
should be performed on cracks in the pavement surface to prevent water from penetrating through to the base or subbase material. Even with these precautions, some movement and related cracking may still occur, requiring periodic maintenance.

6.10 Percolation Tests

To provide information to designers regarding the anticipated performance of on-site material, five (5) percolation tests were performed. The percolation test borings were drilled to depth of 5 feet below existing grade. Each hole was then filled with water and allowed to saturate prior to testing. Water surface measurements were taken at specific times intervals from a fixed referenced point in the test hole. The percolation test schematic is presented in the figure below, the field-testing results are summarized in the table below.

Percolation Test No.	Depth below Existing Grade to Top of Surface Being Tested, (inches)	Recommended Percolation Rate for Basin Design, (minutes/inch)
P-1	60	30.0
P-2	60	16.0
P-3	60	16.0
P-4	60	13.0
P-5	60	60.0

The percolation test results are only applicable at the depth and location indicated in the report.



7.0 EARTHWORK

7.1 General

The conclusions contained in this report for the proposed construction are contingent upon compliance with recommendations presented in this section. Any excavating, trenching, or disturbance that occurs after completion of the earthwork must be backfilled, compacted and tested in accordance with the recommendations contained herein. It is not reasonable to rely upon our conclusions and recommendations if any future unobserved and untested trenching, earthwork activities or backfilling occurs.

If any unobserved and untested earthwork, trenching or backfilling occurs, then the conclusions and recommendations in this report may not be relied on. We recommend that Western Technologies Inc. be retained to provide services during these phases of the project. Observation and testing of all foundation excavations should be performed prior to placement of reinforcing steel and concrete to confirm that foundations are constructed on satisfactory bearing materials.

7.2 Site Clearing

Strip and remove any existing vegetation, debris, and any other deleterious materials from the structures areas. The structures area is defined as areas within the structures' footprint plus 5 feet beyond the perimeter of that footprint. All exposed surfaces should be free of mounds and depressions that could prevent uniform compaction.

7.3 Excavation

We anticipate that excavations for the foundations and utility trenches for the proposed construction can be accomplished with conventional equipment. Excavations penetrating the underlying auger refusal material may require the use of heavy-duty equipment to facilitate removal. The speed and ease of excavation is dependent on the nature of the deposit, the type of equipment used, and the skill and experience of the equipment operator. Note that selection of excavation equipment is the responsibility of the contractor.

The soils to be penetrated by the proposed excavations may vary significantly across the site. Our soil classifications are based solely on the materials encountered in widely spaced

exploratory test borings. The contractor should verify that similar conditions exist throughout the proposed area of excavation. If different subsurface conditions are found at the time of construction, we should be contacted immediately to evaluate the conditions encountered.

7.3.1 Temporary Excavations and Slopes

Temporary, non-surcharged construction excavations should be sloped or shored. The individual contractor should be made responsible for designing and constructing stable, temporary excavations as required to maintain stability of both the excavation sides and bottom. All excavations should be sloped or shored in the interest of safety following local and federal regulations, including current OSHA excavation and trench safety standards. OSHA recommends a maximum slope inclination of $\frac{3}{4}$:1 (horizontal: vertical) for Type A soils, 1:1 for Type B soils, and $1\frac{1}{2}$:1 for Type C soils.

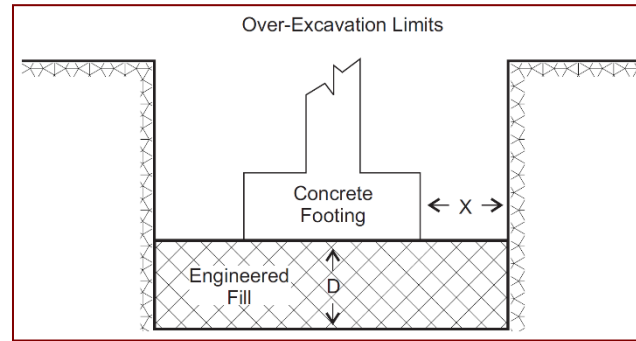
As a safety measure, it is recommended that all vehicles and soil piles be kept a minimum lateral distance back from the crest of the slope at least equal to the slope height. The exposed slope face should be protected against the elements.

If any excavation, including a utility trench, is extended to a depth of more than 20 feet, it will be necessary to have the side slopes designed by a professional engineer.

We recommend that the contractor retain a geotechnical engineer to observe the soils exposed in all excavations and provide engineering design for the slopes. This will provide an opportunity to classify the soil types encountered, and to modify the excavation slopes as necessary. This also allows the opportunity to analyze the stability of the excavation slopes during construction.

7.4 Building Footings Preparation

In footing areas, remove existing soils as required to a minimum depth of 3 feet below the bottom of the footing (depth D in the diagram below). Removal should extend a minimum of 3 feet beyond the footing edges (length X in the diagram below). Replace with engineered fill material.



7.5 Slab-on-Grade Preparation

Slabs-on-grade should be founded on engineered fill material. Remove existing soils to a minimum depth of 24 inches below the bottom of the slab, which includes any fill required to achieve final grade. Replace with properly compacted, low-expansive, fill material..

7.6 Pavement Preparation

The subgrade should be scarified, moistened as required, and recompactd for a minimum depth of 12 inches prior to placement of fill and pavement materials.

7.7 Materials

Clean imported materials or on-site soils with low expansive potentials and maximum dimension of 6 inches or imported materials may be used as fill material for the following:

- Foundation areas
- Pavement
- Backfill

Frozen soils should not be used as fill or backfill.

Imported soils should conform to the following:

- Gradation (ASTM C136): percent finer by weight
 - 6" 100
 - 4" 85-100
 - $\frac{3}{4}$ " 70-100
 - No. 4 Sieve 50-100
 - No. 200 Sieve 30 (max)
- Maximum Plasticity Index 5
- Maximum soluble sulfates (%) 0.10

Base course should conform to the New Mexico Department of Transportation (NMDOT) Standard Specifications for Road and Bridge Construction or the New Mexico Standard Specifications for Public Works Construction, whichever is applicable.

7.8 Placement and Compaction

- a. Place and compact fill in horizontal lifts, using equipment and procedures that will produce recommended water contents and densities throughout the lift.
- b. Uncompacted lift thickness should not exceed 10 inches.
- c. Materials should be compacted to the following:

Minimum Percent Material Compaction (ASTM D1557)

- On-site or imported soil, reworked and fill 95
- Base course below slabs-on-grade 95
- Aggregate base below pavement 96
- Nonstructural backfill 90

Imported soils should be compacted to within a water content range of 3 percent below to 3 percent above optimum. On-site soils used as backfill should be compacted to within a water content range of 1 percent below to 3 percent above optimum.

7.9 **Compliance**

Recommendations for foundations, slab-on-grade and pavements supported on compacted fills or prepared subgrade depend upon compliance with the **EARTHWORK** recommendations. To assess compliance, observation and testing should be performed under the direction of a WT geotechnical engineer. Please contact us to provide these observation and testing services.

8.0 **PLAN REVIEW**

Foundation and grading plans were not available at the time of this report. WT should be retained to review the final plans to determine if they are consistent with the recommendations presented in this report. If the Client does not retain WT to review the plans and specifications, WT shall have no responsibility for the suitability of the plans for project application.

9.0 **ADDITIONAL SERVICES**

The recommendations provided in this report are based on the assumption that a sufficient schedule of tests and observations will be performed during construction to verify compliance. At a minimum, these tests and observations should be comprised of the following:

- Observations and testing during site preparation and earthwork,
- Observation of foundation excavations, and
- Consultation as may be required during construction.

Retaining the geotechnical engineer who developed your report to provide construction observation is the best way to verify compliance and to help you manage the risks associated with unanticipated conditions.

10.0 **LIMITATIONS**

This report has been prepared assuming the project criteria described in **2.0 PROJECT DESCRIPTION**. If changes in the project criteria occur, or if different subsurface conditions are encountered or become known, the conclusions and recommendations presented herein shall

become invalid. In any such event, WT should be contacted in order to assess the effect that such variations may have on our conclusions and recommendations. If WT is not retained for the construction observation and testing services to determine compliance with this report, our professional responsibility is accordingly limited.

The recommendations presented are based entirely upon data derived from a limited number of samples obtained from widely spaced explorations. The attached logs are indicators of subsurface conditions only at the specific locations and times noted. This report assumes the uniformity of the geology and soil structure between explorations, however variations can and often do exist. Whenever any deviation, difference, or change is encountered or becomes known, WT should be contacted.

This report is for the exclusive benefit of our client alone. There are no intended third-party beneficiaries of our contract with the client or this report, and nothing contained in the contract or this report shall create any express or implied contractual or any other relationship with, or claim or cause of action for, any third party against WT.

This report is valid for the earlier of one year from the date of issuance, a change in circumstances, or discovered variations. After expiration, no person or entity shall rely on this report without the express written authorization of WT.

11.0 CLOSURE

We prepared this report as an aid to the designers of the proposed project. The comments, statements, recommendations and conclusions set forth in this report reflect the opinions of the authors. These opinions are based upon data obtained at the location of the explorations, and from laboratory tests. Work on your project was performed in accordance with generally accepted standards and practices utilized by professionals providing similar services in this locality. No other warranty, express or implied, is made.



Approximate Boring Location



Approximate Perc Test Location



Proposed Building Area



NOT TO SCALE



Western Technologies
An RMA Company

PROJECT:

LOS PRADOS DEVELOPMENT

JOB NO.:

32-224032-0

BORING LOCATION DIAGRAM

PLATE

1

Allowable Soil Bearing Capacity	The recommended maximum contact stress developed at the interface of the foundation element and the supporting material.
Backfill	A specified material placed and compacted in a confined area.
Base Course	A layer of specified aggregate material placed on a subgrade or subbase.
Base Course Grade	Top of base course.
Bench	A horizontal surface in a sloped deposit.
Caisson/Drilled Shaft	A concrete foundation element cast in a circular excavation which may have an enlarged base (or belled caisson).
Concrete Slabs-On-Grade	A concrete surface layer cast directly upon base course, subbase or subgrade.
Crushed Rock Base Course	A base course composed of crushed rock of a specified gradation.
Differential Settlement	Unequal settlement between or within foundation elements of a structure.
Engineered Fill	Specified soil or aggregate material placed and compacted to specified density and/or moisture conditions under observations of a representative of a soil engineer.
Existing Fill	Materials deposited through the action of man prior to exploration of the site.
Existing Grade	The ground surface at the time of field exploration.
Expansive Potential	The potential of a soil to expand (increase in volume) due to absorption of moisture.
Fill	Materials deposited by the actions of man.
Finished Grade	The final grade created as a part of the project.
Gravel Base Course	A base course composed of naturally occurring gravel with a specified gradation.
Heave	Upward movement.
Native Grade	The naturally occurring ground surface.
Native Soil	Naturally occurring on-site soil.
Rock	A natural aggregate of mineral grains connected by strong and permanent cohesive forces. Usually requires drilling, wedging, blasting or other methods of extraordinary force for excavation.
Sand and Gravel Base Course	A base course of sand and gravel of a specified gradation.
Sand Base Course	A base course composed primarily of sand of a specified gradation.
Scarify	To mechanically loosen soil or break down existing soil structure.
Settlement	Downward movement.
Soil	Any unconsolidated material composed of discrete solid particles, derived from the physical and/or chemical disintegration of vegetable or mineral matter, which can be separated by gentle mechanical means such as agitation in water.
Strip	To remove from present location.
Subbase	A layer of specified material placed to form a layer between the subgrade and base course.
Subbase Grade	Top of subbase.
Subgrade	Prepared native soil surface.

COARSE-GRAINED SOILS
LESS THAN 50% FINES

GROUP SYMBOLS	DESCRIPTION	MAJOR DIVISIONS
GW	WELL-GRADED GRAVEL OR WELL-GRADED GRAVEL WITH SAND, LESS THAN 5% FINES	GRAVELS MORE THAN HALF OF COARSE FRACTION IS LARGER THAN NO. 4 SIEVE SIZE
GP	POORLY-GRADED GRAVEL OR POORLY-GRADED GRAVEL WITH SAND, LESS THAN 5% FINES	
GM	SILTY GRAVEL OR SILTY GRAVEL WITH SAND, MORE THAN 12% FINES	
GC	CLAYEY GRAVEL OR CLAYEY GRAVEL WITH SAND, MORE THAN 12% FINES	
SW	WELL-GRADED SAND OR WELL-GRADED SAND WITH GRAVEL, LESS THAN 5% FINES	SANDS MORE THAN HALF OF COARSE FRACTION IS SMALLER THAN NO. 4 SIEVE SIZE
SP	POORLY-GRADED SAND OR POORLY-GRADED SAND WITH GRAVEL, LESS THAN 5% FINES	
SM	SILTY SAND OR SILTY SAND WITH GRAVEL, MORE THAN 12% FINES	
SC	CLAYEY SAND OR CLAYEY SAND WITH GRAVEL, MORE THAN 12% FINES	

NOTE: Coarse-grained soils receive dual symbols if they contain 5% to 12% fines (e.g., SW-SM, GP-GC).

FINE-GRAINED SOILS
MORE THAN 50% FINES

GROUP SYMBOLS	DESCRIPTION	MAJOR DIVISIONS
ML	SILT, SILT WITH SAND OR GRAVEL, SANDY SILT, OR GRAVELLY SILT	SILTS AND CLAYS LIQUID LIMIT LESS THAN 50
CL	LEAN CLAY OF LOW TO MEDIUM PLASTICITY, SANDY CLAY, OR GRAVELLY CLAY	
OL	ORGANIC SILT OR ORGANIC CLAY OF LOW TO MEDIUM PLASTICITY	
MH	ELASTIC SILT, SANDY ELASTIC SILT, OR GRAVELLY ELASTIC SILT	SILTS AND CLAYS LIQUID LIMIT MORE THAN 50
CH	FAT CLAY OF HIGH PLASTICITY, SANDY FAT CLAY, OR GRAVELLY FAT CLAY	
OH	ORGANIC SILT OR ORGANIC CLAY OF HIGH PLASTICITY	
PT	PEAT AND OTHER HIGHLY ORGANIC SOILS	HIGHLY ORGANIC SOILS

NOTE: Fine-grained soils may receive dual classification based upon plasticity characteristics (e.g. CL-ML).

SOIL SIZES

COMPONENT	SIZE RANGE
BOULDERS	Above 12 in.
COBBLES	3 in. – 12 in.
GRAVEL	No. 4 – 3 in.
Coarse	¾ in. – 3 in.
Fine	No. 4 – ¾ in.
SAND	No. 200 – No. 4
Coarse	No. 10 – No. 4
Medium	No. 40 – No. 10
Fine	No. 200 – No. 40
Fines (Silt or Clay)	Below No. 200

NOTE: Only sizes smaller than three inches are used to classify soils

CONSISTENCY

CLAYS & SILTS	BLOWS PER FOOT
VERY SOFT	0 – 2
SOFT	3 – 4
FIRM	5 – 8
STIFF	9 – 15
VERY STIFF	16 – 30
HARD	OVER 30

RELATIVE DENSITY

SANDS & GRAVELS	BLOWS PER FOOT
VERY LOOSE	0 – 4
LOOSE	5 – 10
MEDIUM DENSE	11 – 30
DENSE	31 – 50
VERY DENSE	OVER 50

NOTE: Number of blows using 140-pound hammer falling 30 inches to drive a 2-inch-OD (1½-inch ID) split-barrel sampler (ASTM D1586).

PLASTICITY OF FINE GRAINED SOILS

PLASTICITY INDEX	TERM
0	NON-PLASTIC
1 – 7	LOW
8 – 20	MEDIUM
Over 20	HIGH

DEFINITION OF WATER CONTENT

DRY
SLIGHTLY DAMP
DAMP
MOIST
WET
SATURATED



METHOD OF CLASSIFICATION

PLATE

A-2

The number shown in **"BORING NO."** or **"TEST PIT NO."** refers to the approximate location of the same number indicated on the "Boring and Test Pit Location Diagram" as positioned in the field by pacing or measurement from property lines and/or existing features, or through the use of Global Positioning System (GPS) devices. The accuracy of GPS devices is somewhat variable.

"DRILLING TYPE" refers to the exploratory equipment used in the boring wherein **HSA = hollow stem auger**, and the dimension presented is the outside diameter of the HSA used.

"EQUIPMENT TYPE" refers to the equipment used in the excavation of the test pit, and may include the width of the bucket on the excavator and the use of "rock" teeth or attachments.

"N" in "BLOW COUNTS" refers to a 2-in. outside diameter split-barrel sampler driven into the ground with a 140 lb. drop-hammer dropped 30 in. repeatedly until a penetration of 18 in. is achieved or until refusal. The number of blows, or "blow count", of the hammer is recorded for each of three 6-in. increments totaling 18 in. The number of blows required for advancing the sampler for the last 12 in. (2nd and 3rd increments) is defined as the Standard Penetration Test (SPT) "N"-Value. Refusal to penetration is considered more than 50 blows for a 6-inch increment. (Ref. ASTM D1586).

If,

N = a whole # e.g. "15", it represents the SPT blow counts for the last 12 inches.

N = stacked numbers e.g., 5/10/20, it represents the blow counts for each 6 inches increment.

"R" in "BLOW COUNTS" refers to a 3-in. outside diameter ring-lined split spoon sampler driven into the ground with a 140 lb. drop-hammer dropped 30 inches repeatedly until a penetration of 12 inches is achieved or until refusal. The number of blows required to advance the sampler 12 inches is defined as the "R" blow count. The "R" blow count requires an engineered conversion to an equivalent SPT N-Value. Refusal to penetration is considered more than 50 blows for a 6-inch increment. (Ref. ASTM D3550).

If,

R = a whole # e.g. "15", it represents the unconverted blow counts for 12 inches.

For refusal (penetration less than 12 inches), R=a whole #/X" e.g., 50/10"

"CS" in "BLOWS/FT." refers to a 2½-in. outside diameter California style split-barrel sampler, lined with brass sleeves, driven into the ground with a 140-pound hammer dropped 30 inches repeatedly until a penetration of 18 inches is achieved or until refusal. The number of blows of the hammer is recorded for each of the three 6-inch increments totaling 18 inches. The number of blows required for advancing the sampler for the last 12 inches (2nd and 3rd increments) is defined as the "CS" blow count. The "CS" blow count requires an engineered conversion to an equivalent SPT N-Value. Refusal to penetration is considered more than 50 blows for a 6-inch increment. (Ref. ASTM D3550)

"SAMPLE TYPE" refers to the form of sample recovery, in which **N** = Split-barrel sample, **R** = Ring-lined sample, **CS** = California style split-barrel sample, **G** = Grab sample, **B** = Bucket sample, **C** = Core sample (ex. diamond-bit rock coring), **S** = Shelby Tube.

"DRY DENSITY (LBS/CU FT)" refers to the laboratory-determined dry density in pounds per cubic foot. The symbol **"NR"** indicates that no sample was recovered.

"WATER (MOISTURE) CONTENT (% OF DRY WT.)" refers to the laboratory-determined water content in percent using the standard test method ASTM D2216.

"USCS" refers to the "Unified Soil Classification System" Group Symbol for the soil type as defined by ASTM D2487 and D2488. The soils were classified visually in the field, and where appropriate, classifications were modified by visual examination of samples in the laboratory and/or by appropriate tests.

These notes and boring logs are intended for use in conjunction with the purposes of our services defined in the text. Boring log data should not be construed as part of the construction plans nor as defining construction conditions.

Boring logs depict our interpretations of subsurface conditions at the locations and on the date(s) noted. Variations in subsurface conditions and characteristics may occur between borings. Groundwater levels may fluctuate due to seasonal variations and other factors.

The stratification lines shown on the boring logs represent our interpretation of the approximate boundary between soil or rock types based upon visual field classification at the boring location. The transition between materials is approximate and may be more or less gradual than indicated.

Project: LOS PRADOS SUBDIVISION DEVELOPMENT	BORING NO. 1	 Western Technologies An RMA Company
Project Number: 32-224032-0		

Date(s) Drilled 2/9/2024	Logged By S. O'HERRON-ALEX	Checked By A. KABA
Drilling Method HOLLOW STEM AUGER	Drill Bit Size/Type 7"	Total Depth of Borehole 6
Drill Rig Type CME 75	Drilling Contractor ENVIRO-DRILL	Approximate Surface Elevation NOT DETERMINED
Groundwater Level and Date Measured NO GROUNDWATER ENCOUNTERED	Sampling Method(s) GRAB, RING, SPT	Hammer Data 140-LB AUTOHAMMER
Borehole Backfill AUGER CUTTINGS	Location SEE LOCATION DIAGRAM	

Depth (feet)	WATER CONTENT	DRY DENSITY (LBS/CU FT)	SAMPLE TYPE	SAMPLE	BLOW COUNTS	USCS	GRAPHIC LOG	SOIL DESCRIPTION	REMARKS AND OTHER TESTS
0				G		GC-GM		Silty Clayey GRAVEL and Cobbles; brown, very dense, damp	
6.4	6.4	120		R	50/11"				
5	2.3	106		R	50/6"				
								REFUSAL ENCOUNTERED AT 6 FEET	
10									
15									
20									
25									

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Project: LOS PRADOS SUBDIVISION DEVELOPMENT	BORING NO. 2	 Western Technologies An RMA Company
Project Number: 32-224032-0		

Date(s) Drilled 2/8/2024	Logged By S. O'HERRON-ALEX	Checked By A. KABA
Drilling Method HOLLOW STEM AUGER	Drill Bit Size/Type 7"	Total Depth of Borehole 5
Drill Rig Type CME 75	Drilling Contractor ENVIRO-DRILL	Approximate Surface Elevation NOT DETERMINED
Groundwater Level and Date Measured NO GROUNDWATER ENCOUNTERED	Sampling Method(s) GRAB, RING, SPT	Hammer Data 140-LB AUTOHAMMER
Borehole Backfill AUGER CUTTINGS	Location SEE LOCATION DIAGRAM	

Depth (feet)	WATER CONTENT	DRY DENSITY (LBS/CU FT)	SAMPLE TYPE	SAMPLE	BLOW COUNTS	USCS	GRAPHIC LOG	SOIL DESCRIPTION	REMARKS AND OTHER TESTS
0	4.0			G		GP-GC		Poorly-graded GRAVEL with Clay; brown, dense, damp	
				N	9 15 18				
5	NR			N	50/1"			very dense REFUSAL ENCOUNTERED AT 5 FEET	
10									
15									
20									
25									

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Project: LOS PRADOS SUBDIVISION DEVELOPMENT	BORING NO. 3	 Western Technologies An RMA Company
Project Number: 32-224032-0		

Date(s) Drilled 2/8/2024	Logged By S. O'HERRON-ALEX	Checked By A. KABA
Drilling Method HOLLOW STEM AUGER	Drill Bit Size/Type 7"	Total Depth of Borehole 21.5
Drill Rig Type CME 75	Drilling Contractor ENVIRO-DRILL	Approximate Surface Elevation NOT DETERMINED
Groundwater Level and Date Measured NO GROUNDWATER ENCOUNTERED	Sampling Method(s) GRAB, RING, SPT	Hammer Data 140-LB AUTOHAMMER
Borehole Backfill AUGER CUTTINGS	Location SEE LOCATION DIAGRAM	

Depth (feet)	WATER CONTENT	DRY DENSITY (LBS/CU FT)	SAMPLE TYPE	SAMPLE	BLOW COUNTS	USCS	GRAPHIC LOG	SOIL DESCRIPTION	REMARKS AND OTHER TESTS
0	13.7			G		CL		Sandy Lean CLAY; brown, firm, damp	
7.1		86		R	12				
5	3.8	113		R	50/1"			with gravel and cobbles, hard	
10	0.7	106		R	50/9"				
15				N	21 32 30				
20				N	10 13 17			very stiff	
								BORING TERMINATED AT 21.5 FEET	
25									

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Project: LOS PRADOS SUBDIVISION DEVELOPMENT	BORING NO. 4	 Western Technologies An RMA Company
Project Number: 32-224032-0		

Date(s) Drilled 2/8/2024	Logged By S. O'HERRON-ALEX	Checked By A. KABA
Drilling Method HOLLOW STEM AUGER	Drill Bit Size/Type 7"	Total Depth of Borehole 21.5
Drill Rig Type CME 75	Drilling Contractor ENVIRO-DRILL	Approximate Surface Elevation NOT DETERMINED
Groundwater Level and Date Measured NO GROUNDWATER ENCOUNTERED	Sampling Method(s) GRAB, RING, SPT	Hammer Data 140-LB AUTOHAMMER
Borehole Backfill AUGER CUTTINGS	Location SEE LOCATION DIAGRAM	

Depth (feet)	WATER CONTENT	DRY DENSITY (LBS/CU FT)	SAMPLE TYPE	SAMPLE	BLOW COUNTS	USCS	GRAPHIC LOG	SOIL DESCRIPTION	REMARKS AND OTHER TESTS
0	7.2			G		SC		Clayey SAND; brown, loose, damp	
				N	3 5 5			light brown	
5				N	12 17 20			with gravel, dense	
10				N	68/5"			with cobbles	
15				N	18 20 26	SP-SC		Poorly-graded SAND with Clay; brown, dense, damp	
20				N	15 21 31				
								BORING TERMINATED AT 21.5 FEET	
25									

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Project: LOS PRADOS SUBDIVISION DEVELOPMENT	BORING NO. 5	 Western Technologies An RMA Company
Project Number: 32-224032-0		

Date(s) Drilled 2/8/2024	Logged By S. O'HERRON-ALEX	Checked By A. KABA
Drilling Method HOLLOW STEM AUGER	Drill Bit Size/Type 7"	Total Depth of Borehole 21.5
Drill Rig Type CME 75	Drilling Contractor ENVIRO-DRILL	Approximate Surface Elevation NOT DETERMINED
Groundwater Level and Date Measured NO GROUNDWATER ENCOUNTERED	Sampling Method(s) GRAB, RING, SPT	Hammer Data 140-LB AUTOHAMMER
Borehole Backfill AUGER CUTTINGS	Location SEE LOCATION DIAGRAM	

Depth (feet)	WATER CONTENT	DRY DENSITY (LBS/CU FT)	SAMPLE TYPE	SAMPLE	BLOW COUNTS	USCS	GRAPHIC LOG	SOIL DESCRIPTION	REMARKS AND OTHER TESTS
0	4.2			G		GC		Clayey GRAVEL with Sand and Cobbles; brown, dense, damp	
15				N	15 22 23				
5				N	21 25 32			very dense	
10				N	20 26 36				
15				N	18 19 22			light brown, dense	
20				N	16 21 28				
21.5								BORING TERMINATED AT 21.5 FEET	
25									

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Project: LOS PRADOS SUBDIVISION DEVELOPMENT	BORING NO. 6	 Western Technologies An RMA Company
Project Number: 32-224032-0		

Date(s) Drilled 2/8/2024	Logged By S. O'HERRON-ALEX	Checked By A. KABA
Drilling Method HOLLOW STEM AUGER	Drill Bit Size/Type 7"	Total Depth of Borehole 21.5
Drill Rig Type CME 75	Drilling Contractor ENVIRO-DRILL	Approximate Surface Elevation NOT DETERMINED
Groundwater Level and Date Measured NO GROUNDWATER ENCOUNTERED	Sampling Method(s) GRAB, RING, SPT	Hammer Data 140-LB AUTOHAMMER
Borehole Backfill AUGER CUTTINGS	Location SEE LOCATION DIAGRAM	

Depth (feet)	WATER CONTENT	DRY DENSITY (LBS/CU FT)	SAMPLE TYPE	SAMPLE	BLOW COUNTS	USCS	GRAPHIC LOG	SOIL DESCRIPTION	REMARKS AND OTHER TESTS
0	10.5			G		CL		Lean CLAY with Sand; brown, very stiff, damp	
4.8		103		R	44				
5	NR			R	26			light brown	
								with gravel and cobbles	
10	NR			R	50/6"			hard	
15				N	21 31 37	SP-SC		Poorly-graded SAND with Clay; light brown, very dense, damp	
20				N	95/5"				
								BORING TERMINATED AT 21.5 FEET	
25									

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Project: LOS PRADOS SUBDIVISION DEVELOPMENT	BORING NO. 7	 Western Technologies An RMA Company
Project Number: 32-224032-0		

Date(s) Drilled 2/8/2024	Logged By S. O'HERRON-ALEX	Checked By A. KABA
Drilling Method HOLLOW STEM AUGER	Drill Bit Size/Type 7"	Total Depth of Borehole 21.5
Drill Rig Type CME 75	Drilling Contractor ENVIRO-DRILL	Approximate Surface Elevation NOT DETERMINED
Groundwater Level and Date Measured NO GROUNDWATER ENCOUNTERED	Sampling Method(s) GRAB, RING, SPT	Hammer Data 140-LB AUTOHAMMER
Borehole Backfill AUGER CUTTINGS	Location SEE LOCATION DIAGRAM	

Depth (feet)	WATER CONTENT	DRY DENSITY (LBS/CU FT)	SAMPLE TYPE	SAMPLE	BLOW COUNTS	USCS	GRAPHIC LOG	SOIL DESCRIPTION	REMARKS AND OTHER TESTS
0	6.5			G		SC-SM		Silty Clayey SAND; light brown, loose, damp	
3				N	3 5 5				
5				N	4 5 7			medium dense	
10				N	26 37 41	SP-SC		with gravel and cobbles Poorly-graded SAND with Clay; light brown, very dense, damp	
15				N	50/4"				
20				N	15 18 34				
21.5								BORING TERMINATED AT 21.5 FEET	

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Project: LOS PRADOS SUBDIVISION DEVELOPMENT	BORING NO. 8	 Western Technologies An RMA Company
Project Number: 32-224032-0		

Date(s) Drilled 2/8/2024	Logged By S. O'HERRON-ALEX	Checked By A. KABA
Drilling Method HOLLOW STEM AUGER	Drill Bit Size/Type 7"	Total Depth of Borehole 21.5
Drill Rig Type CME 75	Drilling Contractor ENVIRO-DRILL	Approximate Surface Elevation NOT DETERMINED
Groundwater Level and Date Measured NO GROUNDWATER ENCOUNTERED	Sampling Method(s) GRAB, RING, SPT	Hammer Data 140-LB AUTOHAMMER
Borehole Backfill AUGER CUTTINGS	Location SEE LOCATION DIAGRAM	

Depth (feet)	WATER CONTENT	DRY DENSITY (LBS/CU FT)	SAMPLE TYPE	SAMPLE	BLOW COUNTS	USCS	GRAPHIC LOG	SOIL DESCRIPTION	REMARKS AND OTHER TESTS
0	8.2			G		CL		Sandy Lean CLAY; brown, firm, damp	
5.8		84		R	9				
6.2		86		R	11			with gravel and cobbles	
10	1.2	117		R	23			stiff	
15				N	16 18 19	SP-SC		Poorly-graded SAND with Clay; light brown, dense, damp	
20				N	15 21 25				
21.5								BORING TERMINATED AT 21.5 FEET	

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Project: LOS PRADOS SUBDIVISION DEVELOPMENT	BORING NO. 9	 Western Technologies An RMA Company
Project Number: 32-224032-0		

Date(s) Drilled 2/9/2024	Logged By S. O'HERRON-ALEX	Checked By A. KABA
Drilling Method HOLLOW STEM AUGER	Drill Bit Size/Type 7"	Total Depth of Borehole 21.5
Drill Rig Type CME 75	Drilling Contractor ENVIRO-DRILL	Approximate Surface Elevation NOT DETERMINED
Groundwater Level and Date Measured NO GROUNDWATER ENCOUNTERED	Sampling Method(s) GRAB, RING, SPT	Hammer Data 140-LB AUTOHAMMER
Borehole Backfill AUGER CUTTINGS	Location SEE LOCATION DIAGRAM	

Depth (feet)	WATER CONTENT	DRY DENSITY (LBS/CU FT)	SAMPLE TYPE	SAMPLE	BLOW COUNTS	USCS	GRAPHIC LOG	SOIL DESCRIPTION	REMARKS AND OTHER TESTS
0	9.8			G		CL		Sandy Lean CLAY; light brown, very stiff, damp	
7				N	7 9 9				
9				N	9 10 12				
10				N	9 18 28	SP-SC		with gravel and cobbles Poorly-graded SAND with Clay; light brown, dense, damp	
15				N	90/5"			very dense	
20				N	7 10 14	SC-SM		Silty Clayey SAND; brown, medium dense, damp	
21.5								BORING TERMINATED AT 21.5 FEET	

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Project: LOS PRADOS SUBDIVISION DEVELOPMENT	BORING NO. 10	 Western Technologies An RMA Company
Project Number: 32-224032-0		

Date(s) Drilled 2/9/2024	Logged By S. O'HERRON-ALEX	Checked By A. KABA
Drilling Method HOLLOW STEM AUGER	Drill Bit Size/Type 7"	Total Depth of Borehole 21.5
Drill Rig Type CME 75	Drilling Contractor ENVIRO-DRILL	Approximate Surface Elevation NOT DETERMINED
Groundwater Level and Date Measured NO GROUNDWATER ENCOUNTERED	Sampling Method(s) GRAB, RING, SPT	Hammer Data 140-LB AUTOHAMMER
Borehole Backfill AUGER CUTTINGS	Location SEE LOCATION DIAGRAM	

Depth (feet)	WATER CONTENT	DRY DENSITY (LBS/CU FT)	SAMPLE TYPE	SAMPLE	BLOW COUNTS	USCS	GRAPHIC LOG	SOIL DESCRIPTION	REMARKS AND OTHER TESTS
0	11.1			G		CL		Sandy Lean CLAY; light brown, hard, damp	
11 19 20				N					
5				N	5 6 7			stiff	
								with gravel and cobbles	
10				N	9 13 15	SP-SC		Poorly-graded SAND with Clay; light brown, medium dense, damp	
15	NR			N	15 19 23			dense	
20				N	13 18 19				
21.5								BORING TERMINATED AT 21.5 FEET	
25									

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Project: LOS PRADOS SUBDIVISION DEVELOPMENT	BORING NO. 11	 Western Technologies An RMA Company
Project Number: 32-224032-0		

Date(s) Drilled 2/8/2024	Logged By S. O'HERRON-ALEX	Checked By A. KABA
Drilling Method HOLLOW STEM AUGER	Drill Bit Size/Type 7"	Total Depth of Borehole 21.5
Drill Rig Type CME 75	Drilling Contractor ENVIRO-DRILL	Approximate Surface Elevation NOT DETERMINED
Groundwater Level and Date Measured NO GROUNDWATER ENCOUNTERED	Sampling Method(s) GRAB, RING, SPT	Hammer Data 140-LB AUTOHAMMER
Borehole Backfill AUGER CUTTINGS	Location SEE LOCATION DIAGRAM	

Depth (feet)	WATER CONTENT	DRY DENSITY (LBS/CU FT)	SAMPLE TYPE	SAMPLE	BLOW COUNTS	USCS	GRAPHIC LOG	SOIL DESCRIPTION	REMARKS AND OTHER TESTS
0	8.4			G		CL		Sandy Lean CLAY; brown, very stiff, damp	
8.7		80		R	29				
5	4.4	93		R	19			light brown, stiff	
								with gravel and cobbles	
10	1.1	119		R	26	SP-SC		Poorly-graded SAND with Clay; light brown, medium dense, damp	
15				N	30 36 43			very dense	
20				N	15 19 25				
								BORING TERMINATED AT 21.5 FEET	
25									

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Project: LOS PRADOS SUBDIVISION DEVELOPMENT Project Number: 32-224032-0	BORING NO. 12	
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Date(s) Drilled 2/9/2024	Logged By S. O'HERRON-ALEX	Checked By A. KABA
Drilling Method HOLLOW STEM AUGER	Drill Bit Size/Type 7"	Total Depth of Borehole 5
Drill Rig Type CME 75	Drilling Contractor ENVIRO-DRILL	Approximate Surface Elevation NOT DETERMINED
Groundwater Level and Date Measured NO GROUNDWATER ENCOUNTERED	Sampling Method(s) GRAB, RING, SPT	Hammer Data 140-LB AUTOHAMMER
Borehole Backfill AUGER CUTTINGS	Location SEE LOCATION DIAGRAM	

Depth (feet)	WATER CONTENT	DRY DENSITY (LBS/CU FT)	SAMPLE TYPE	SAMPLE	BLOW COUNTS	USCS	GRAPHIC LOG	SOIL DESCRIPTION	REMARKS AND OTHER TESTS
0						SC		Clayey SAND with Gravel and Cobbles; brown	
5				G				BORING TERMINATED AT 5 FEET	
10									
15									
20									
25									

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Project: LOS PRADOS SUBDIVISION DEVELOPMENT Project Number: 32-224032-0	BORING NO. 13	
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Date(s) Drilled 2/9/2024	Logged By S. O'HERRON-ALEX	Checked By A. KABA
Drilling Method HOLLOW STEM AUGER	Drill Bit Size/Type 7"	Total Depth of Borehole 5
Drill Rig Type CME 75	Drilling Contractor ENVIRO-DRILL	Approximate Surface Elevation NOT DETERMINED
Groundwater Level and Date Measured NO GROUNDWATER ENCOUNTERED	Sampling Method(s) GRAB, RING, SPT	Hammer Data 140-LB AUTOHAMMER
Borehole Backfill AUGER CUTTINGS	Location SEE LOCATION DIAGRAM	

Depth (feet)	WATER CONTENT	DRY DENSITY (LBS/CU FT)	SAMPLE TYPE	SAMPLE	BLOW COUNTS	USCS	GRAPHIC LOG	SOIL DESCRIPTION	REMARKS AND OTHER TESTS
0						SM		Silty SAND; light brown, damp	
7.5				G				with gravel	
5								BORING TERMINATED AT 5 FEET	
10									
15									
20									
25									

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Project: LOS PRADOS SUBDIVISION DEVELOPMENT	BORING NO. 14	 Western Technologies An RMA Company
Project Number: 32-224032-0		

Date(s) Drilled 2/8/2024	Logged By S. O'HERRON-ALEX	Checked By A. KABA
Drilling Method HOLLOW STEM AUGER	Drill Bit Size/Type 7"	Total Depth of Borehole 5
Drill Rig Type CME 75	Drilling Contractor ENVIRO-DRILL	Approximate Surface Elevation NOT DETERMINED
Groundwater Level and Date Measured NO GROUNDWATER ENCOUNTERED	Sampling Method(s) GRAB, RING, SPT	Hammer Data 140-LB AUTOHAMMER
Borehole Backfill AUGER CUTTINGS	Location SEE LOCATION DIAGRAM	

Depth (feet)	WATER CONTENT	DRY DENSITY (LBS/CU FT)	SAMPLE TYPE	SAMPLE	BLOW COUNTS	USCS	GRAPHIC LOG	SOIL DESCRIPTION	REMARKS AND OTHER TESTS
0						SC-SM		Silty Clayey SAND; light brown, damp	
9.2				G					
5								BORING TERMINATED AT 5 FEET	
10									
15									
20									
25									

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Project: LOS PRADOS SUBDIVISION DEVELOPMENT Project Number: 32-224032-0	BORING NO. 15	 Western Technologies An RMA Company
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Date(s) Drilled 2/9/2024	Logged By S. O'HERRON-ALEX	Checked By A. KABA
Drilling Method HOLLOW STEM AUGER	Drill Bit Size/Type 7"	Total Depth of Borehole 5
Drill Rig Type CME 75	Drilling Contractor ENVIRO-DRILL	Approximate Surface Elevation NOT DETERMINED
Groundwater Level and Date Measured NO GROUNDWATER ENCOUNTERED	Sampling Method(s) GRAB, RING, SPT	Hammer Data 140-LB AUTOHAMMER
Borehole Backfill AUGER CUTTINGS	Location SEE LOCATION DIAGRAM	

Depth (feet)	WATER CONTENT	DRY DENSITY (LBS/CU FT)	SAMPLE TYPE	SAMPLE	BLOW COUNTS	USCS	GRAPHIC LOG	SOIL DESCRIPTION	REMARKS AND OTHER TESTS
0						CL		Sandy Lean CLAY; brown, damp	
6.8				G				with gravel	
5								BORING TERMINATED AT 5 FEET	
10									
15									
20									
25									

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Project: LOS PRADOS SUBDIVISION DEVELOPMENT Project Number: 32-224032-0	BORING NO. 16	
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Date(s) Drilled 2/9/2024	Logged By S. O'HERRON-ALEX	Checked By A. KABA
Drilling Method HOLLOW STEM AUGER	Drill Bit Size/Type 7"	Total Depth of Borehole 5
Drill Rig Type CME 75	Drilling Contractor ENVIRO-DRILL	Approximate Surface Elevation NOT DETERMINED
Groundwater Level and Date Measured NO GROUNDWATER ENCOUNTERED	Sampling Method(s) GRAB, RING, SPT	Hammer Data 140-LB AUTOHAMMER
Borehole Backfill AUGER CUTTINGS	Location SEE LOCATION DIAGRAM	

Depth (feet)	WATER CONTENT	DRY DENSITY (LBS/CU FT)	SAMPLE TYPE	SAMPLE	BLOW COUNTS	USCS	GRAPHIC LOG	SOIL DESCRIPTION	REMARKS AND OTHER TESTS
0						SC-SM		Silty Clayey SAND with Gravel; brown, damp	
				G				with cobbles	
5								BORING TERMINATED AT 5 FEET	
10									
15									
20									
25									

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Project: LOS PRADOS SUBDIVISION DEVELOPMENT Project Number: 32-224032-0	BORING NO. 17	
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Date(s) Drilled 2/9/2024	Logged By S. O'HERRON-ALEX	Checked By A. KABA
Drilling Method HOLLOW STEM AUGER	Drill Bit Size/Type 7"	Total Depth of Borehole 5
Drill Rig Type CME 75	Drilling Contractor ENVIRO-DRILL	Approximate Surface Elevation NOT DETERMINED
Groundwater Level and Date Measured NO GROUNDWATER ENCOUNTERED	Sampling Method(s) GRAB, RING, SPT	Hammer Data 140-LB AUTOHAMMER
Borehole Backfill AUGER CUTTINGS	Location SEE LOCATION DIAGRAM	

Depth (feet)	WATER CONTENT	DRY DENSITY (LBS/CU FT)	SAMPLE TYPE	SAMPLE	BLOW COUNTS	USCS	GRAPHIC LOG	SOIL DESCRIPTION	REMARKS AND OTHER TESTS
0						SC-SM		Silty Clayey SAND; brown, damp	
5				G				BORING TERMINATED AT 5 FEET	
10									
15									
20									
25									

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Project: LOS PRADOS SUBDIVISION DEVELOPMENT Project Number: 32-224032-0	BORING NO. 18	
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Date(s) Drilled 2/9/2024	Logged By S. O'HERRON-ALEX	Checked By A. KABA
Drilling Method HOLLOW STEM AUGER	Drill Bit Size/Type 7"	Total Depth of Borehole 5
Drill Rig Type CME 75	Drilling Contractor ENVIRO-DRILL	Approximate Surface Elevation NOT DETERMINED
Groundwater Level and Date Measured NO GROUNDWATER ENCOUNTERED	Sampling Method(s) GRAB, RING, SPT	Hammer Data 140-LB AUTOHAMMER
Borehole Backfill AUGER CUTTINGS	Location SEE LOCATION DIAGRAM	

Depth (feet)	WATER CONTENT	DRY DENSITY (LBS/CU FT)	SAMPLE TYPE	SAMPLE	BLOW COUNTS	USCS	GRAPHIC LOG	SOIL DESCRIPTION	REMARKS AND OTHER TESTS
0						CL		Lean CLAY with Sand; brown, damp	
10.3				G					
5								BORING TERMINATED AT 5 FEET	
10									
15									
20									
25									

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Project: LOS PRADOS SUBDIVISION DEVELOPMENT Project Number: 32-224032-0	BORING NO. 19	
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Date(s) Drilled 2/9/2024	Logged By S. O'HERRON-ALEX	Checked By A. KABA
Drilling Method HOLLOW STEM AUGER	Drill Bit Size/Type 7"	Total Depth of Borehole 5
Drill Rig Type CME 75	Drilling Contractor ENVIRO-DRILL	Approximate Surface Elevation NOT DETERMINED
Groundwater Level and Date Measured NO GROUNDWATER ENCOUNTERED	Sampling Method(s) GRAB, RING, SPT	Hammer Data 140-LB AUTOHAMMER
Borehole Backfill AUGER CUTTINGS	Location SEE LOCATION DIAGRAM	

Depth (feet)	WATER CONTENT	DRY DENSITY (LBS/CU FT)	SAMPLE TYPE	SAMPLE	BLOW COUNTS	USCS	GRAPHIC LOG	SOIL DESCRIPTION	REMARKS AND OTHER TESTS
0						SC		Clayey SAND; brown, damp	
8.0				G					
5								BORING TERMINATED AT 5 FEET	
10									
15									
20									
25									

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Project: LOS PRADOS SUBDIVISION DEVELOPMENT Project Number: 32-224032-0	BORING NO. 20	 Western Technologies An RMA Company
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Date(s) Drilled 2/9/2024	Logged By S. O'HERRON-ALEX	Checked By A. KABA
Drilling Method HOLLOW STEM AUGER	Drill Bit Size/Type 7"	Total Depth of Borehole 5
Drill Rig Type CME 75	Drilling Contractor ENVIRO-DRILL	Approximate Surface Elevation NOT DETERMINED
Groundwater Level and Date Measured NO GROUNDWATER ENCOUNTERED	Sampling Method(s) GRAB, RING, SPT	Hammer Data 140-LB AUTOHAMMER
Borehole Backfill AUGER CUTTINGS	Location SEE LOCATION DIAGRAM	

Depth (feet)	WATER CONTENT	DRY DENSITY (LBS/CU FT)	SAMPLE TYPE	SAMPLE	BLOW COUNTS	USCS	GRAPHIC LOG	SOIL DESCRIPTION	REMARKS AND OTHER TESTS
0						GW-GM		Well-graded GRAVEL with Silt and Sand; brown, damp with cobbles	
3.0				G					
5								BORING TERMINATED AT 5 FEET	
10									
15									
20									
25									

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Project: LOS PRADOS SUBDIVISION DEVELOPMENT Project Number: 32-224032-0	BORING NO. 21	
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Date(s) Drilled 2/9/2024	Logged By S. O'HERRON-ALEX	Checked By A. KABA
Drilling Method HOLLOW STEM AUGER	Drill Bit Size/Type 7"	Total Depth of Borehole 5
Drill Rig Type CME 75	Drilling Contractor ENVIRO-DRILL	Approximate Surface Elevation NOT DETERMINED
Groundwater Level and Date Measured NO GROUNDWATER ENCOUNTERED	Sampling Method(s) GRAB, RING, SPT	Hammer Data 140-LB AUTOHAMMER
Borehole Backfill AUGER CUTTINGS	Location SEE LOCATION DIAGRAM	

Depth (feet)	WATER CONTENT	DRY DENSITY (LBS/CU FT)	SAMPLE TYPE	SAMPLE	BLOW COUNTS	USCS	GRAPHIC LOG	SOIL DESCRIPTION	REMARKS AND OTHER TESTS
0						SC		Clayey SAND with Gravel; brown, damp	
4.6				G					
5								BORING TERMINATED AT 5 FEET	
10									
15									
20									
25									

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Project: LOS PRADOS SUBDIVISION DEVELOPMENT Project Number: 32-224032-0	BORING NO. 22	
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Date(s) Drilled 2/9/2024	Logged By S. O'HERRON-ALEX	Checked By A. KABA
Drilling Method HOLLOW STEM AUGER	Drill Bit Size/Type 7"	Total Depth of Borehole 5
Drill Rig Type CME 75	Drilling Contractor ENVIRO-DRILL	Approximate Surface Elevation NOT DETERMINED
Groundwater Level and Date Measured NO GROUNDWATER ENCOUNTERED	Sampling Method(s) GRAB, RING, SPT	Hammer Data 140-LB AUTOHAMMER
Borehole Backfill AUGER CUTTINGS	Location SEE LOCATION DIAGRAM	

Depth (feet)	WATER CONTENT	DRY DENSITY (LBS/CU FT)	SAMPLE TYPE	SAMPLE	BLOW COUNTS	USCS	GRAPHIC LOG	SOIL DESCRIPTION	REMARKS AND OTHER TESTS
0						CL		Sandy Lean CLAY; brown, damp	
8.8				G				with gravel	
5								BORING TERMINATED AT 5 FEET	
10									
15									
20									
25									

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Project: LOS PRADOS SUBDIVISION DEVELOPMENT Project Number: 32-224032-0	BORING NO. 23	
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Date(s) Drilled 2/9/2024	Logged By S. O'HERRON-ALEX	Checked By A. KABA
Drilling Method HOLLOW STEM AUGER	Drill Bit Size/Type 7"	Total Depth of Borehole 5
Drill Rig Type CME 75	Drilling Contractor ENVIRO-DRILL	Approximate Surface Elevation NOT DETERMINED
Groundwater Level and Date Measured NO GROUNDWATER ENCOUNTERED	Sampling Method(s) GRAB, RING, SPT	Hammer Data 140-LB AUTOHAMMER
Borehole Backfill AUGER CUTTINGS	Location SEE LOCATION DIAGRAM	

Depth (feet)	WATER CONTENT	DRY DENSITY (LBS/CU FT)	SAMPLE TYPE	SAMPLE	BLOW COUNTS	USCS	GRAPHIC LOG	SOIL DESCRIPTION	REMARKS AND OTHER TESTS
0						CL		Lean CLAY with Sand; brown, damp	
10.0				G					
5								BORING TERMINATED AT 5 FEET	
10									
15									
20									
25									

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Project: LOS PRADOS SUBDIVISION DEVELOPMENT Project Number: 32-224032-0	BORING NO. 24	 Western Technologies An RMA Company
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Date(s) Drilled 2/9/2024	Logged By S. O'HERRON-ALEX	Checked By A. KABA
Drilling Method HOLLOW STEM AUGER	Drill Bit Size/Type 7"	Total Depth of Borehole 5
Drill Rig Type CME 75	Drilling Contractor ENVIRO-DRILL	Approximate Surface Elevation NOT DETERMINED
Groundwater Level and Date Measured NO GROUNDWATER ENCOUNTERED	Sampling Method(s) GRAB, RING, SPT	Hammer Data 140-LB AUTOHAMMER
Borehole Backfill AUGER CUTTINGS	Location SEE LOCATION DIAGRAM	

Depth (feet)	WATER CONTENT	DRY DENSITY (LBS/CU FT)	SAMPLE TYPE	SAMPLE	BLOW COUNTS	USCS	GRAPHIC LOG	SOIL DESCRIPTION	REMARKS AND OTHER TESTS
0						CL		Lean CLAY with Sand; brown, damp	
11.2				G					
5								BORING TERMINATED AT 5 FEET	
10									
15									
20									
25									

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Project: LOS PRADOS SUBDIVISION DEVELOPMENT Project Number: 32-224032-0	BORING NO. 25	 Western Technologies An RMA Company
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Date(s) Drilled 2/9/2024	Logged By S. O'HERRON-ALEX	Checked By A. KABA
Drilling Method HOLLOW STEM AUGER	Drill Bit Size/Type 7"	Total Depth of Borehole 5
Drill Rig Type CME 75	Drilling Contractor ENVIRO-DRILL	Approximate Surface Elevation NOT DETERMINED
Groundwater Level and Date Measured NO GROUNDWATER ENCOUNTERED	Sampling Method(s) GRAB, RING, SPT	Hammer Data 140-LB AUTOHAMMER
Borehole Backfill AUGER CUTTINGS	Location SEE LOCATION DIAGRAM	

Depth (feet)	WATER CONTENT	DRY DENSITY (LBS/CU FT)	SAMPLE TYPE	SAMPLE	BLOW COUNTS	USCS	GRAPHIC LOG	SOIL DESCRIPTION	REMARKS AND OTHER TESTS
0						CL		Sandy Lean CLAY; light brown, damp	
6.7				G					
5								BORING TERMINATED AT 5 FEET	
10									
15									
20									
25									

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Boring No.	Depth (ft.)	USCS Class.	Initial Dry Density (pcf)	Initial Water Content (%)	Compression Properties			Expansion Properties		Plasticity		Soluble Chloride (ppm)	Soluble Sulfate (ppm)	Minimum Resistivity (OHM-CM)	pH	Remarks
					Surcharge (ksf)	Total Compression (%)		Surcharge (ksf)	Expansion (%)	Liquid Limit	Plasticity Index					
						In-Situ	After Saturation									
2	0-5	GP-GC		4.0						24	4	ND	ND			
1	2-3	SP-SC	120	6.4	0.5	0.3	0.8									2
					1.0		1.3									2
					2.0		1.8									2
					4.0		1.8									2
3	2-3	CL	86	7.1				0.1	0.2							1,2
3	2-3	CL	86	7.1	0.5	1.1										
					1.0	2.2										
					2.0	3.0	11.0									2
					4.0		14.5									2
3	10-11	CL	106	0.7	0.5	0.8										
					1.0	1.5										
					2.0	2.0	5.8									2,4
					4.0		7.0									2

Note: Initial Dry Density and Initial Water Content are in-situ values unless otherwise noted.
NP = Non-Plastic

Remarks

1. Compacted density (approx. 95% of ASTM D1557 max. density at moisture content slightly below optimum.)
2. Submerged to approximate saturation.
3. Slight rebound after saturation.
4. Sample disturbance observed.



PROJECT: LOS PRADOS SUBDIVISION DEVELOPMENT
JOB NO.: 32-224032-0

SOIL PROPERTIES

PLATE
B-1

Boring No.	Depth (ft.)	USCS Class.	Initial Dry Density (pcf)	Initial Water Content (%)	Compression Properties			Expansion Properties		Plasticity		Soluble Chloride (ppm)	Soluble Sulfate (ppm)	Minimum Resistivity (OHM-CM)	pH	Remarks	
					Surcharge (ksf)	Total Compression (%)		Surcharge (ksf)	Expansion (%)	Liquid Limit	Plasticity Index						
						In-Situ	After Saturation										
6	2-3	CL	103	4.8	0.5	1.3											
					1.0	1.5											
					2.0	2.1											10.0
					4.0												12.9
8	5-6	CL	86	6.2	0.6	0.8											
					1.1	1.3											
					2.2	2.6											17.0
					4.4												21.3
11	5-6	SP-SC	93	4.4	0.6	3.3											
					1.1	4.3											
					2.2	6.3											16.3
					4.4												19.4

Note: Initial Dry Density and Initial Water Content are in-situ values unless otherwise noted.
NP = Non-Plastic

Remarks

1. Compacted density (approx. 95% of ASTM D1557 max. density at moisture content slightly below optimum.)
2. Submerged to approximate saturation.
3. Slight rebound after saturation.
4. Sample disturbance observed.



PROJECT: LOS PRADOS SUBDIVISION DEVELOPMENT
JOB NO.: 32-224032-0

SOIL PROPERTIES

PLATE
B-2

SOIL PROPERTIES																			
Boring No.	Depth (ft.)	Soil Class	Particle Size Distribution - (%) Passing by Weight														Plasticity		Remarks
			1¼"	1"	¾"	½"	3/8"	#4	#8	#10	#16	#30	#40	#50	#100	#200	L L	P I	
2	0-5	GP-GC	-	100	51	34	28	22	20	20	18	16	15	14	12	10	24	4	
3	0-5	CL	-	-	-	100	99	99	97	97	95	93	90	87	79	69	29	14	
4	0-5	SC	-	-	-	100	99	98	96	95	92	86	80	73	58	47	28	11	
5	0-5	GC	-	100	81	65	59	48	43	43	40	36	34	31	27	22	24	8	
6	0-5	CL	-	-	-	100	99	99	98	98	97	96	95	93	87	79	35	20	
7	0-5	SC-SM	-	-	100	96	93	87	81	80	76	70	65	59	47	37	21	4	
8	0-5	CL	-	100	97	95	94	91	89	89	87	83	81	78	70	60	29	13	
9	0-5	CL	-	100	97	97	96	94	91	90	87	83	80	77	68	55	38	19	
10	0-5	CL	-	-	100	99	99	98	96	96	94	91	89	85	75	57	31	6	
11	0-5	CL	-	-	-	-	100	99	96	96	94	91	89	86	75	62	41	22	
13	0-5	SM	-	-	100	98	97	97	95	94	92	88	84	78	57	39	NV	NP	
14	0-5	SC-SM	-	-	100	98	97	95	93	93	91	84	77	70	58	45	25	4	
15	0-5	CL	-	100	98	95	92	88	85	84	80	74	70	67	59	51	25	10	
18	0-5	CL	-	-	-	-	100	99	99	99	98	96	94	92	85	75	37	23	
19	0-5	SC	-	-	100	93	92	86	82	81	77	73	70	66	58	48	28	11	
20	0-5	GW-GM	-	100	71	58	52	42	36	35	30	23	19	16	12	9	NV	NP	

Note: NP = Non-Plastic

Samples obtained excluded cobbles and boulders.



PROJECT: LOS PRADOS SUBDIVISION DEVELOPMENT
JOB NO.: 32-224032-0

SOIL PROPERTIES

PLATE

B-3

SOIL PROPERTIES																			
Boring No.	Depth (ft.)	Soil Class	Particle Size Distribution - (%) Passing by Weight														Plasticity		Remarks
			1¼"	1"	¾"	½"	3/8"	#4	#8	#10	#16	#30	#40	#50	#100	#200	L L	P I	
21	0-5	SC	-	100	98	89	82	74	68	67	64	60	56	52	42	35	30	17	
22	0-5	CL	-	100	96	95	94	92	89	89	88	81	78	74	63	50	31	15	
23	0-5	CI	-	-	-	-	-	100	99	99	99	97	96	93	85	77	37	22	
24	0-5	CL	-	-	-	-	-	100	99	98	97	95	94	92	86	74	29	12	
25	0-5	CI	-	-	-	-	-	100	99	99	98	96	94	92	86	65	31	16	

Note: NP = Non-Plastic

Samples obtained excluded cobbles and boulders.



PROJECT: LOS PRADOS SUBDIVISION DEVELOPMENT
JOB NO.: 32-224032-0

SOIL PROPERTIES

PLATE

B-4

City of Santa Fe, New Mexico
UTILITY SERVICE APPLICATION

***Fill in all highlighted fields on this application. Applicant must sign and date application.**

- Check one only:**
- ☒ Sewer Service Technical Evaluation Request
 - ☒ Water Service Technical Evaluation Request
 - ☐ Agreement for Metered Service (AMS)
 - ☐ Agreement to Construct and Dedicate Public Improvements (ACD)
 - ☐ Annexation Application Water Budget
 - ☒ Water Offset Program/Water Rights Compliance Evaluation Request

WORK ORDER # _____

Applicant Name: Homewise, Inc.

Project Address: 3730 South Meadows Rd.

***Required - Attach a Plat of the Property (legal lot of record and proposed development)**

Plat Filing Information: Year 2023 ☐ Book 908 ☐ Page 15 ☐ Township, Range, Section: T16N, R8E, S1

Location: (check one only) Inside Corporate City Limits ☒ Outside Corporate City Limits ☐

Property Uniform Property Code: 1048096450107000000 **Existing Well:** Yes _____ No X

Legal Description including lot size: 14.41 acres, Lying within Section 1, T 16N, R 8E, on South Meadows Rd.

Short Description of Project: Preliminary Subdivision plat for 55 detached homes and 42 town-homes.

Including a 5.12 acre park that will be dedicated to the City of Santa Fe.

Construction Start Date: Q4 2024

***RESIDENTIAL PROJECT - Complete the following**

- | | |
|--|--------------------|
| 1. Type of project: (i.e. Single Family Residence, Subdivision, Lot split, Apartments) | <u>Subdivision</u> |
| 2. Total number of lots approved on final plat/development plan: | <u>1</u> |
| 3. Total number of homes existing or under construction: | <u>0</u> |
| 4. Size of service requested: (5/8", 3/4", 1" or 2") | _____ |

***Please fill in all categories below that apply for which water service is requested:**

--- COMPLETED BY APPLICANT ---

**Number of
Lots or Units**

96 Single Family Dwelling Unit, lot size less than 6,000 sq. ft.
1 Single Family Dwelling Unit, lot size 6,000-10,890 sq. ft.
0 Single Family Dwelling Unit, lot size greater than 10,890 sq. ft.

0 Mobile Home (in Mobile home park)
0 Accessory Dwelling Unit

0 Apartment/Condominium
0 Senior Complex

97 **Total**

--- COMPLETED BY STAFF ---

Water Use Factors	Annual Water Demand
----------------------	------------------------

.15 afy per d.u.	_____
.17 afy per d.u.	_____
.25 afy per d.u.	_____

.17 afy per d.u.	_____
.09 afy per d.u.	_____

.16 afy per d.u.	_____
.12 afy per d.u.	_____

Total Residential Water Demand _____ **AFY**

City of Santa Fe, New Mexico
UTILITY SERVICE APPLICATION

***COMMERCIAL PROJECT - Complete the following**

Type of Project: (i.e. Office, Retail, Mixed, etc.) _____

Total gross floor area of building: _____ square feet

Total area of lot, tract or parcel: _____ acres

Automatic Fire Sprinkler System: _____ Yes ☒ No

Building Construction Type: _____

Building Square Footage: _____

Site Plan Attached: _____ Yes _____ No

*Please check all use categories below that are planned for the building and the gross floor areas of each use within the proposed building.

---- COMPLETED BY APPLICANT ----

Check Type of Use

Gross Floor Area

Commercial

- | | |
|---|-------|
| ☐ Office – Non-medical | _____ |
| ☐ Medical Office | _____ |
| ☐ Office – City/State | _____ |
| ☐ Research and Development Lab | _____ |
| ☐ Manufacturing – Goods | _____ |
| ☐ Manufacturing – Consumables | _____ |
| ☐ Laundromat, Commercial | _____ |
| ☐ Laundromat, Other | _____ |
| ☐ Drycleaner | _____ |
| ☐ Plant Nursery | _____ |
| ☐ Gyms with showers | _____ |
| ☐ Gyms without showers | _____ |
| ☐ Salons | _____ |
| ☐ Pet Grooming | _____ |
| ☐ Pet Daycare | _____ |
| ☐ Retail, Large
(Individual stores or shopping areas > 75,000 sq ft) | _____ |
| ☐ Neighborhood Center/Medium Retail
(Individual stores or shopping areas 75,000-25,000 sq ft) | _____ |
| ☐ Retail, Small
(Individual stores or shopping areas < 25,000 sq ft) | _____ |
| ☐ Gallery | _____ |
| ☐ Grocery Store | _____ |
| ☐ Restaurant (full service) | _____ |
| ☐ Restaurant (limited service) | _____ |
| ☐ Gasoline Station w/ Car Wash | _____ |

---- COMPLETED BY STAFF ----

Water Use Factors

Annual Water Demand

- | | |
|----------------------------|-------|
| (0.70 afy per 10,000 s.f.) | _____ |
| (0.72 afy per 10,000 s.f.) | _____ |
| (0.58 afy per 10,000 s.f.) | _____ |
| (1.18 afy per 10,000 s.f.) | _____ |
| (0.21 afy per site) | _____ |
| (2.33 afy per site) | _____ |
| (0.78 afy per machine) | _____ |
| (0.22 afy per machine) | _____ |
| (0.41 afy per site) | _____ |
| (0.56 afy per 10,000 s.f.) | _____ |
| (8.94 afy per site) | _____ |
| (0.77 afy per site) | _____ |
| (0.21 afy per site) | _____ |
| (0.52 afy per site) | _____ |
| (0.11 afy per site) | _____ |
| (0.45 afy per 10,000 s.f.) | _____ |
| (0.43 afy per 10,000 s.f.) | _____ |
| (0.06 afy per site) | _____ |
| (0.60 afy per site) | _____ |
| (1.27 afy per 10,000 s.f.) | _____ |
| (0.02 afy per seat) | _____ |
| (1.63 afy per Site) | _____ |
| (6.56 afy per Site) | _____ |

☐ Gasoline Station _____

(0.88 afy per Site) _____

City of Santa Fe, New Mexico
UTILITY SERVICE APPLICATION

☐ Car Wash (full service) _____
☐ Car Wash (limited service) _____
☐ Auto Repair _____
☐ Car Rental _____

(5.66 afy per Site) _____
(0.94 afy per Wash Bay) _____
(0.12 afy per site) _____
(0.12 afy per site) _____

☐ Car Sales _____

(0.07 afy per 10,000 s.f.) _____

☐ Self Storage _____

(0.13 afy per site) _____

☐ Wholesale, Warehousing _____
_____ Industrial, Manufacturing _____

(0.4 afy per 10,000 s.f.) _____
(applicant estimate of water use) _____

☐ Church w/ day care or school) _____
_____ Church w/o day care or school) _____

(1.3 afy per Site) _____
(0.6 afy per Site) _____

☐ Hotel _____
_____ Motel _____

No. of rooms _____
No. of rooms _____

(.13 afy per room) _____
(.09 afy per room) _____

Public Services

☐ School, Elementary _____
_____ School, Middle or Junior High _____
☐ School, Senior High _____
_____ Schools, Daycare _____

(0.53 afy per 100 students) _____
(1.68 afy per 100 students) _____
(2.64 afy per 100 students) _____
(0.85 afy per 100 kids) _____

☐ Places of Worship _____
_____ With Daycare and school _____

(0.15 afy per site) _____
(0.95 afy per site) _____

☐ Parks _____

(1.48 afy per acre) _____

☐ **Other** (not listed above) Please attach _____

(with attachment) _____

water demand calculations and assumptions used

Total Floor Area _____

Total Commercial Water Demand _____ AFY

Total Residential Water Demand _____ AFY

TOTAL PROJECT WATER DEMAND _____ AFY

City of Santa Fe, New Mexico
UTILITY SERVICE APPLICATION

OWNER: <u>Homewise, Inc.</u> Mailing Address: <u>1301 Siler Rd</u> <u>Bldg D</u> <u>Santa Fe, NM 87507</u> Phone Number: _____ Mobile Number: _____	*Only If Applicable AGENT: <u>Jennifer Jenkins of JenkinsGavin, Inc.</u> Title: <u>Principal</u> Mailing Address: <u>130 Grant Ave</u> <u>Suite 101</u> <u>Santa Fe, NM 87501</u> Phone Number: <u>505-820-7444</u> Mobile Number: <u>505-930-6149</u>
Information Provided By: Check one: Owner ☐ Agent ☒ Signature: <u>Jennifer Jenkins</u> Date: <u>3.11.24</u>	
Technical Evaluation to be Sent to: Check one: Owner _____ Agent ☒	

COMMENTS: _____

APPLICANTS, PLEASE NOTE:

- Ordinance 2008-53, prohibits new connections outside the presumptive city limits including the Agua Fria traditional historic community (AFTHC) unless specific conditions are met. Applications for service outside the presumptive city limits and AFTHC must include documentation showing these conditions are met or the application will be rejected. The documents required are shown below.
- A map of the proposed project in relation to the existing city limits and the presumptive city limits
- A detailed description of the proposed development including the type and size of proposed land uses
- The health, safety and welfare or other legal reason for the connection
- A site water budget
- Documentation from the County of Santa Fe that county water service is not available
- Documentation from the wastewater division regarding sewer availability
- A certified Santa Fe Homes Proposal as set forth in Section 14-8.11 SFCC 1987 if applicable